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WOOD FRAMING SPECIFICATIONS [06.10.00]

- 1. WOOD FRAMING SPECIFICATIONS: ANCHORAGE, FIRING AND CONNECTORS NOT SHOWN ON THE CONSTRUCTION DOCUMENTS SHALL BE AT A MINIMUM ADHERE TO THE PRESCRIPTIVE DESIGN FOR EACH CONCRETE MIX PROPORTIONS OF MATERIALS FOR CONCRETE SHALL BE ESTABLISHED TO.

WOOD TRUSS SPECIFICATIONS [06.17.00]

- 1. TRUSS SHALL BE DESIGNED BY THE TRUSS MANUFACTURER IN ACCORDANCE WITH THE TRUSS PLATE INSTITUTE NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION (ANSI/TPI 1-2016) AND SHALL CONFORM TO THE DESIGN OF THE TRUSS MANUFACTURER'S DESIGN MANUAL.

WOOD SHRINKAGE

- 1. REGARDLESS OF THE BUILDING TYPE, BUILDING DESIGNS MUST COMPENSATE FOR THE FACT THAT WOOD SHRINKAGE CONTINUES UNTIL WOOD REACHES ITS EQUILIBRIUM MOISTURE CONTENT (EMC), WHICH AVERAGES 6% OF MOISTURE CONTENT FOR MOST STRUCTURES IN THE U.S.

REINFORCED CONCRETE - 03.30.00

- 1. GENERAL: ALL CONCRETE SHALL CONFORM TO THE LATEST EDITION OF ASTM C309 SPECIFICATIONS FOR STRUCTURAL CONCRETE AND IN THESE CONSTRUCTION DOCUMENTS.

Table with columns: ELEMENT, Fc, EXPOSURE CATEGORY, MAX CL, MAX FLY ASH, MAX W/C RATIO, MAX COARSE AGG. SIZE, MIN AIR CONTENT. Includes entry for INTERIOR SLABS ON GROUND.

CONCRETE FINISHING AND CURING

- 1. FINISHING: FINISHING OPERATIONS AND FINAL FINISHING SHALL BE COMPLETED PRIOR TO THE ACCUMULATION OF BLEED WATER ON THE SURFACE. FINAL FINISHING SHOULD NOT BEGIN UNTIL THE BLEED WATER HAS EVAPORATED AND THE WOOD SHEATHING IS DISAPPROVED FROM THE SURFACE.

CONCRETE CRACKS

- 1. EVEN WITH PROPER DESIGN AND CONSTRUCTION ALL CONCRETE WILL CRACK. PLASTIC SHRINKAGE CRACKS CONTINUE TO OPEN AS THE SLAB CURES UP TO APPROXIMATELY ONE YEAR, AND REACH 50% OF THEIR FINAL SIZE IN APPROXIMATELY TWO YEARS.

RETEMPERING (ADDING WATER TO CONCRETE ON-SITE)

- 1. WATER SHALL NOT BE ADDED TO THE MIX TRUCKS ON THE JOB SITE IN EXCESS OF THE VOLUME OF WATER THAT IS SPECIFICALLY INDICATED TO HAVE BEEN WITHHELD FROM THE READY MIX SUPPLIER.

FLOOR FLATNESS AND LEVELNESS

- 1. SCHEDULE OVERALL VALUES FOR FLATNESS (S<sub>F</sub>) AND LEVELNESS (S<sub>L</sub>) SHALL CONFORM TO THE VALUES LISTED BELOW FOR THE FLOOR SURFACE CLASSIFICATION NOTED FOR EACH SUB CATEGORY TYPES.

STRUCTURAL STEEL - 05.10.00

- 1. GENERAL: ALL STRUCTURAL STEEL IS TO BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF AISC 360 SPECIFICATION FOR STRUCTURAL STEEL BUILDING.

- 1. CONNECTIONS: CONNECTION DESIGN: ALL STEEL CONNECTIONS NOT FULLY DETAILED WITHIN THESE DRAWINGS SHALL BE DESIGNED BY A CONNECTION ENGINEER WHO SHALL BE DESIGNATED BY THE CONTRACTOR.

DRAWING INTERPRETATION:

- 1. DRAWING VIEWS LABELED AS: A. PARTIAL PLANE SECTIONS, DETAIL OR SCHEDULES LABELED WITH "TYPICAL" AT THE BEGINNING OF THEIR TITLE SHALL APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THE THOSE SPECIFICALLY SHOWN.

STRUCTURAL DEFERRED SUBMITTALS:

- 1. STRUCTURAL DEFERRED SUBMITTALS ARE THOSE PORTIONS OF THE DESIGN WHICH REQUIRE STRUCTURAL ENGINEERING THAT ARE NOT SUBMITTED AT THE TIME OF THE APPLICATION BUT ARE TO BE SUBMITTED TO THE BUILDING OFFICIAL AT A LATER DATE.

GENERAL CONDITIONS

- 1. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF THE STRUCTURE, INCLUDING THE DESIGN OF THE FOUNDATION, AND SHALL BE RESPONSIBLE FOR THE DESIGN OF THE FOUNDATION.

CONTRACTOR QUALIFICATION

- 1. WORK SHALL BE PERFORMED BY A QUALIFIED CONTRACTOR AND SUBCONTRACTOR EXPERIENCED IN THIS TYPE OF WORK. SUCH KNOWLEDGE SHALL INCLUDE MAINTENANCE RECORDS FOR PERFORMING WORK OF THIS NATURE.

FUTURE EXPANSION

- 1. NO PROVISIONS FOR ANY FUTURE EXPANSION HAVE BEEN MADE IN THE STRUCTURAL DESIGN.

SUBSTITUTIONS:

- 1. ALL REQUESTS FOR SUBSTITUTIONS OF MATERIALS OR DETAILS SHOWN IN THE CONTRACT DOCUMENTS SHALL BE SUBMITTED FOR APPROVAL DURING THE BIDDING PERIOD.

REQUEST FOR INFORMATION (RFI)

- 1. RFIs MUST INCLUDE A TRANSPARENT SHEET THAT INDICATES THE FOLLOWING: A. RFI NUMBER; B. RFI CATEGORY; C. REQUEST FOR SUBSTITUTION; D. CORRECTIVE REPAIR; E. ADDITIONAL INFORMATION REQUIRED.

SUBMITTALS

- 1. SUBMITTAL LIST AND SCHEDULE: A. THE GENERAL CONTRACTOR SHALL PREPARE A DETAILED LIST AND SCHEDULE OF ALL SUBMITTAL ITEMS TO BE SENT TO THE STRUCTURAL DESIGNER TEAM TO THE START OF CONSTRUCTION.

INSPECTIONS:

- 1. CONTRACTOR OR WORK FOR WHICH A PERMIT IS REQUIRED SHALL BE SUBJECT TO INSPECTION BY THE BUILDING OFFICIAL, AND SUCH INSPECTION OR WORK SHALL REMAIN ACCESSIBLE AND EXPOSED FOR INSPECTION PURPOSES UNTIL AFTER EXHAUSTIVE TESTING INCLUDES BUT IS NOT LIMITED TO THE FOLLOWING:

DRAWING INTERPRETATION:

- 1. DRAWING VIEWS LABELED AS: A. PARTIAL PLANE SECTIONS, DETAIL OR SCHEDULES LABELED WITH "TYPICAL" AT THE BEGINNING OF THEIR TITLE SHALL APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THE THOSE SPECIFICALLY SHOWN.

REINFORCING STEEL - 03.20.00

- 1. DETAILING OF CONCRETE REINFORCEMENT BARS AND ACCESSORIES SHALL CONFORM TO THE RECOMMENDATIONS OF THE ACI DETAILING MANUAL, ACI 318 AND 318-14 (ACI DETAILING HANDBOOK).

DESIGN CRITERIA

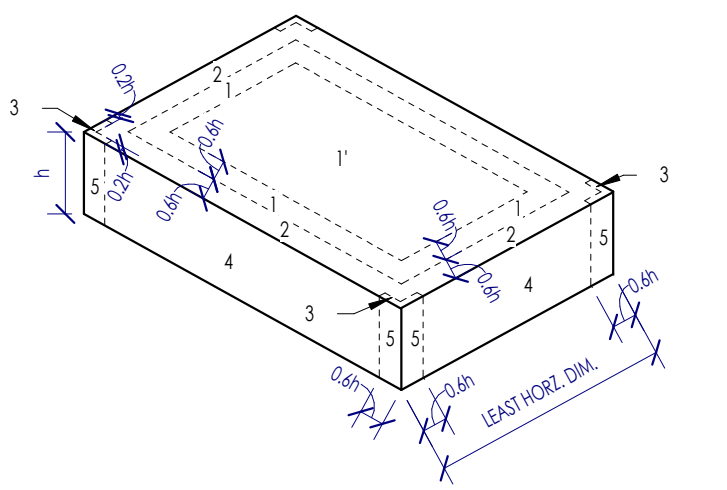
- 1. THE CONSTRUCTION DOCUMENTS ARE BASED ON THE REQUIREMENTS OF THE INTERNATIONAL BUILDING CODE WITH LOCAL AMENDMENTS FROM THE AUTHORITY HAVING JURISDICTION.

Table with columns: C&C - GROSS ULTIMATE WIND PRESSURES, Location, Effective Area, etc.

C&C - GROSS ULTIMATE WIND PRESSURES

Table with columns: Cladding Type, Location, Effective Area, etc. Includes wind pressure data for various building locations.

Table with columns: DESCRIPTION, ZONE. Lists component and cladding zones for the building.



FLAT / HIP / GABLE ROOF - h s 40 0' (0.12) x SLOPE 5:7 (1.5:12)

FOUNDATION DESIGN CRITERIA

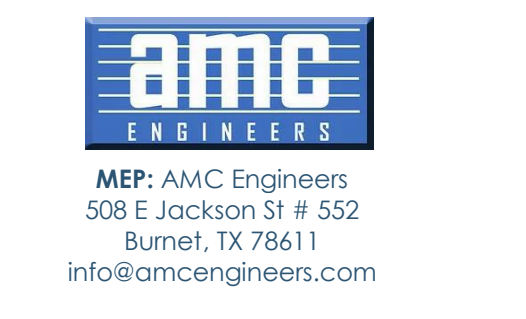
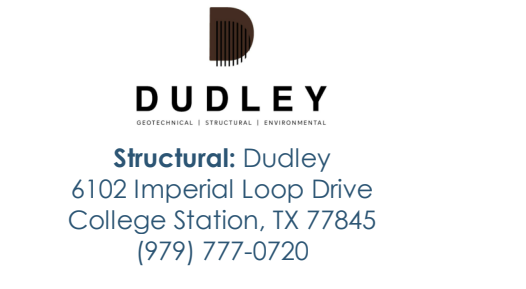
- 1. GEOTECHNICAL REPORT: THE FOUNDATION DESIGN IS BASED ON THE RECOMMENDATIONS PROVIDED IN SITE-SPECIFIC GEOTECHNICAL REPORT. IN DESIGNING THE FOUNDATION FOR THE PROPOSED STRUCTURE, THE FOUNDATION DESIGNER DOES NOT ASSUME RESPONSIBILITY FOR THE ACCURACY OF THE GEOTECHNICAL ENGINEER'S REPORT OR ANY INFORMATION CONTAINED THEREIN.

LATERAL LOAD RESISTING SYSTEM

- 1. ALL LATERAL LOAD RESISTANCE AND STABILITY OF THE BUILDING IS PROVIDED EXCLUSIVELY BY LATERAL LATERAL LOAD RESISTING SYSTEM, THE HORIZONTAL DIMENSIONS DISTRIBUTE THE LATERAL WIND AND SEISMIC FORCES HORIZONTALLY TO THE VERTICAL LATERAL LOAD RESISTING SYSTEM.

STAIR, HANDRAILS, RESTROOM ACCESSORIES AND GUARDRAIL SPECIFICATIONS:

- 1. ALL STAIRS, GUARDRAILS AND HANDRAILS SHALL BE DESIGNED BY A REGISTERED STRUCTURAL ENGINEER BASED ON THE FOLLOWING DESIGN CRITERIA:



STRUCTURAL STATEMENT OF SPECIAL INSPECTIONS & TESTING

- 1. SPECIAL INSPECTIONS AND STRUCTURAL TESTING SHALL BE PROVIDED BY AN INDEPENDENT AGENCY EMPLOYED BY THE OWNER FOR THE ITEMS IDENTIFIED IN THIS SECTION AND IN OTHER AREAS OF THE APPROVED CONSTRUCTION PLANS AND SPECIFICATIONS...
2. THE NAMES AND CREDENTIALS OF THE SPECIAL INSPECTORS TO BE USED SHALL BE SUBMITTED TO THE BUILDING OFFICIAL FOR APPROVAL...
3. DATES OF THE SPECIAL INSPECTIONS...
4. DUES AND RESPONSIBILITIES OF THE CONTRACTOR...

WIND-RESISTING COMPONENTS (7703.1.1.3)

- PERIODIC SPECIAL INSPECTION IS REQUIRED FOR FASTENING OF THE FOLLOWING SYSTEMS AND COMPONENTS:
1. ROOF COVERING, ROOF DECK AND ROOF FRAMING CONNECTIONS
2. EXTERIOR WALL COVERING AND WALL CONNECTIONS TO ROOF AND FLOOR DIAPHRAGMS AND FRAMING

REQUIRED VERIFICATION AND INSPECTION OF STRUCTURAL STEEL CONSTRUCTION (§1703.2.1)

STRUCTURAL STEEL - GENERAL
THE SPECIAL INSPECTOR SHALL INSPECT THE FABRICATED OR ERECTED STEEL FRAME, AS APPROPRIATE, TO VERIFY COMPLIANCE WITH THE DETAIL SHOWN ON THE CONSTRUCTION DOCUMENTS, SUCH AS BRACES, STIFFENERS, MEMBER LOCATIONS AND PROPER APPLICATION OF JOINT DETAILS AT EACH CONNECTION.

STRUCTURAL STEEL - ANCHOR RODS / EMBED PLATES
THE SPECIAL INSPECTOR SHALL BE ON THE PREMISES FOR INSPECTION DURING THE PLACEMENT OF ANCHOR RODS AND OTHER EMBEDMENT SUPPORTING STRUCTURAL STEEL FOR COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS, AS A MINIMUM, THE DIAMETER, GRADE, TYPE AND LENGTH OF THE ANCHOR RODS OR EMBEDDED ITEM, AND THE EXTENT OR DEPTH OF EMBEDMENT INTO THE CONCRETE, SHALL BE VERIFIED PRIOR TO PLACING OF CONCRETE.

STRUCTURAL STEEL - WELDS

Table with 4 columns: Verification and Inspection, Continuous, Periodic, Required. Rows include inspection tasks prior to welding, welding procedure specification, manufacturer certification, material identification, welder identification, fit-up groove welds, configuration and finish of access holes, fit-up fillet welds, check welding equipment.

INSPECTION TASKS DURING WELDING (ASCC 340 TABLE NE 4-2)

Table with 4 columns: Verification and Inspection, Continuous, Periodic, Required. Rows include use of qualified welders, control and handling of welding consumables, no welding over cracked tack welds, environmental conditions, WPS followed, welding techniques, welds cleaned, size, length and location of welds, welds meet visual acceptance criteria, arc strikes, i-area, backing removed, repair activities, document acceptance or rejection of welded joint member.

NON-DESTRUCTIVE TESTING OF WELDED JOINTS

Table with 4 columns: Verification and Inspection, Continuous, Periodic, Required. Rows include fillet welds (MT, PERIODIC MT, INCREASE MT), partial joint penetration (PJP) welds including flare bevel welds (MT, PERIODIC MT, INCREASE MT), complete joint penetration (CJP) welds (X, PERIODIC MT, INCREASE MT).

STRUCTURAL STEEL HIGH-STRENGTH BOLTS (TURN-OF-NUT)

TURN-OF-NUT PRETENSIONING: THE INSPECTOR SHALL OBSERVE THE PRE-INSTALLATION VERIFICATION TESTING REQUIRED IN SECTION 8.2. SUBSEQUENTLY, IT SHALL BE ENSURED BY ROUTINE OBSERVATION THAT THE BOLTING CREW PROPERLY ROTATES THE TURNED ELEMENT RELATIVE TO THE UNTURNED ELEMENT BY THE AMOUNT SPECIFIED IN TABLE 8.2, AS APPROPRIATE, WHEN FASTENER ASSEMBLIES ARE MANIPULATED AFTER THE INITIAL FIT-UP OF THE JOINT BUT PRIOR TO PRETENSIONING. VISUAL INSPECTION AFTER PRETENSIONING IS PERMITTED IN LIEU OF ROUTINE OBSERVATION, NO FURTHER EVIDENCE OF CONFORMANCY IS REQUIRED. A PRETENSION THAT IS GREATER THAN THE VALUE SPECIFIED IN TABLE 8.1 SHALL NOT BE CAUSE FOR REJECTION. A ROTATION THAT EXCEEDS THE REQUIRED VALUES, INCLUDING TOLERANCE, SPECIFIED IN TABLE 8.2 SHALL NOT BE CAUSE FOR REJECTION.

Table 8.2: NUT ROTATION FROM SNUG-TIGHT CONDITION FOR TURN-OF-NUT PRETENSIONING. Columns: Bolt Length, Disposition of Outer Faces of Bolted Parts (Both faces normal to bolt axis, One face normal to bolt axis, Other sloped not more than 1:20, Both faces sloped not more than 1:20 from normal to bolt axis).

- a. NUT ROTATION IS RELATIVE TO BOLT REGARDLESS OF THE ELEMENT (NUT OR BOLT) BEING TURNED. FOR ALL REQUIRED ROTATIONS, THE TOLERANCE IS PLUS OR MINUS.
b. APPLICABLE TO JOINTS IN WHICH ALL MATERIAL WITHIN THE GRIP IS STEEL.

STRUCTURAL STEEL HIGH-STRENGTH BOLTS (ENUG-TIGHT) - INSPECTION TASKS PRIOR TO BOLTING

Table with 4 columns: Verification and Inspection, Continuous, Periodic, Required. Row: Documentation and acceptance or rejection of bolted connections.

STRUCTURAL STEEL HIGH-STRENGTH BOLTS (ENUG-TIGHT) - INSPECTION TASKS DURING BOLTING

Table with 4 columns: Verification and Inspection, Continuous, Periodic, Required. Row: Documentation of acceptance or rejection of bolted connections.

REQUIRED VERIFICATION AND INSPECTION OF GRADING AND DRAINAGE FOR FOUNDATIONS ON EXPANSIVE SOILS

Table with 4 columns: Verification and Inspection, Continuous, Periodic, Required. Rows: Final grades, drainage, grading around structure, plumb line, surface finish.

REQUIRED VERIFICATION AND INSPECTION OF SOILS (TABLE 1703.4)

Table with 4 columns: Verification and Inspection, Continuous, Periodic, Required. Rows: Material strength, excavation, soil classification, soil density, soil fill.

REQUIRED VERIFICATION AND INSPECTION OF WOOD CONSTRUCTION (§1703.5)

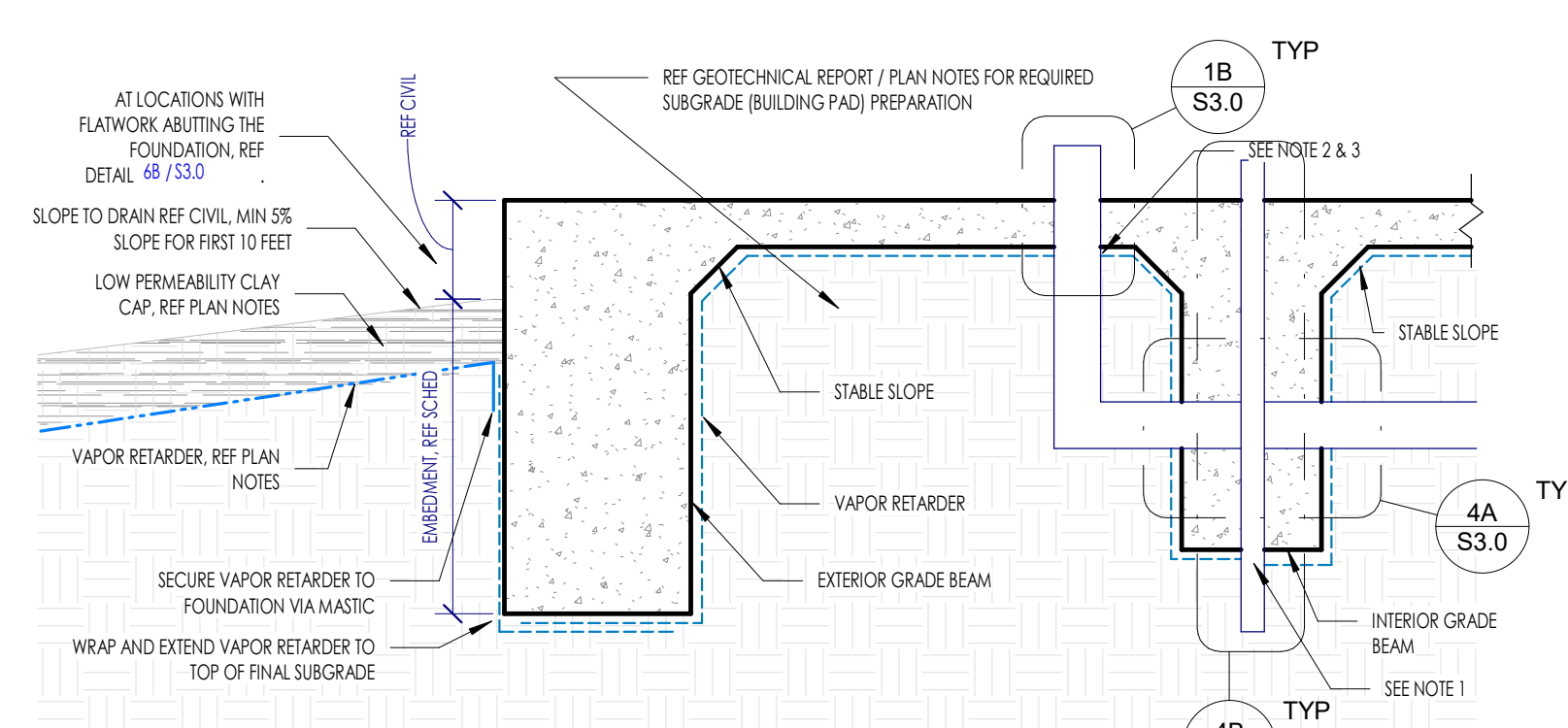
Table with 4 columns: Verification and Inspection, Continuous, Periodic, Required. Rows: Truss fabrication, high-load diaphragms, metal plate-connected wood trusses, inspection of nailing, bolting, anchoring, moisture content.

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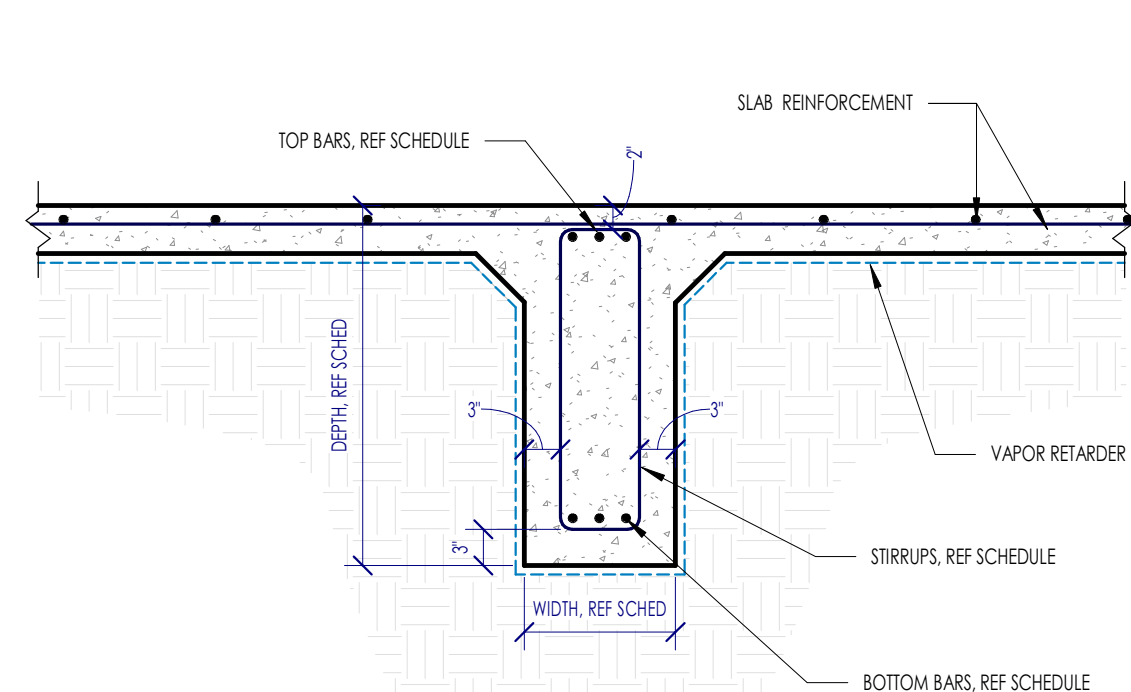
Table with 2 columns: Date, Description. A large empty table for project notes.

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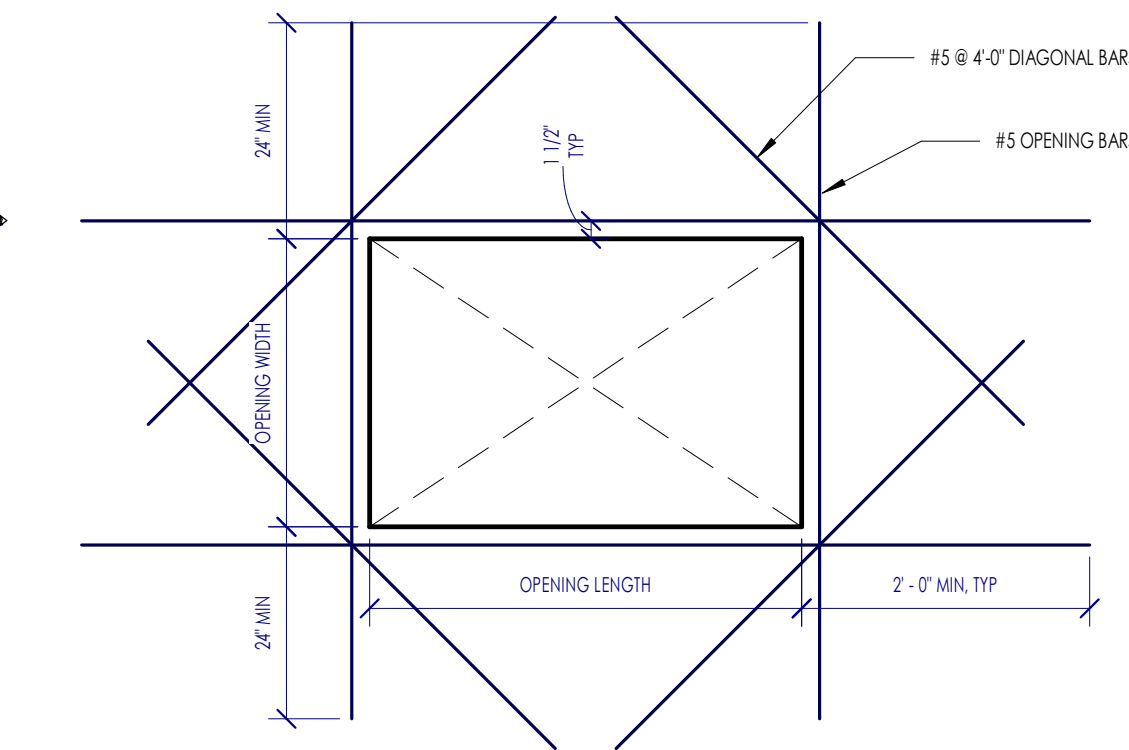


- NOTES:  
1. CUT AND/OR LAP THE VAPOR RETARDER AT THE BOTTOM OF EXTERIOR GRADE BEAMS. THE VAPOR RETARDER SHALL BE SECURED TO THE SIDES OF THE GRADE BEAM. IF LAPS ARE REQUIRED ON TOP OF THE SLAB, THEY MUST BE SEALED PER MFR RECOMMENDATIONS.  
2. ALL PIPE, DUCTING, BEAM, WIRE PENETRATIONS AND BLOCK OUTS SHOULD BE SEALED USING AFR RECOMMENDED WRAP, TAPE AND/OR MASTIC.  
3. IN THE EVENT THAT THE VAPOR RETARDER IS DAMAGED DURING OR AFTER INSTALLATION, REPAIRS MUST BE MADE. FOR HOLE, CUT, OR CRACKS OF VAPOR RETARDER TO A SIZE AND SHAPE THAT COVERS ANY DAMAGE BY A MINIMUM OVERLAP OF 6" IN ALL DIRECTIONS. CLEAN ALL ADHESION AREAS OF DUST, OIL, MOISTURE, AND FROST. TAPE DOWN ALL EDGES USING AFR RECOMMENDED TAPE.

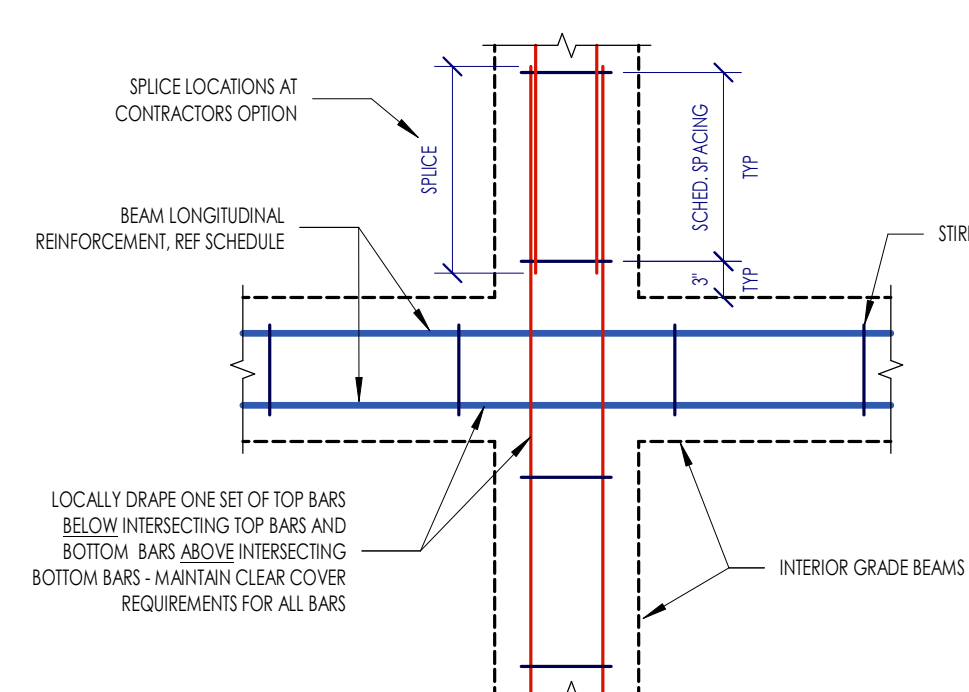
1B TYPICAL SUBGRADE AND VAPOR RETARDER PREPARATION  
NOT TO SCALE



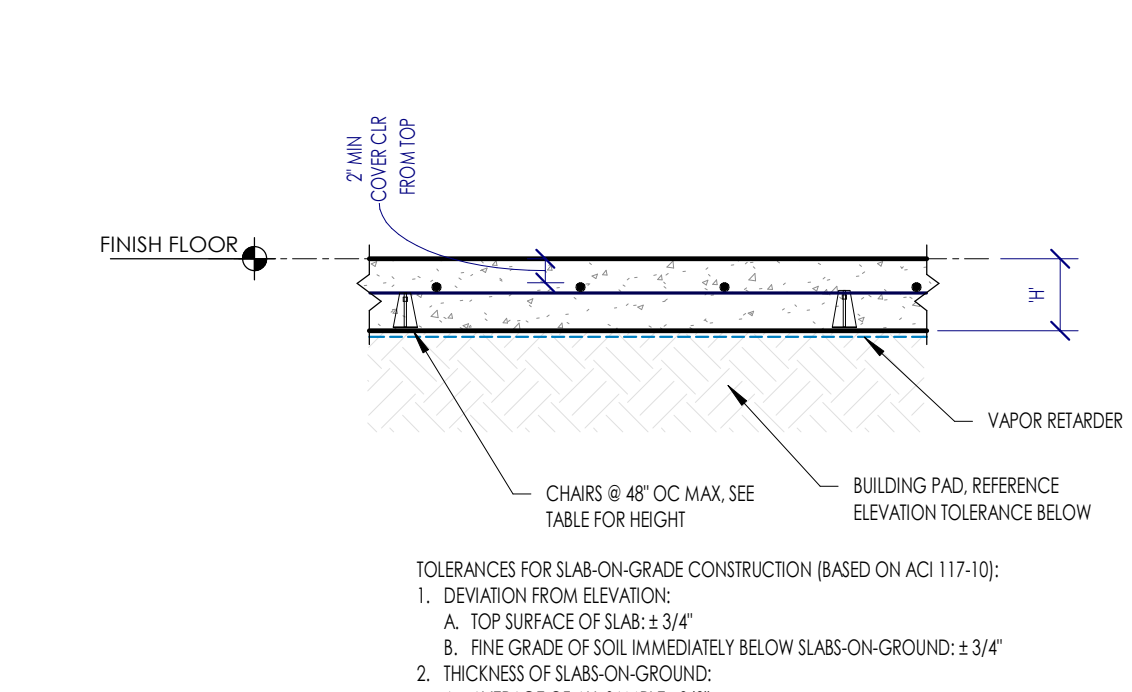
2D TYPICAL INTERIOR GRADE BEAM  
NOT TO SCALE



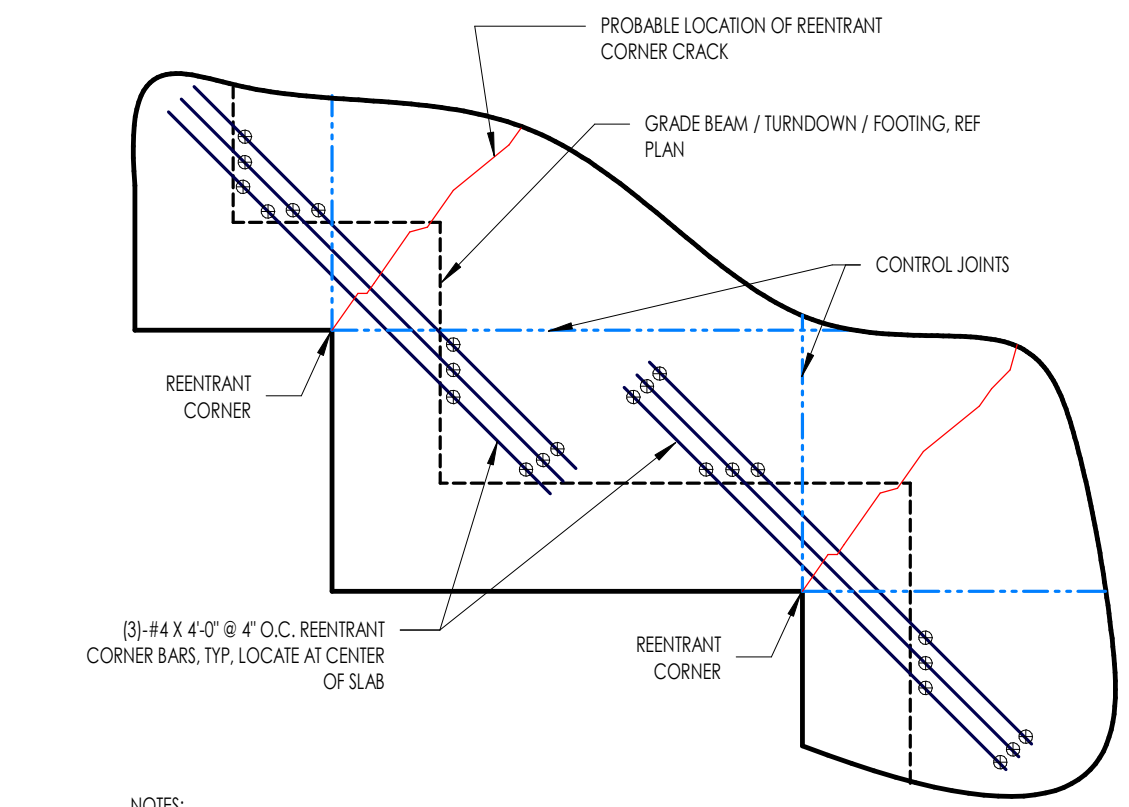
1D TYPICAL REINFORCEMENT AT SLAB BLOCKOUT  
NOT TO SCALE



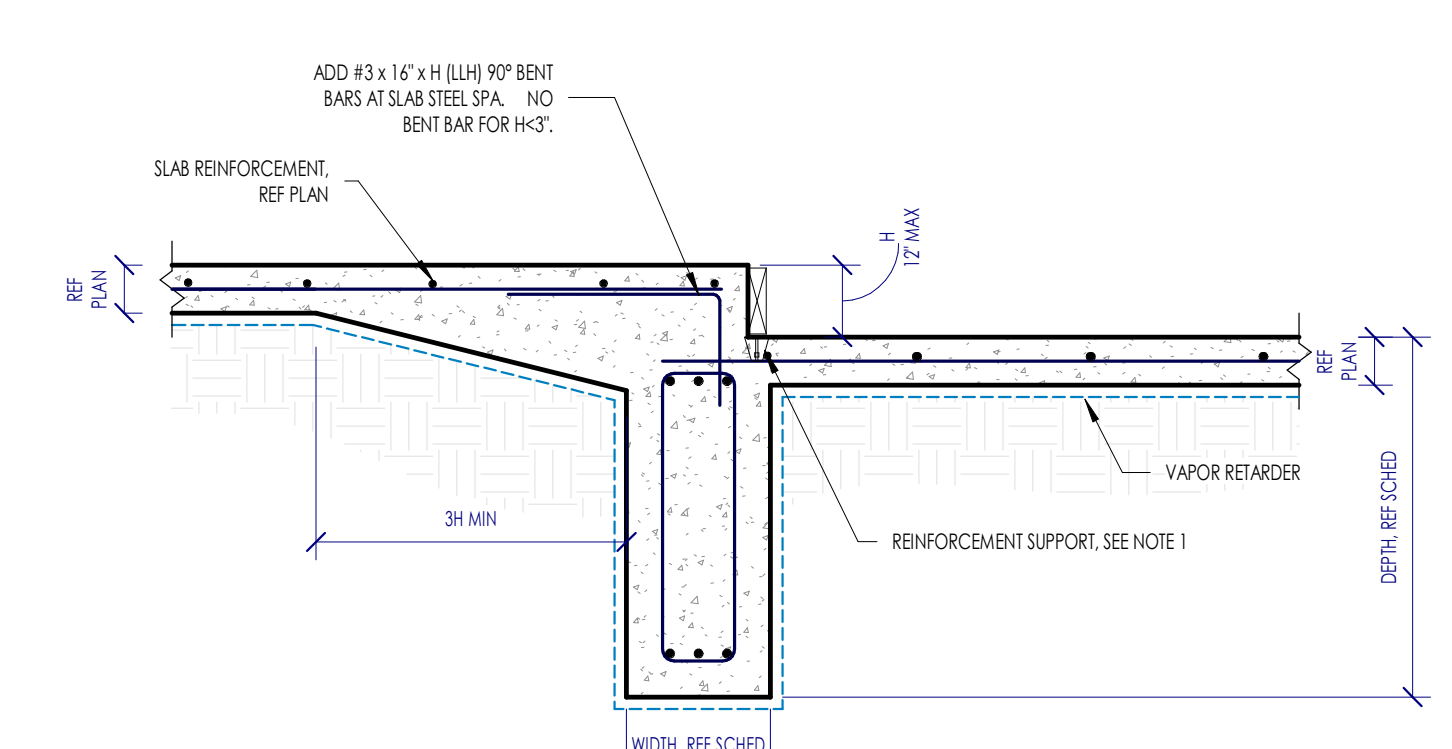
6C TYPICAL INTERIOR BEAM INTERSECTION  
NOT TO SCALE



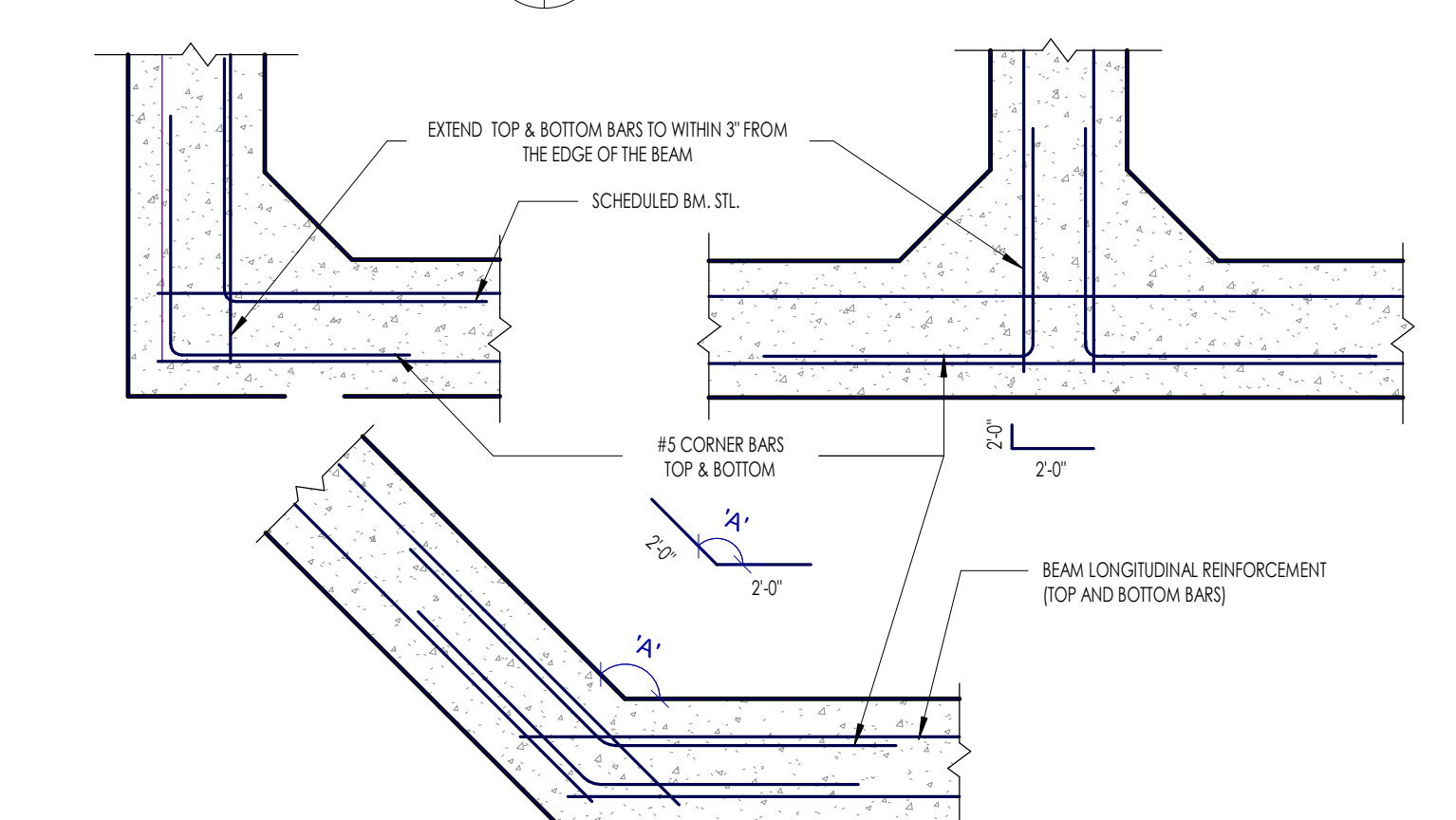
5C TYPICAL SLAB-ON-GRADE SECTION  
NOT TO SCALE



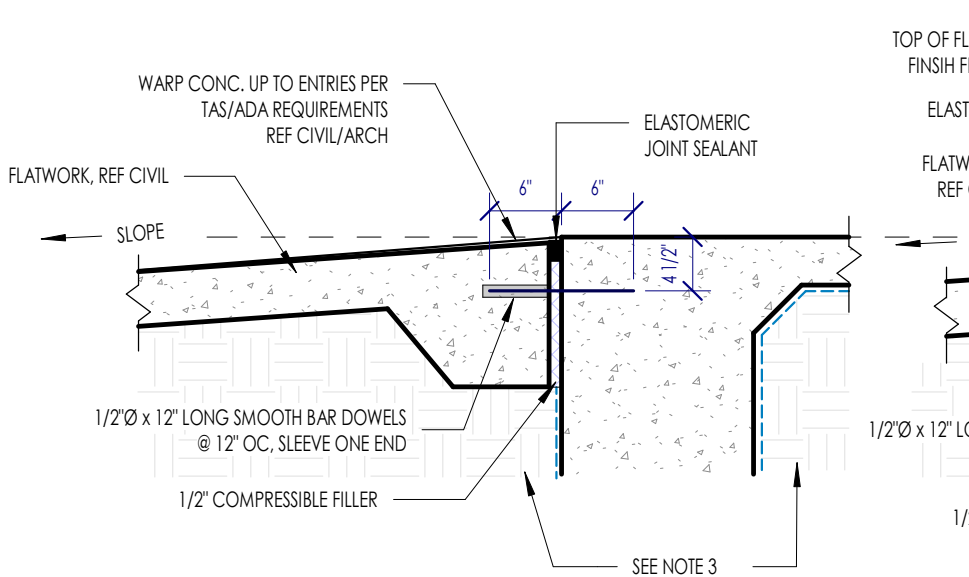
4C TYPICAL REENRANT CORNER BARS  
NOT TO SCALE



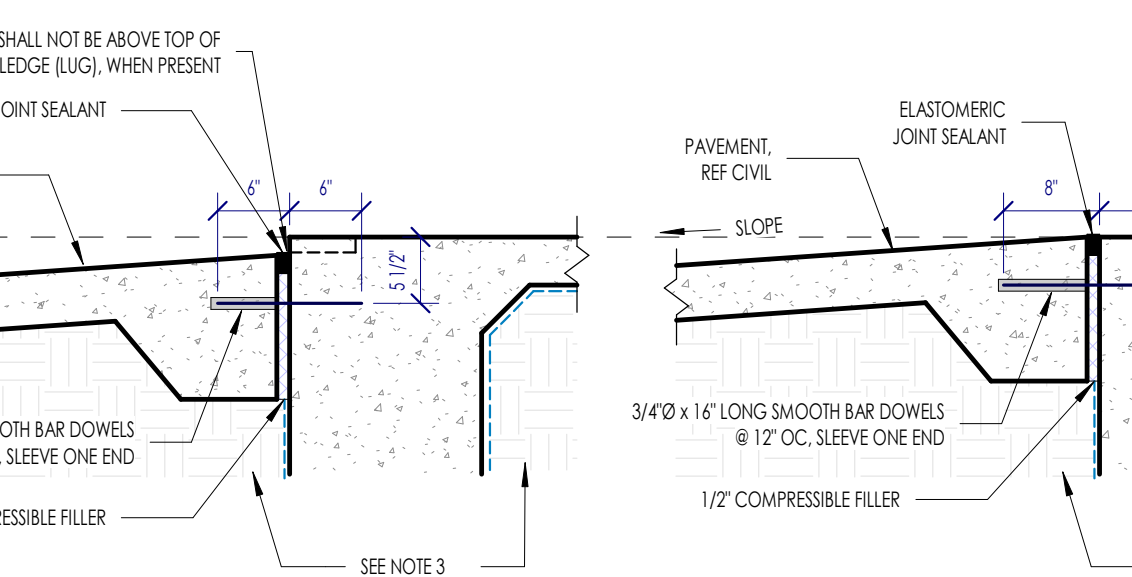
3C TYPICAL SLAB DROP AT GRADE BEAM  
NOT TO SCALE



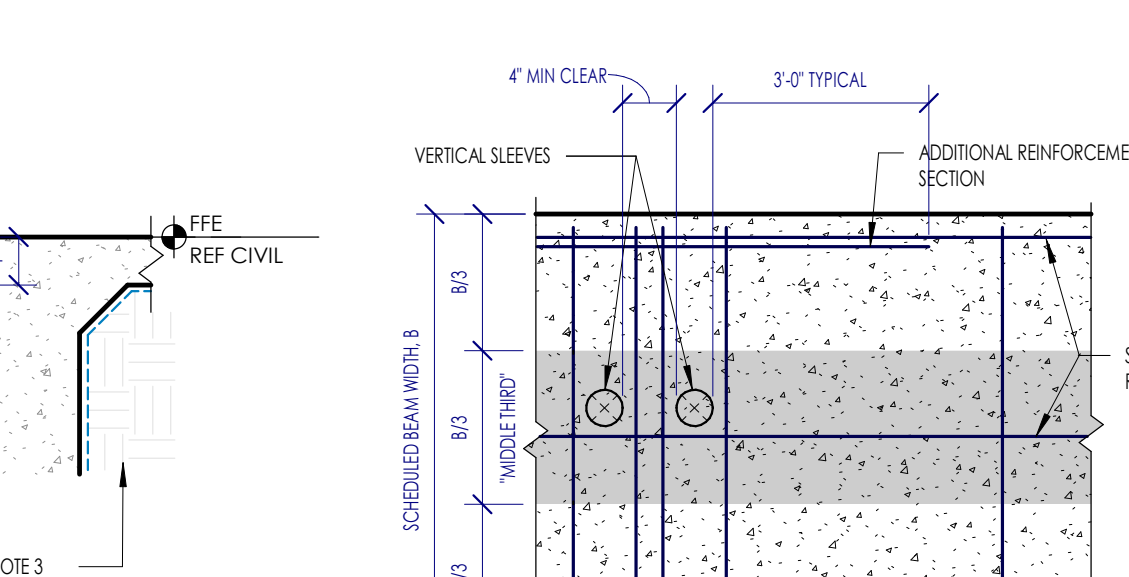
1C TYPICAL CORNER BARS  
NOT TO SCALE



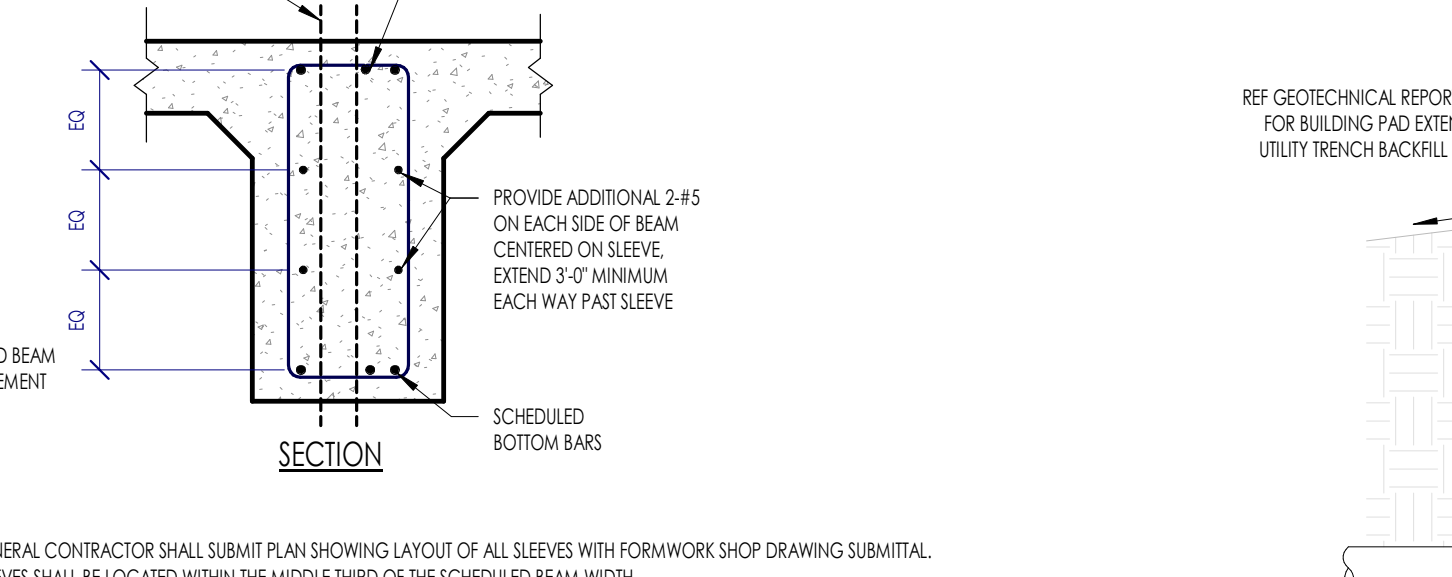
B FLATWORK AT ENTRY DOOR



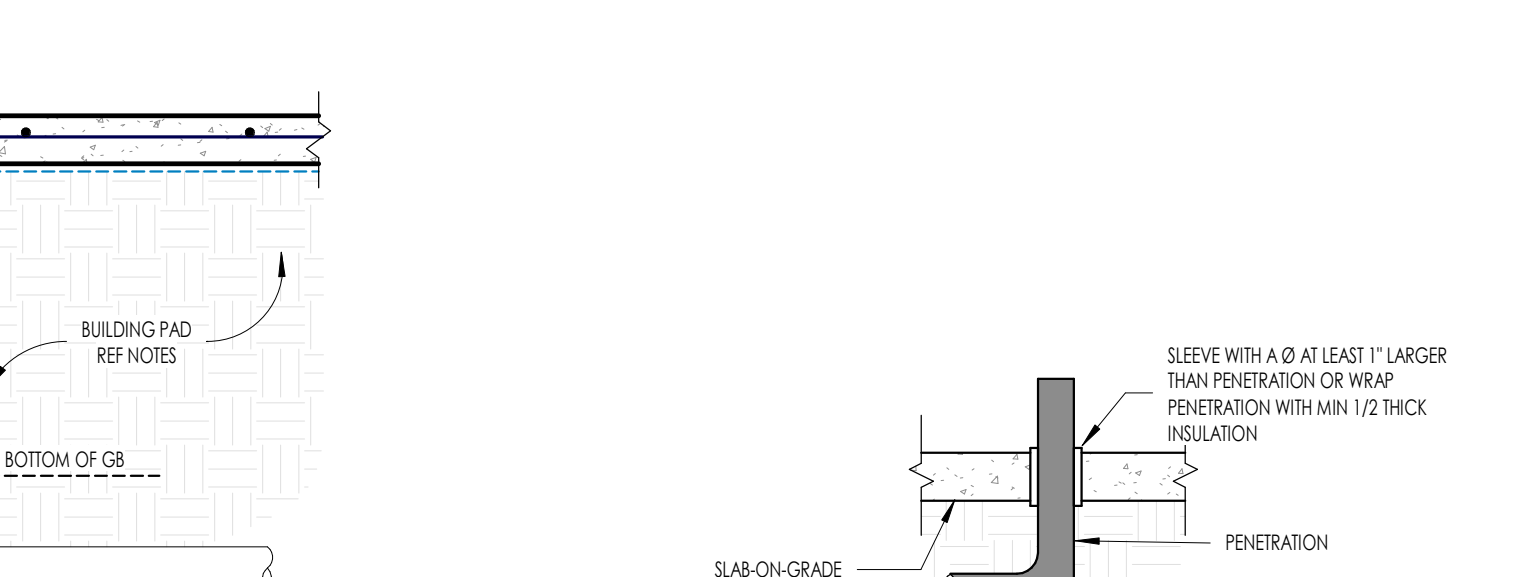
B FLATWORK NOT AT ENTRY DOOR



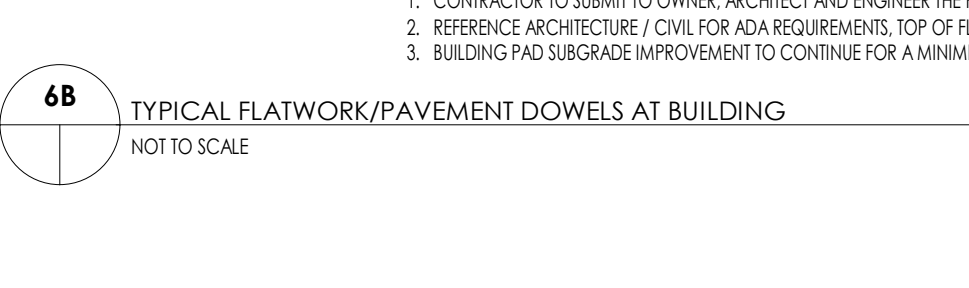
4B TYPICAL VERTICAL PENETRATION IN GRADE BEAM  
NOT TO SCALE



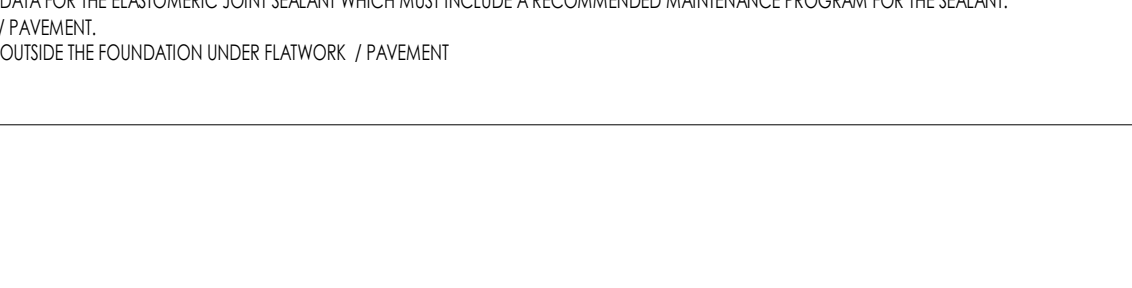
2B TYPICAL UTILITY TRENCH UNDER BUILDING PAD BENTONITE PLUG AT EXTERIOR BEAM.  
NOT TO SCALE



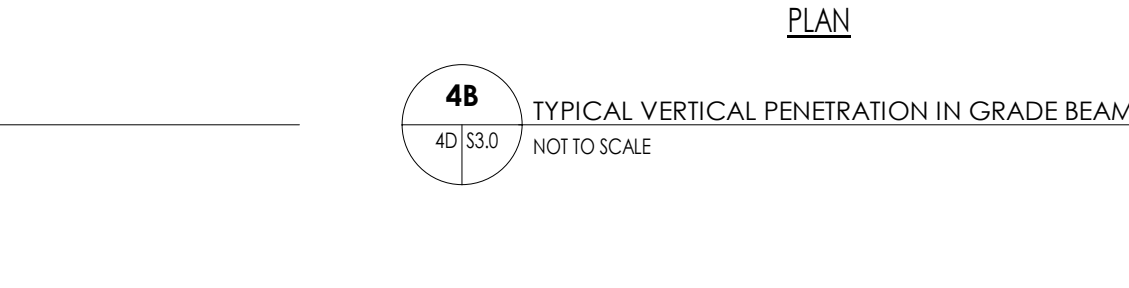
1B VERTICAL PENETRATION THROUGH SLAB-ON-GRADE  
NOT TO SCALE



6B TYPICAL FLATWORK/PAVEMENT DOWELS AT BUILDING  
NOT TO SCALE



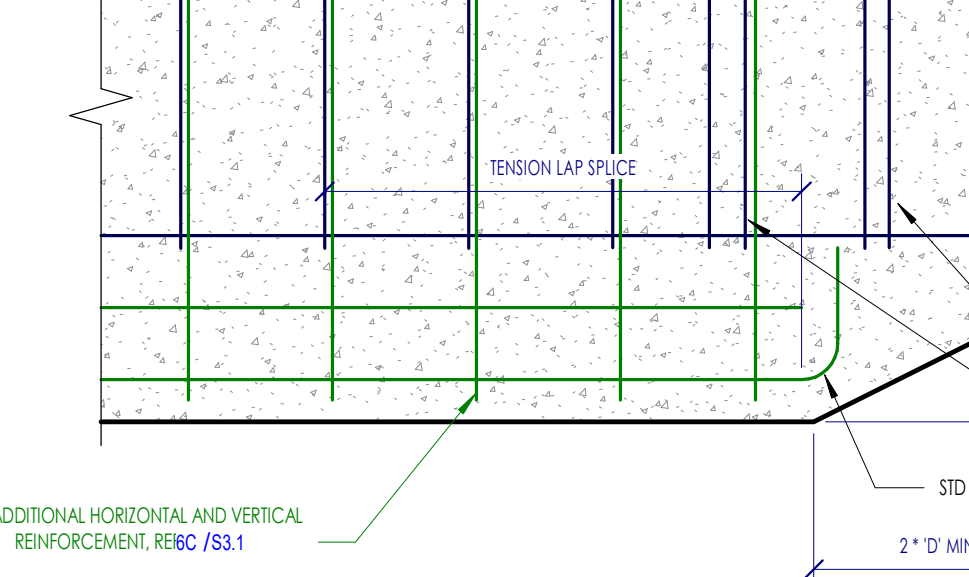
4A TYPICAL HORIZONTAL PENETRATION IN BEAM  
NOT TO SCALE



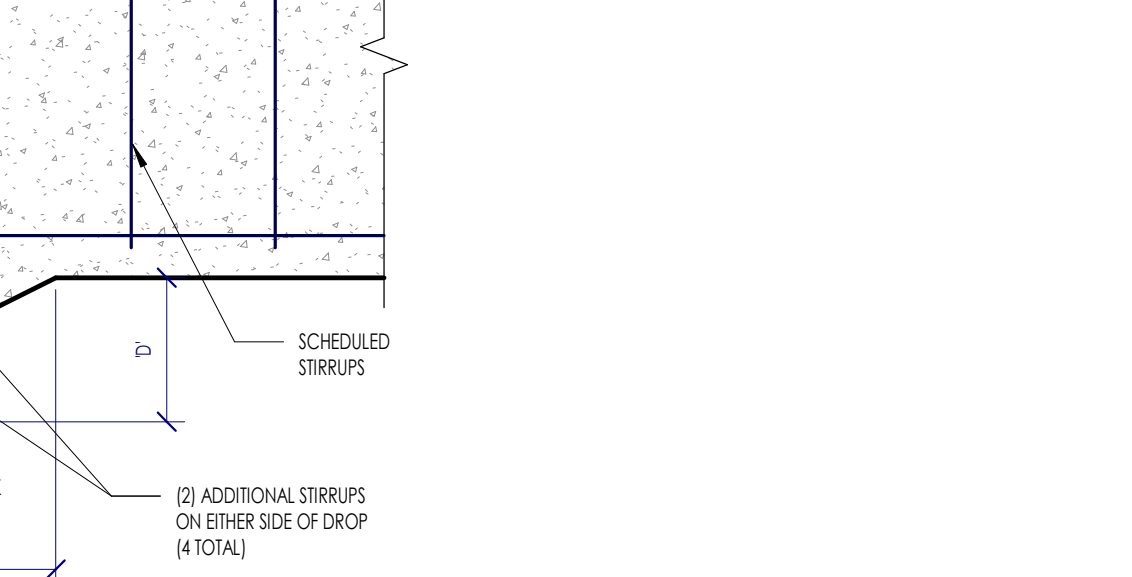
2A TYPICAL DOUBLE WIDE INTERIOR GRADE BEAM  
NOT TO SCALE



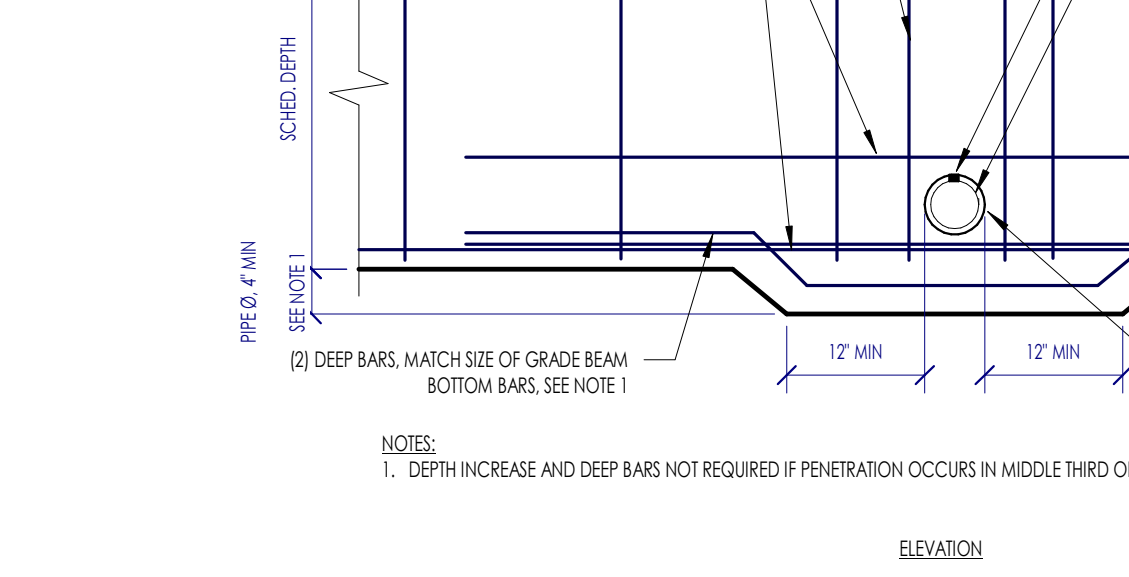
1A TYPICAL CONDUITS EMBEDDED IN SLAB-ON-GRADE  
NOT TO SCALE



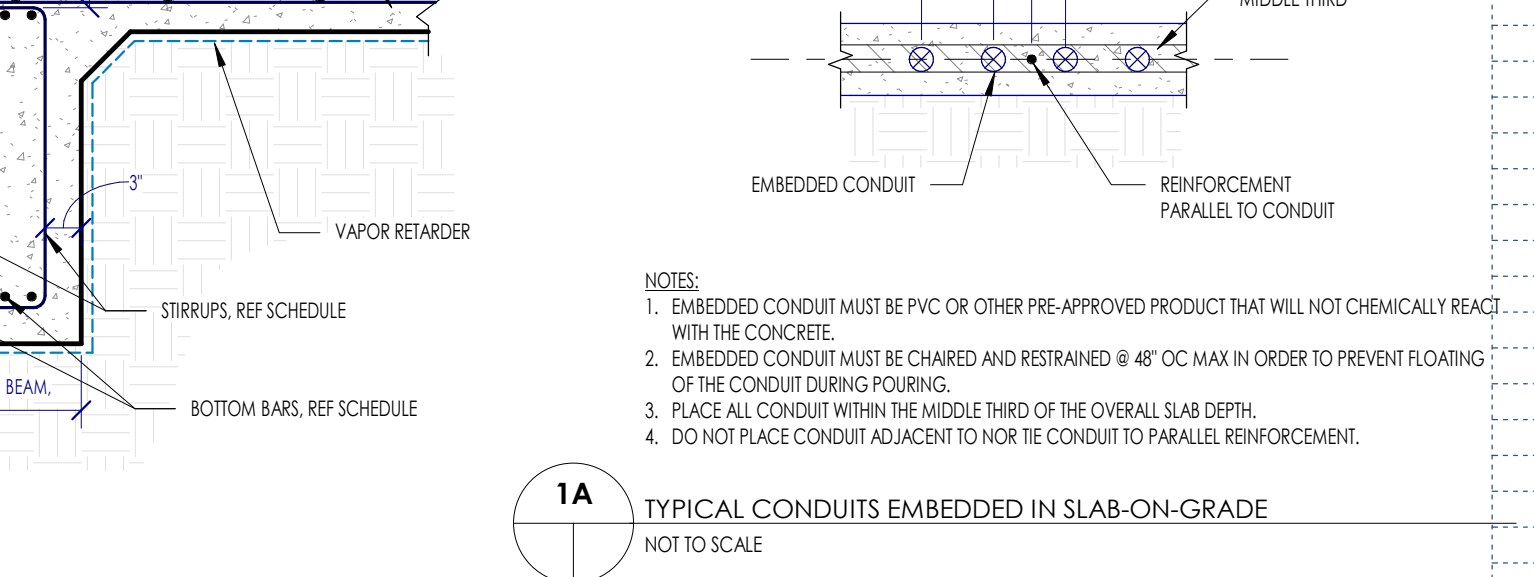
6A TYPICAL DROP TRANSITION IN GRADE BEAM - VERTICAL MOISTURE BARRIER  
NOT TO SCALE



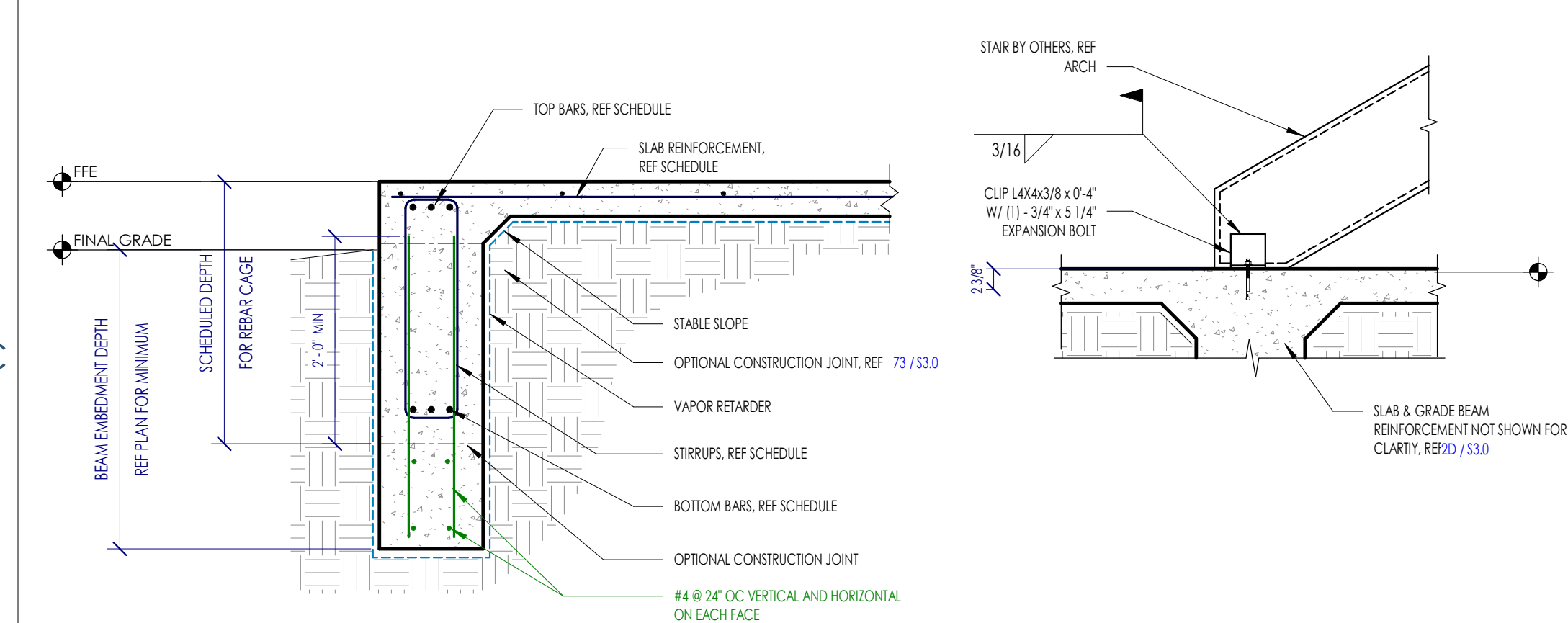
4A TYPICAL HORIZONTAL PENETRATION IN BEAM  
NOT TO SCALE



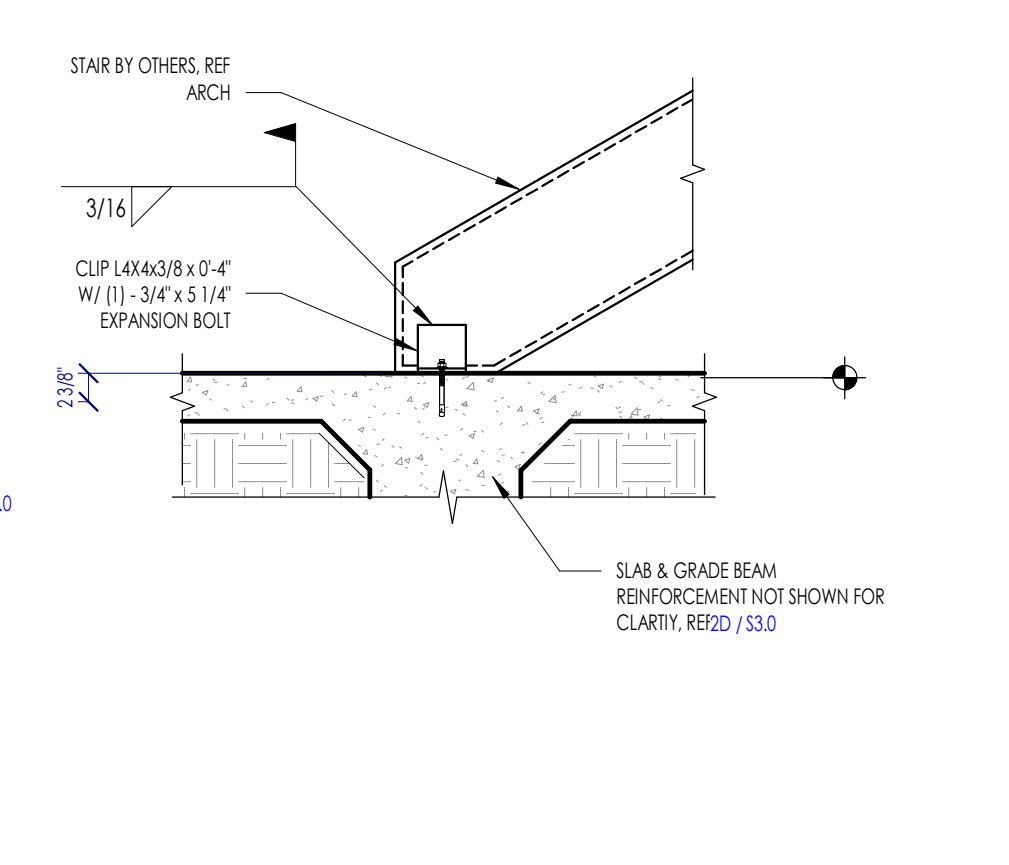
2A TYPICAL DOUBLE WIDE INTERIOR GRADE BEAM  
NOT TO SCALE



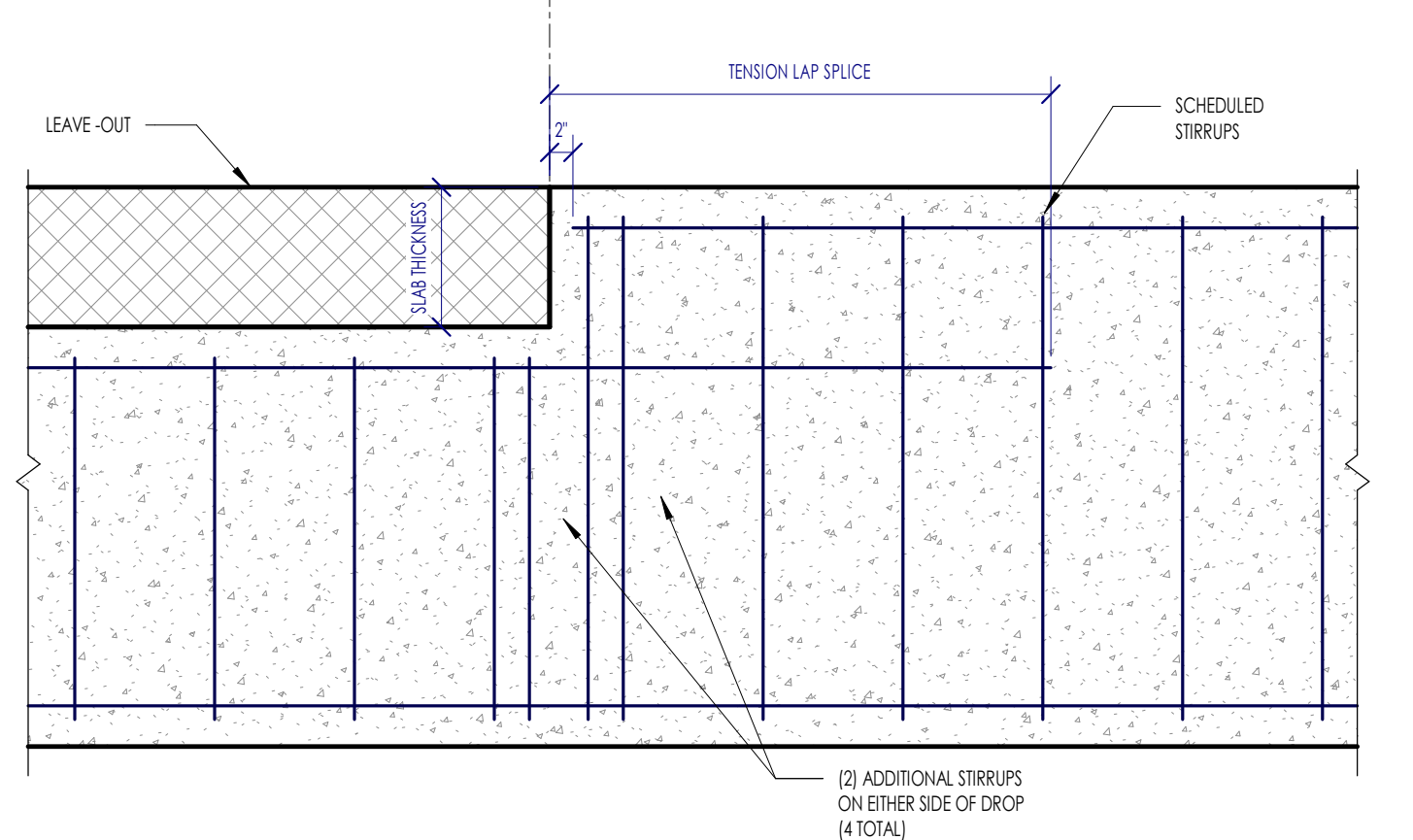
1A TYPICAL CONDUITS EMBEDDED IN SLAB-ON-GRADE  
NOT TO SCALE



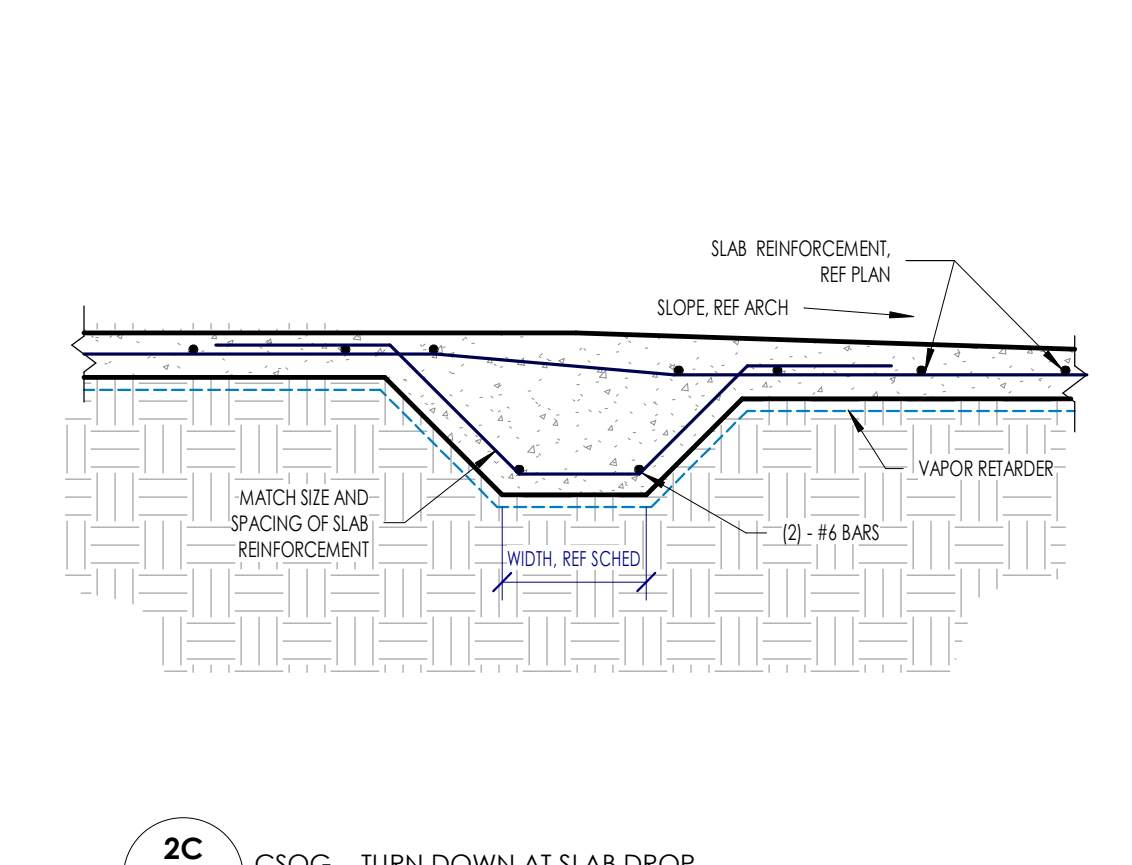
4C TYPICAL EXTERIOR GRADE BEAM - VERTICAL MOISTURE BARRIER NOT TO SCALE



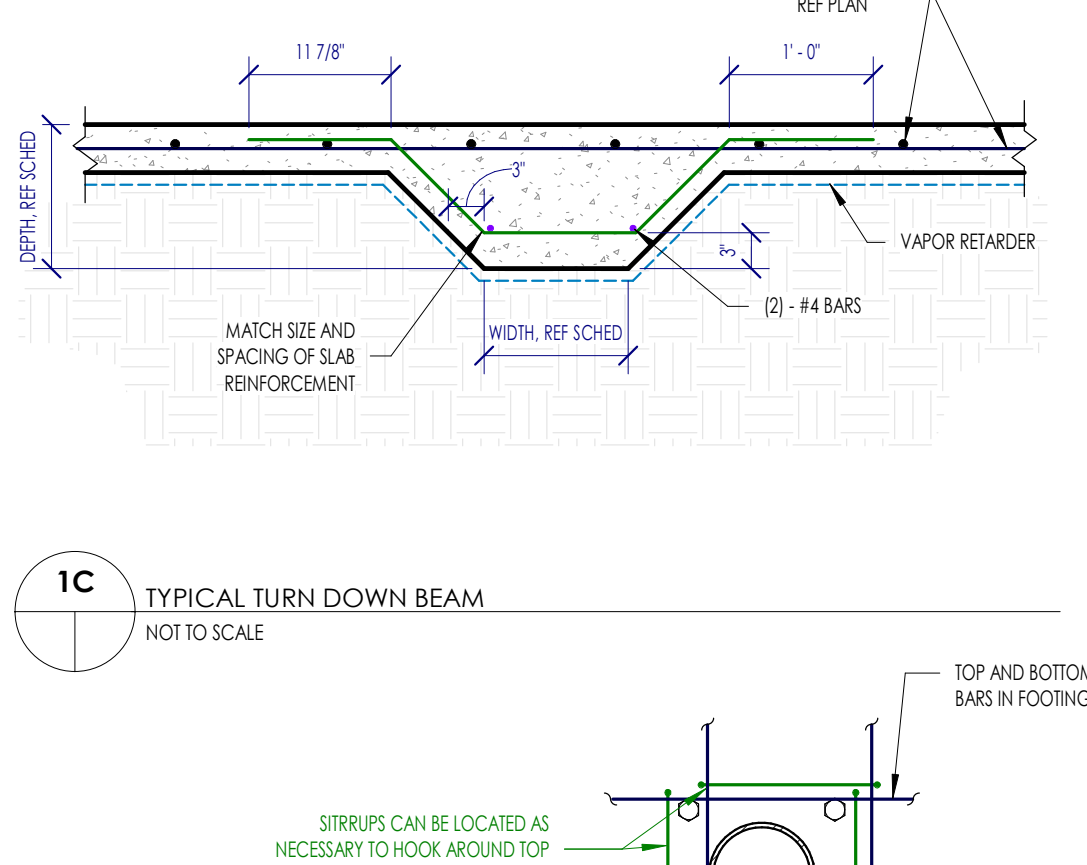
5C TYPICAL GRADE BEAM AT STEEL STAIRS NOT TO SCALE



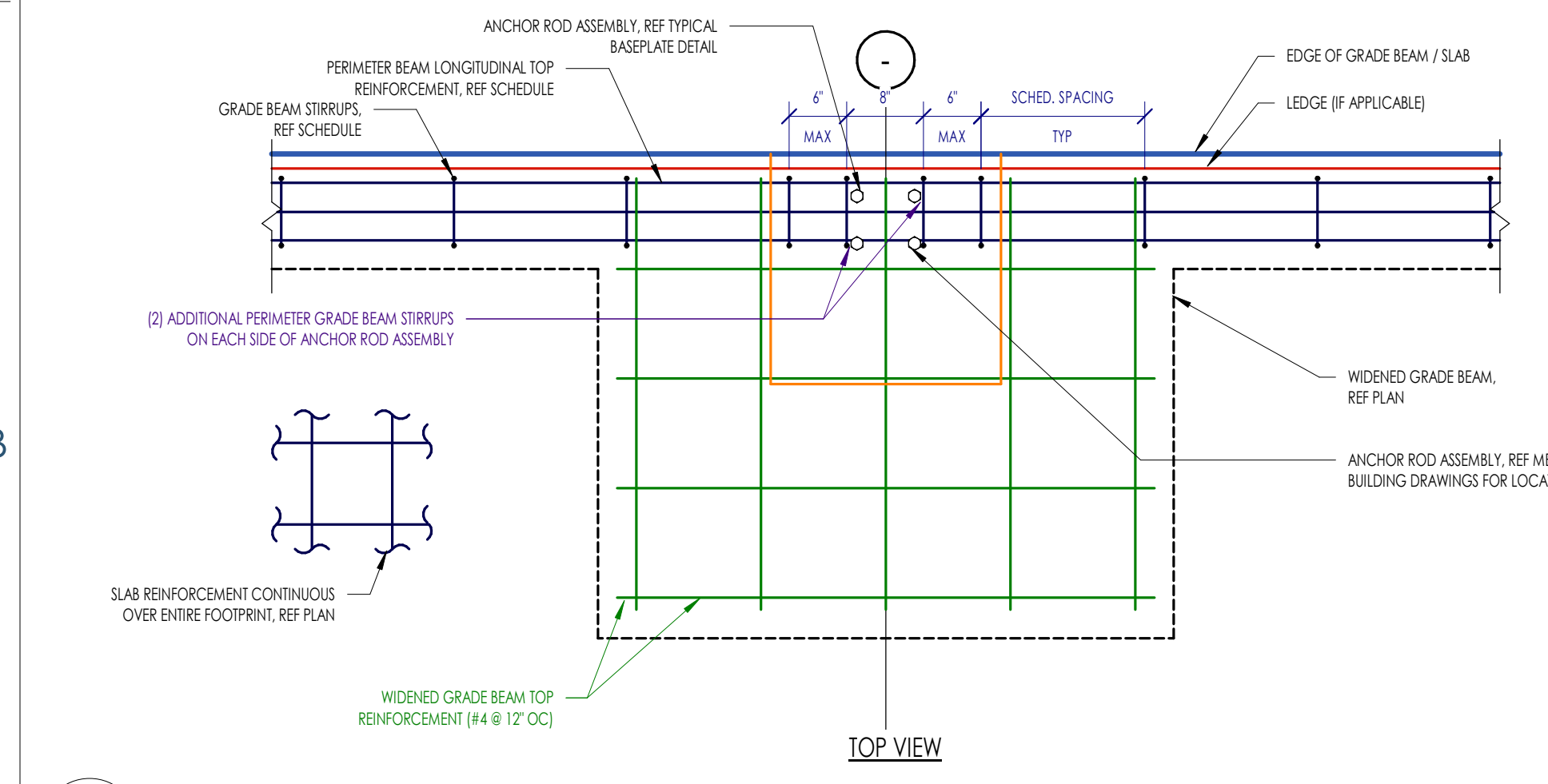
4C TYPICAL DROP TRANSITION IN GRADE BEAM TOP REINFORCEMENT AT SLAB LEAVE-OUT NOT TO SCALE



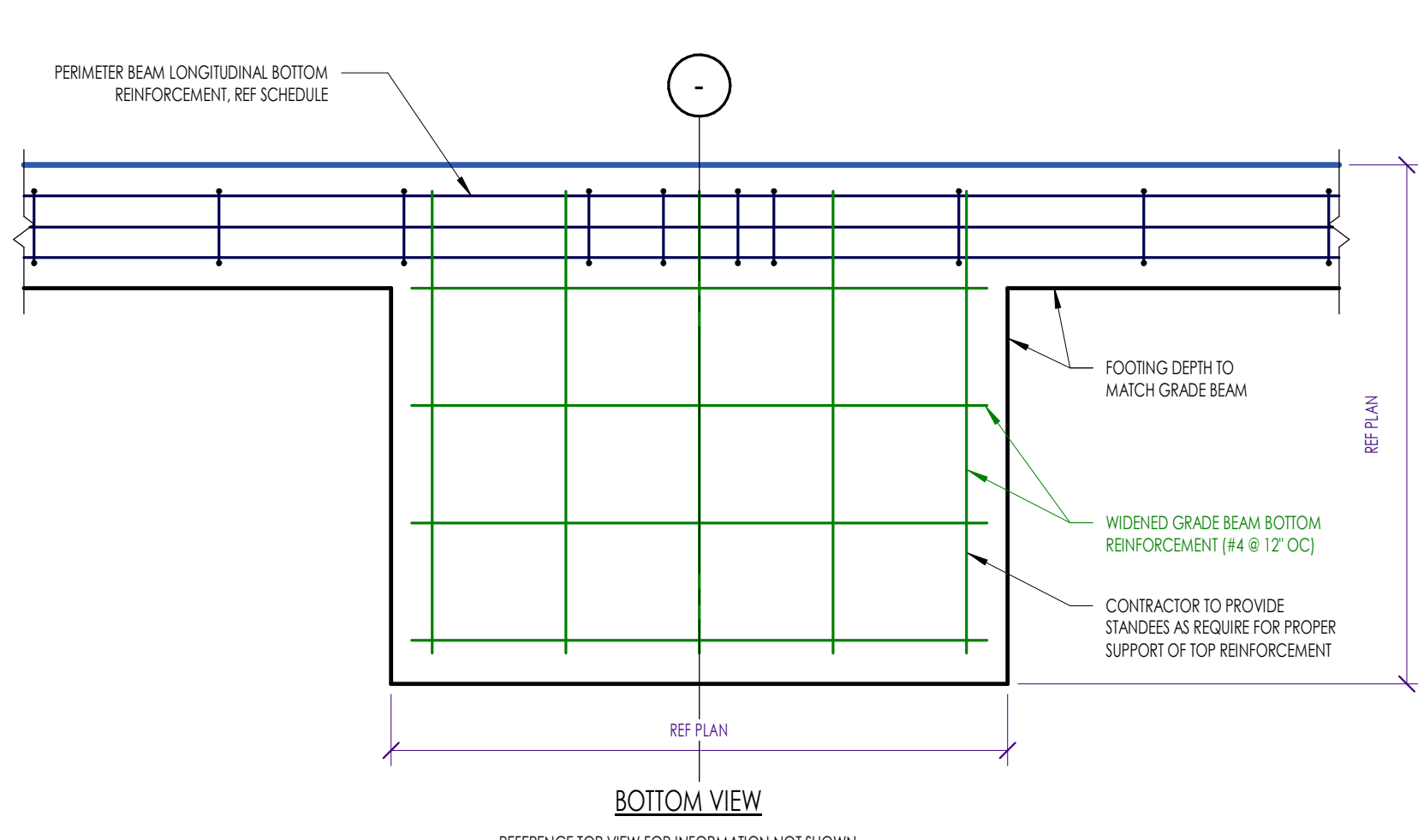
2C CSOG - TURN DOWN AT SLAB DROP NOT TO SCALE



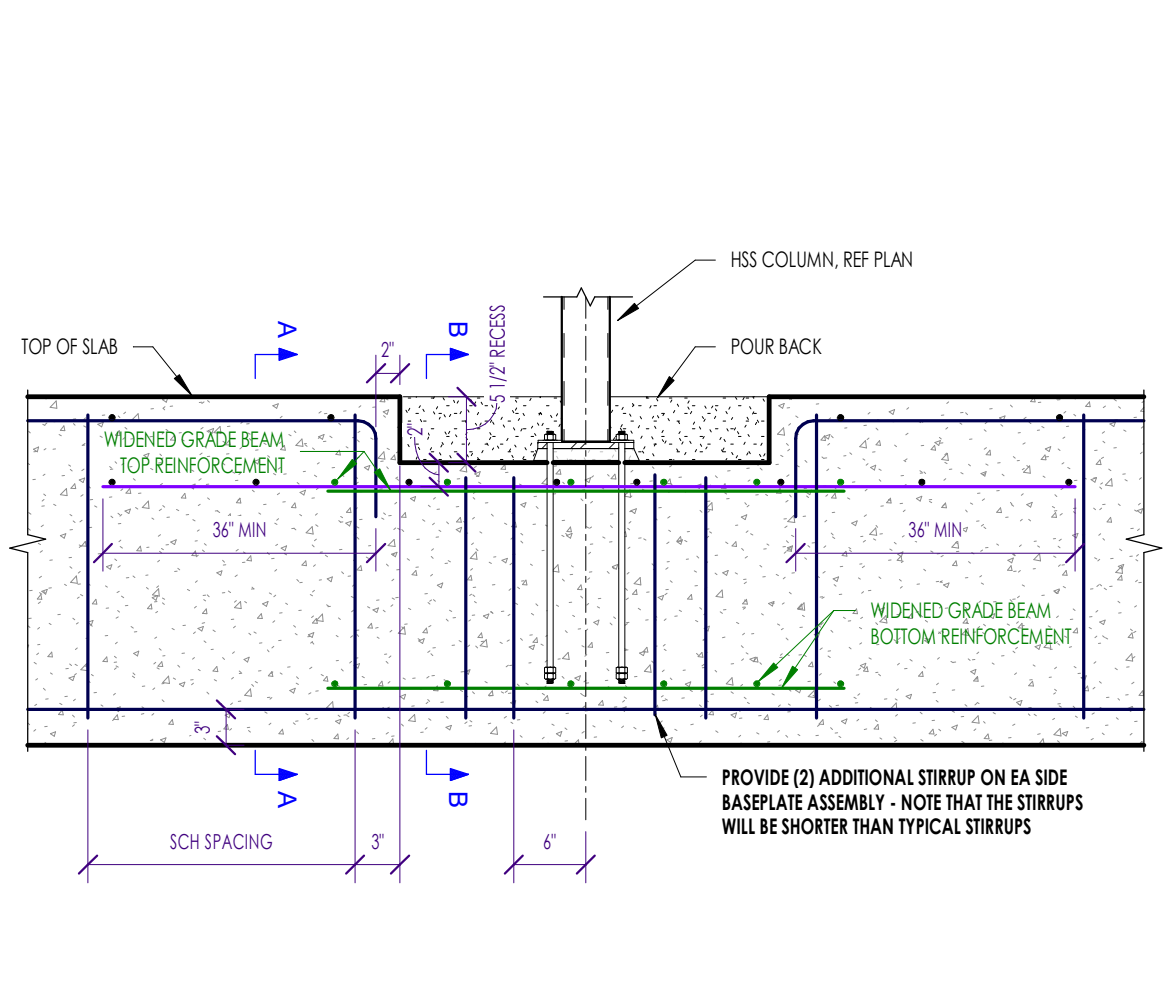
1C TYPICAL TURN DOWN BEAM NOT TO SCALE



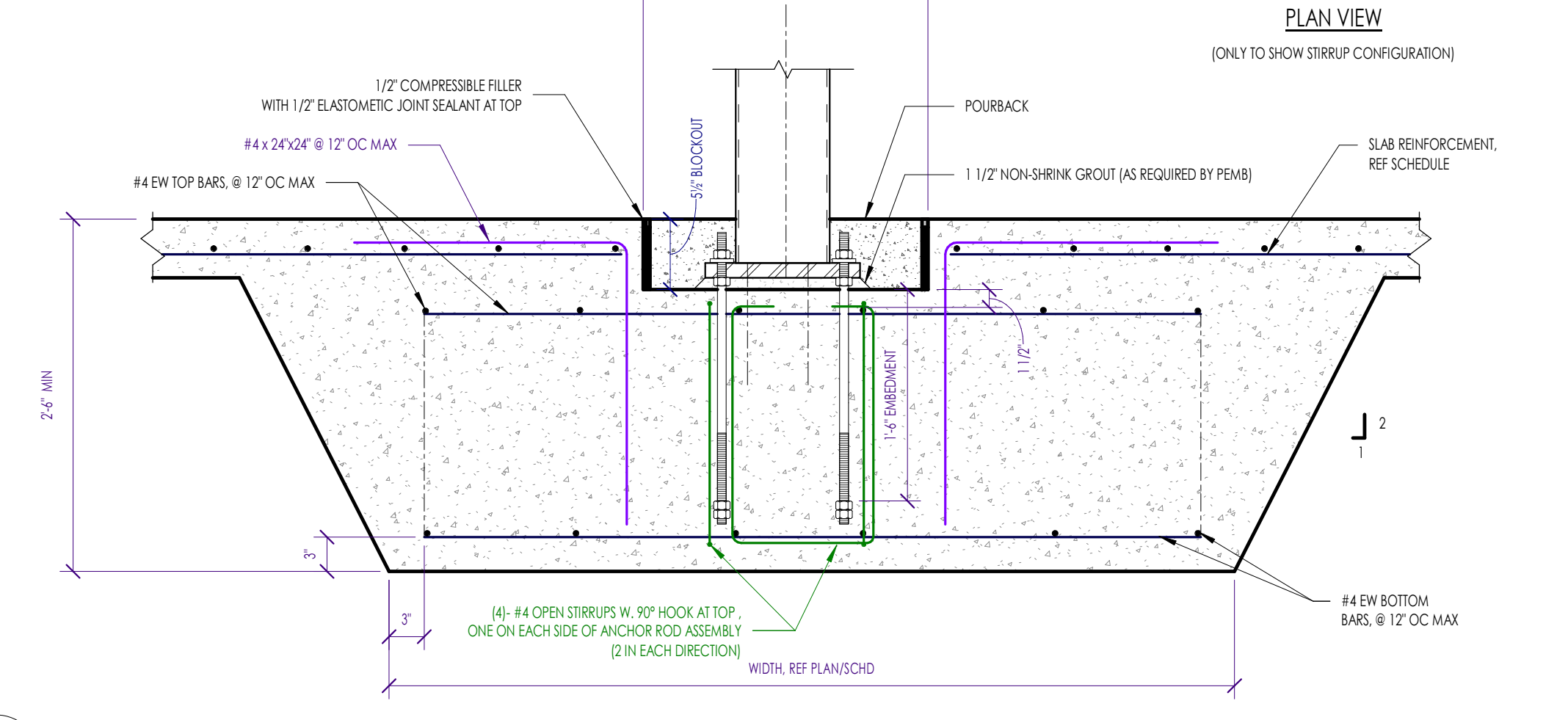
4B TYPICAL WIDENED FOOTING AT COLUMN - EMBEDDED BASE PLATE NOT TO SCALE



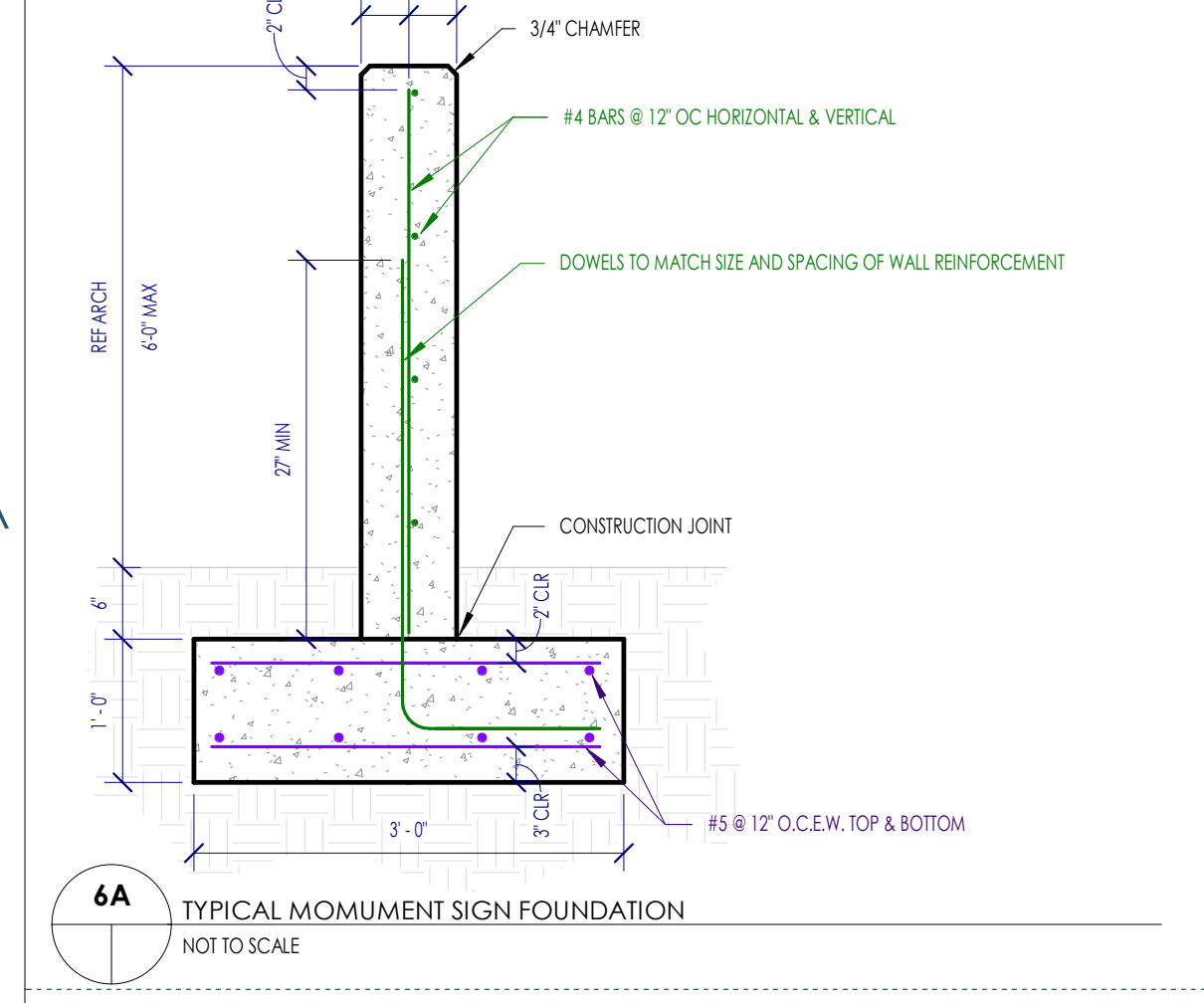
4C TYPICAL DROP TRANSITION IN GRADE BEAM BOTTOM REINFORCEMENT AT SLAB LEAVE-OUT NOT TO SCALE



2B TYPICAL SPREAD FOOTING AT INTERIOR COLUMN NOT TO SCALE



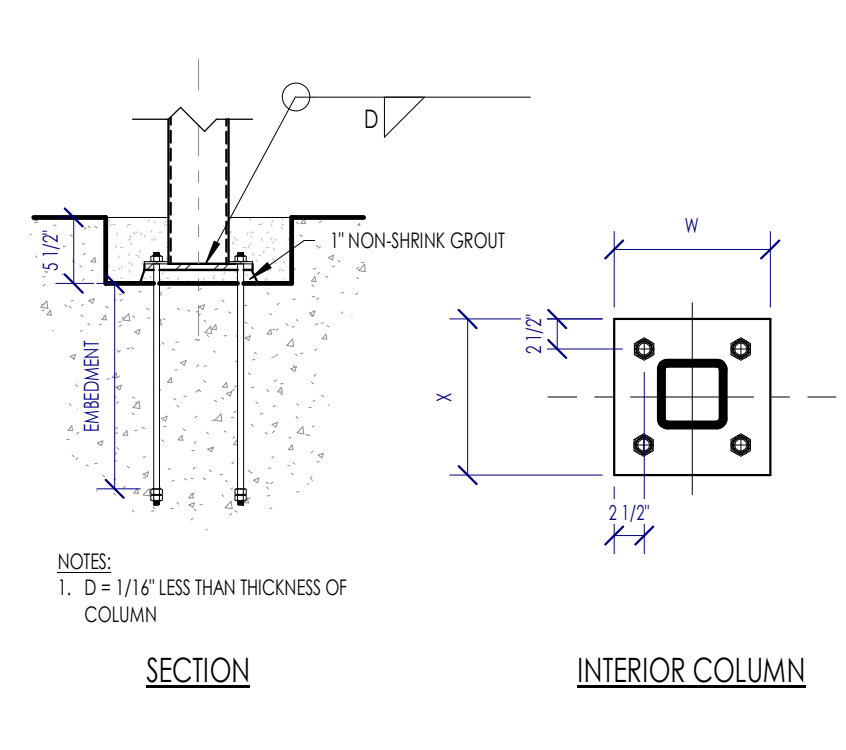
2B TYPICAL SPREAD FOOTING AT INTERIOR COLUMN NOT TO SCALE



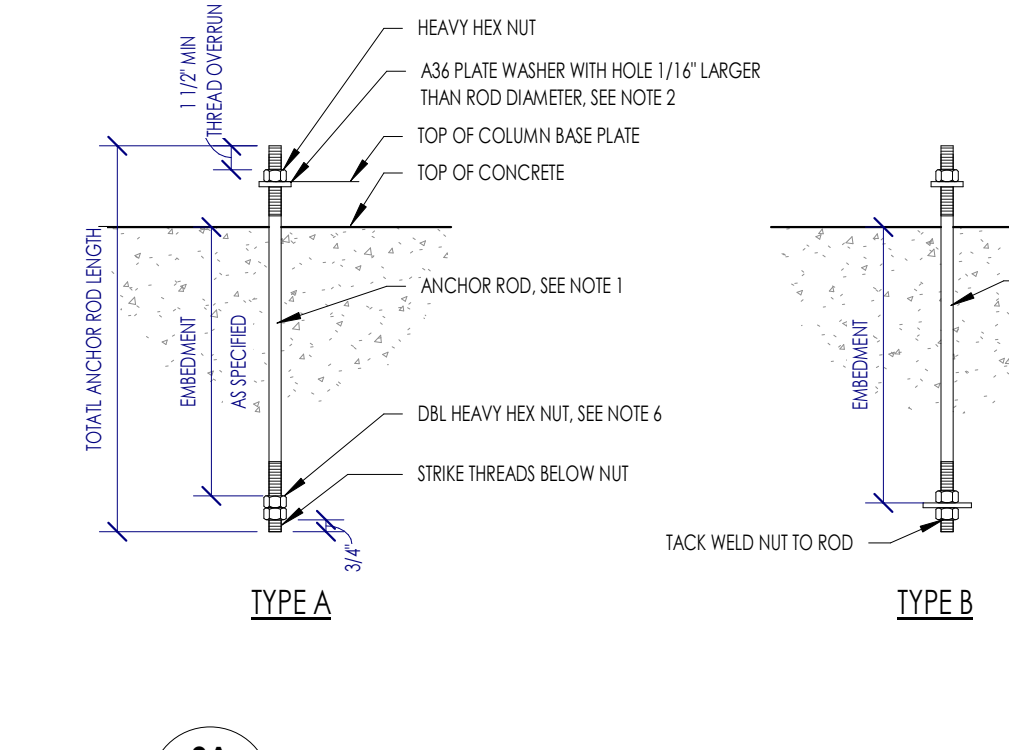
6A TYPICAL MOMENT SIGN FOUNDATION NOT TO SCALE

COLUMN	BASE PLATE DIMENSIONS			CONDITION	ANCHOR BOLTS		
	X	W	T		NO./TYPE	DIA.	EMBEDMENT
HSS36X36	11"	13"	13"	INTERIOR	4/A	1"	1'-0"
HSS36X36	14"	16"	16"	INTERIOR	4/A	1"	1'-0"

5A TYPICAL BASEPLATE DETAIL NOT TO SCALE



3A TYPICAL ANCHOR ROD NOT TO SCALE



3A TYPICAL ANCHOR ROD NOT TO SCALE

ANCHOR ROD DIAMETER	HOLE DIAMETER	SQUARE PLATE WASHER SIZE	PLATE WASHER THICKNESS	TYPE B ANCHOR PLATE
5/8"	1.316"	1 1/2"	1/4"	PL17X10-4
3/4"	1.516"	2"	1/4"	PL17X10-4
7/8"	1.916"	2 1/2"	5/16"	PL17X10-4
1"	1.1316"	3"	3/8"	PL19X10-5
1 1/2"	2.516"	3 1/2"	1/2"	PL19X10-5

3A TYPICAL ANCHOR ROD NOT TO SCALE

NOTES:

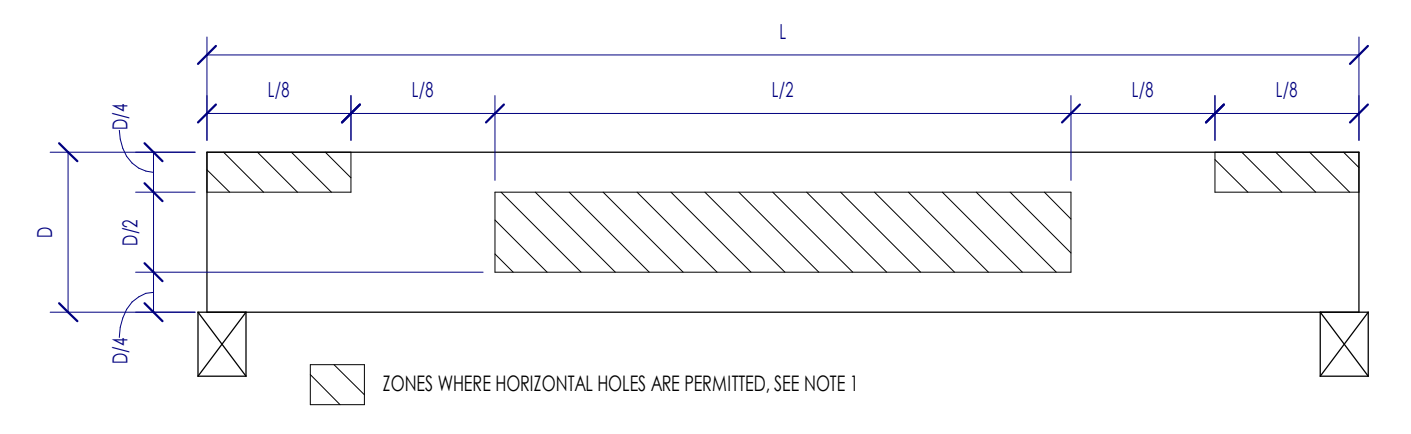
- ALL TYPE A ANCHOR RODS SHALL BE F1554 GRADE 36.
- ALL TYPE B ANCHOR RODS SHALL BE F1554 GRADE 55.1.
- PLATE WASHERS MUST BE WELDED TO THE BASE PLATE WITH MINIMUM 3/16" FLLET WELD ALL-AROUND.
- EMBEDMENT DEPTH ARE PRELIMINARY. FINAL EMBEDMENT TO BE PROVIDED AFTER REVIEW OF METAL BUILDING REACTIONS.
- ALL ANCHOR ROD HOLES SHALL ADHERE TO AISC DESIGN GUIDE 01, TABLE 2.3.
- THE DOUBLE NUT MAY BE OMITTED IF THE NUT IS TACK WELDED TO THE ROD.

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**TYPICAL FASTENING SCHEDULE**

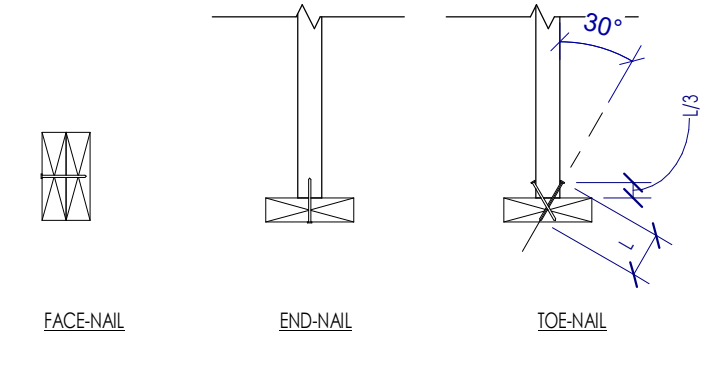
CONNECTION ID	CONNECTION TYPE	FASTENING	FASTENING ORIENTATION
1	JOIST TO BIL OR GIRDER	(2) - 0.131"Ø X 3"	TOENAIL
2	SOLE PLATE TO JOIST OR BLOCKING	0.148"Ø X 3 1/2" NAILS @ 12" OC NAILS	FACE NAIL
3	TOP PLATE TO STUD	(3) - 0.131"Ø X 3" NAILS	END NAIL
4	STUD TO SOLE PLATE - OPTION 1	(2) - 1/4" COMMON   (2) - 0.131"Ø X 3" NAILS	END NAIL
5	STUD TO SOLE PLATE - OPTION 2	(4) 0.131"Ø X 3" NAILS	TOENAIL
6	DOUBLE/MULTIPLE STUDS	REFERENCE DETAIL <b>3A/3A.1</b>	FACE NAIL
7	DOUBLE TOP PLATES	0.131"Ø X 3" NAILS @ 12" OC	FACE NAIL
8	DOUBLE TOP PLATE SPICE	REFERENCE DETAIL <b>3A/3A.1</b>	FACE NAIL
9	BLOCKING BETWEEN JOISTS/RAFTERS TO TOP PLATE	(3) - 0.131"Ø X 3" NAILS	TOENAIL
10	RIM JOIST TO TOP PLATE	0.131"Ø X 3" NAILS @ 6" OC	TOENAIL
11	CeILING JOIST TO TOP PLATE	(3) - 0.131"Ø X 3" NAILS	TOENAIL
12	CeILING JOIST LAP OVER PARTITION	(4) - 0.131"Ø X 3" NAILS	FACE NAIL
13	CeILING JOIST TO PARALLEL RAFTERS	(4) - 0.131"Ø X 3" NAILS	FACE NAIL
14	RAFTER TO TOP PLATE	(3) - 0.131"Ø X 3" NAILS	TOENAIL
15	BUILT-UP CORNER STUDS	0.131"Ø X 3" NAILS @ 18" OC	FACE NAIL
16	BUILT-UP BEAMS	REFERENCE DETAIL <b>2A/2A.0</b>	FACE NAIL
17	COLLAR TIE TO RAFTER	(4) - 0.131"Ø X 3" NAILS	FACE NAIL
18	JACK RAFTER TO HP	(4) - 0.131"Ø X 3" NAILS	TOENAIL
19	RAFTER TO RIDGE BOARD/BEAM	(3) - 0.131"Ø X 3" NAILS	TOENAIL
20	BLOCKING BT STUDS	(3) - 0.131"Ø X 3" NAILS EACH SIDE	TOENAIL

**6D** TYPICAL WOOD FASTENING SCHEDULE  
NOT TO SCALE

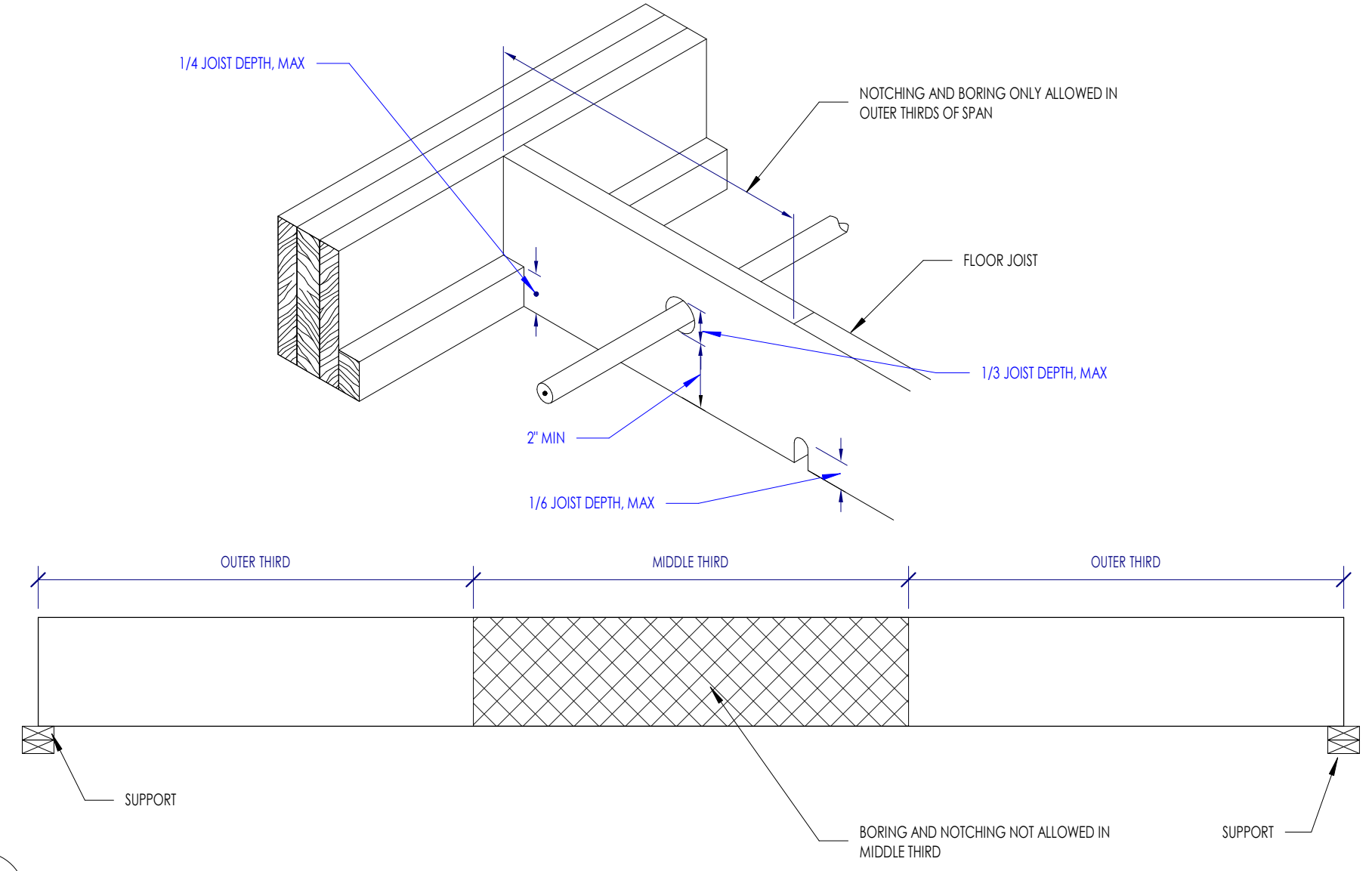


- NOTES:**
- HOLE SIZE: THE HOLE DIAMETER SHALL NOT EXCEED 1/4" OR D/10, WHICHEVER IS SMALLER.
  - SPACING: FOR LARGER HOLE DIAMETERS OR FOR HOLES OUTSIDE OF THE PERMITTED ZONES, WRITTEN PERMISSION MUST BE OBTAINED FROM THE E.O.R.
  - LIMITATIONS: THE ABOVE CRITERIA ONLY APPLY TO SIMPLY SUPPORTED, UNIFORMLY LOADED GLUE LAMINATED BEAMS. FOR BEAMS THAT ARE EITHER CONTINUOUS ACROSS MULTIPLE SPANS OR THAT ARE SUPPORTING NON-UNIFORM LOADS, WRITTEN PERMISSION MUST BE OBTAINED FROM THE E.O.R.

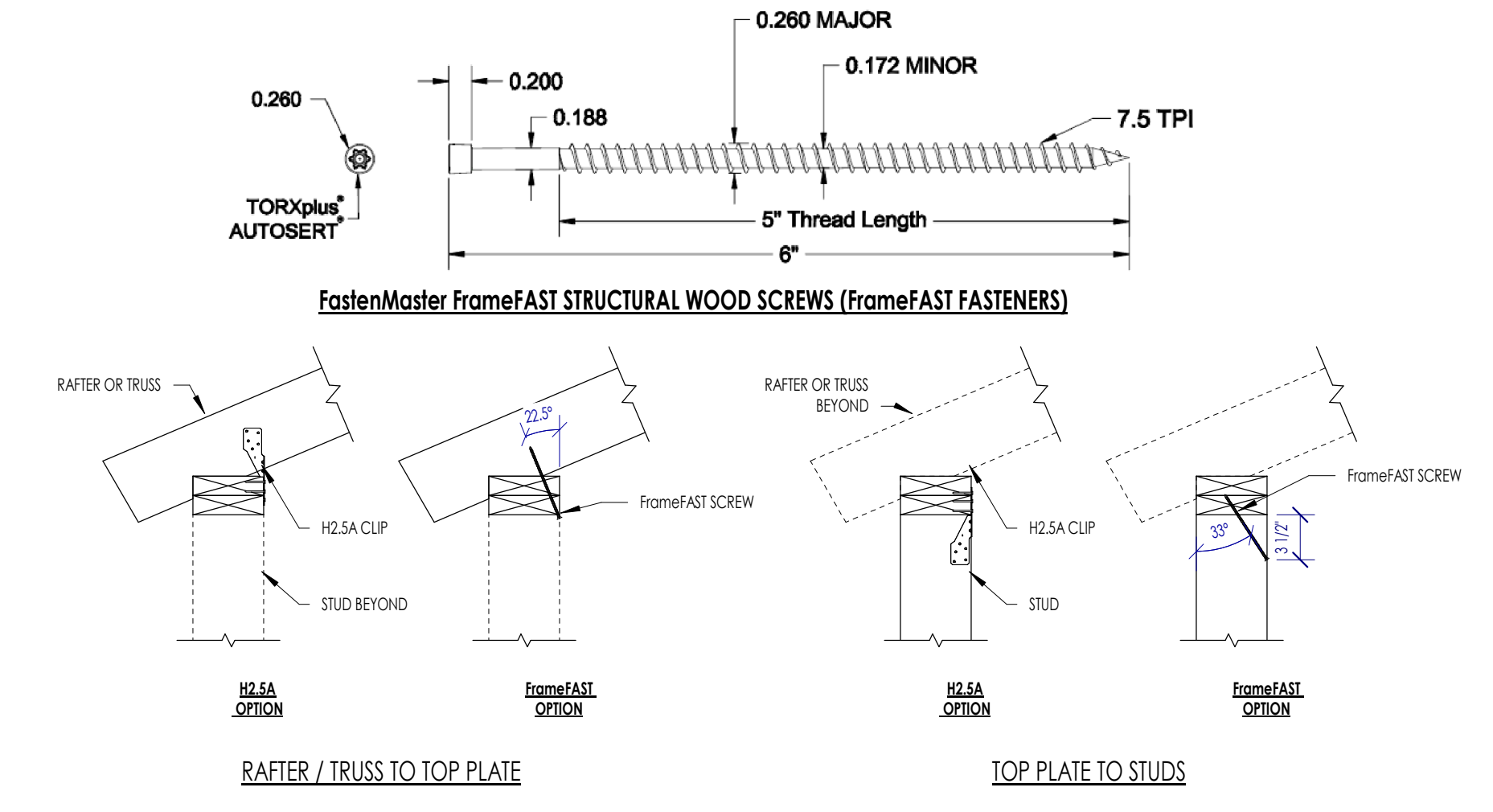
**4E** ALLOWABLE HORIZONTAL HOLE LOCATIONS IN GLUE LAMINATED TIMBER BEAMS  
NOT TO SCALE



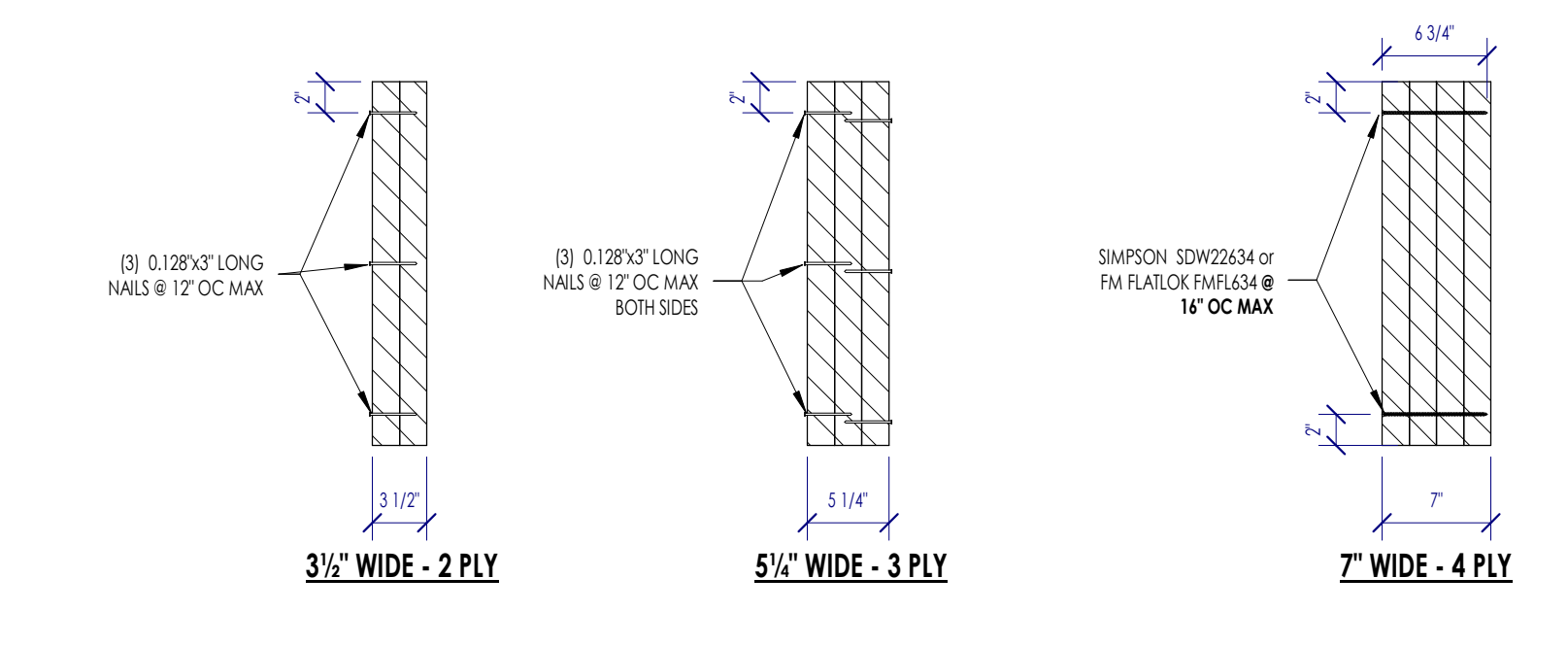
**4D** TYPICAL NAILING CONFIGURATIONS  
NOT TO SCALE



**6C** ALLOWABLE NOTCHING AND BORING OF FLOOR JOISTS  
NOT TO SCALE



**4C** ALLOWABLE SUBSTITUTION OF H2-5A CLIPS WITH FrameFAST SCREWS - UPLIFT LOAD PATH  
NOT TO SCALE



**2C** TYPICAL LVL MULTIPLE PLY FASTENING REQUIREMENTS  
NOT TO SCALE

**FASTENER SCHEDULE - TO BEAM TOP FLANGE**

L (ft)	PAF FASTENER	BOLT / ROD*
≤ 0.35	X-4 4\"/>	

**FASTENER SCHEDULE - TO BEAM WEB / BOTTOM FLANGE**

L <sub>c</sub> (ft)	PAF FASTENER	BOLT / ROD*
≤ 0.35	(3) - X-4 4\"/>	

**NAILER SCHEDULE - TO BEAM FLANGE**

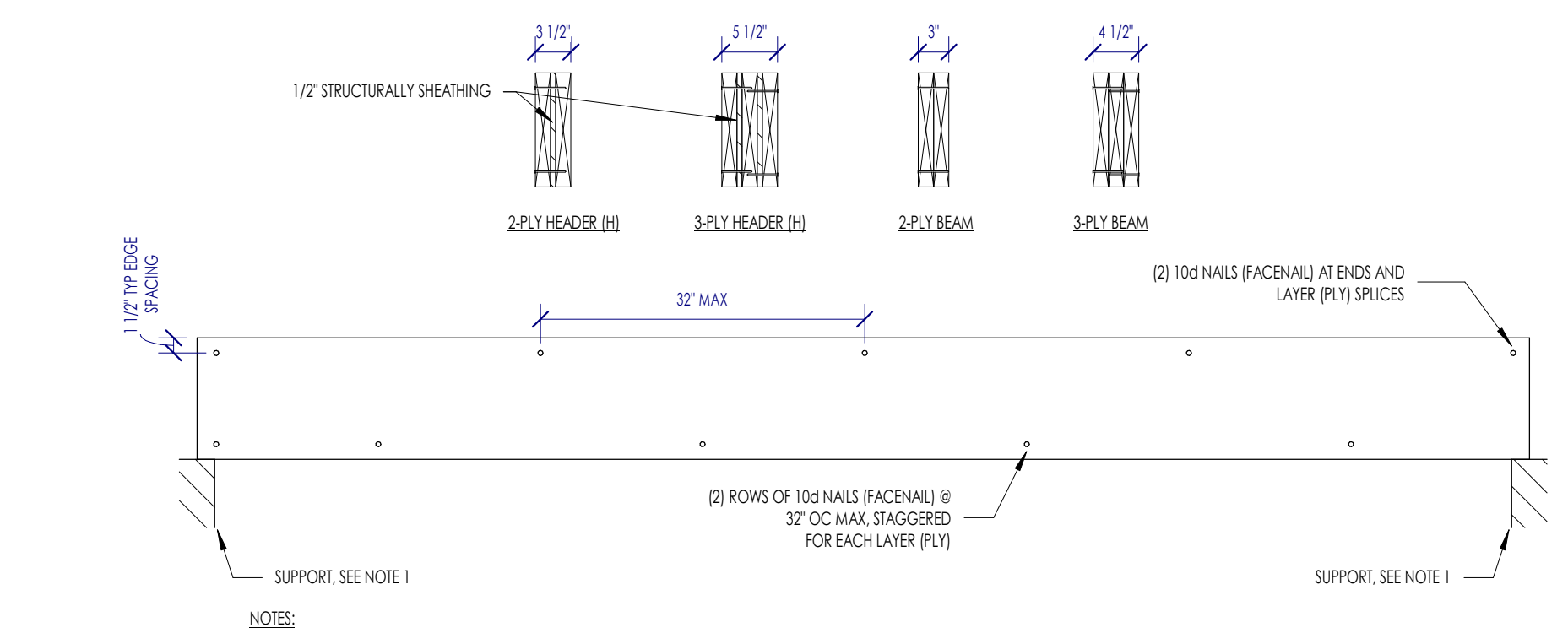
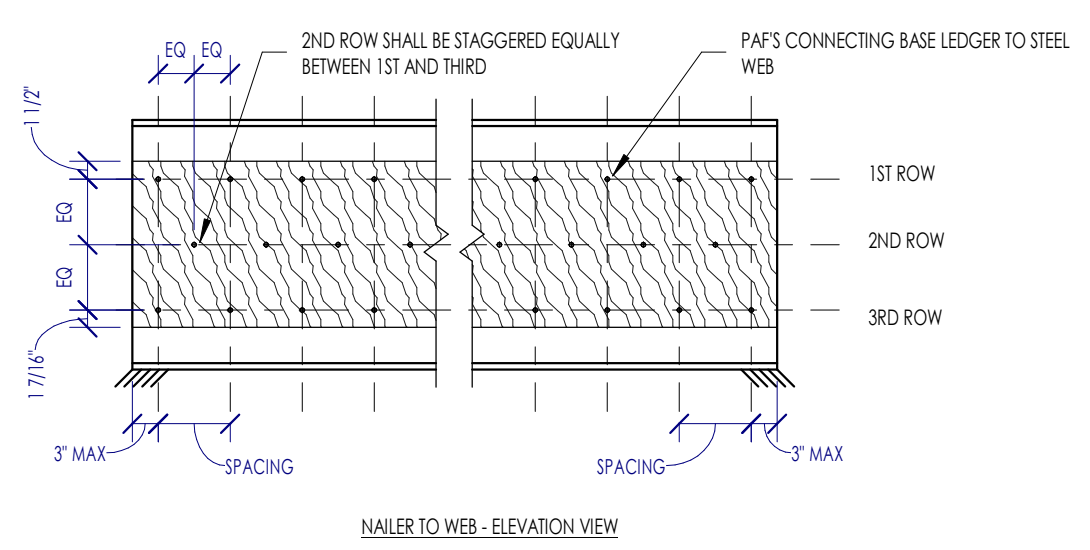
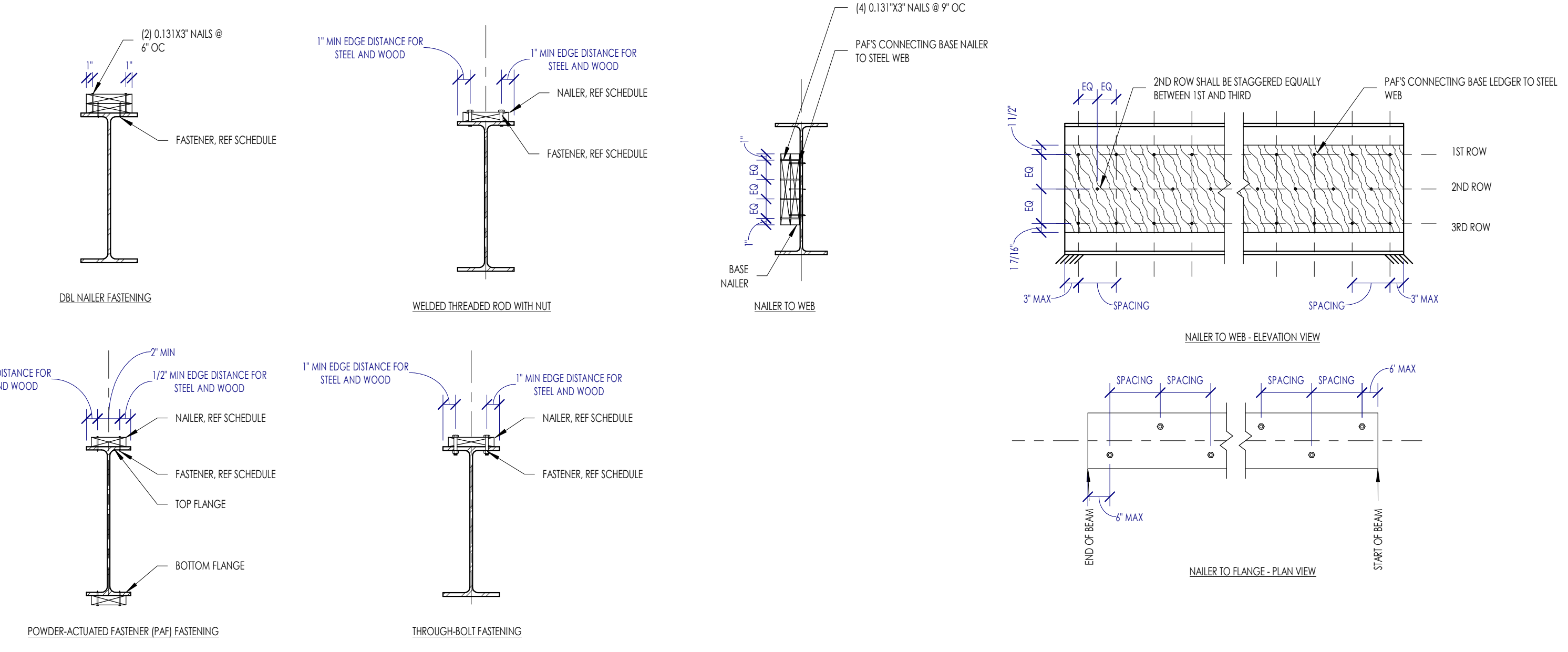
b (ft)	NAILER SIZE
≤ 5.5	2x4
5.5 < b ≤ 7.25	2x6
1 > 7.25	2x8

**NAILER SCHEDULE - TO BEAM WEB**

d (ft)	NAILER SIZE
≤ 5.5	2x4
5 < d ≤ 6.75	2x6
6.75 < d ≤ 8.75	2x8
8.75 < d ≤ 10.75	2x10
10.75 < d ≤ 15	(2) - 2x8
15 < d ≤ 19	(2) - 2x10
19 < d ≤ 23	(2) - 2x12
d > 23	(3) - 2x8

- NOTES:**
- ALL FASTENERS SHALL BE STAGGERED.
  - FASTENER DESCRIPTIONS: ALL FASTENERS ARE POWDER-ACTUATED FASTENERS MPFD BY HULT, INC.
    - A. 40147
    - Q. UNIVERSAL KNURELED SHANK FASTENER WITH A SHANK DIAMETER OF 0.157" AND A SHANK LENGTH OF 47 mm (1.85").
    - B. 03147
    - O. HEAVY DUTY SMOOTH SHANK FASTENER WITH A SHANK DIAMETER OF 0.177" AND A SHANK LENGTH OF 47 mm (1.85").
  - FASTENER INSTALLATION SHALL FOLLOW ALL SPECIFICATIONS PER THE MPF.
  - THROUGH BOLTS SHALL BE GALVANNEED ASTM A507 BOLTS. THROUGH BOLTS SHALL BE GALVANNEED ASTM F1554 GR.36.

**6A** WOOD NAILER TO TOP OF STRUCTURAL STEEL  
NOT TO SCALE



- NOTES:**
- UNLESS NOTED OTHERWISE ON PLAN, REFER TO THE FOLLOWING DETAILS FOR THE SUPPORT FRAMING:
    - A. SUPPORT FOR HEADERS IN EXTERIOR WALLS **4C/3A.1**
    - B. SUPPORT FOR HEADERS IN INTERIOR WALLS **3B/3A.1**
    - C. SUPPORT FOR BEAMS & GIRDERS SUPPORTED BY WALL - REFERENCE BEAM SCHEDULE

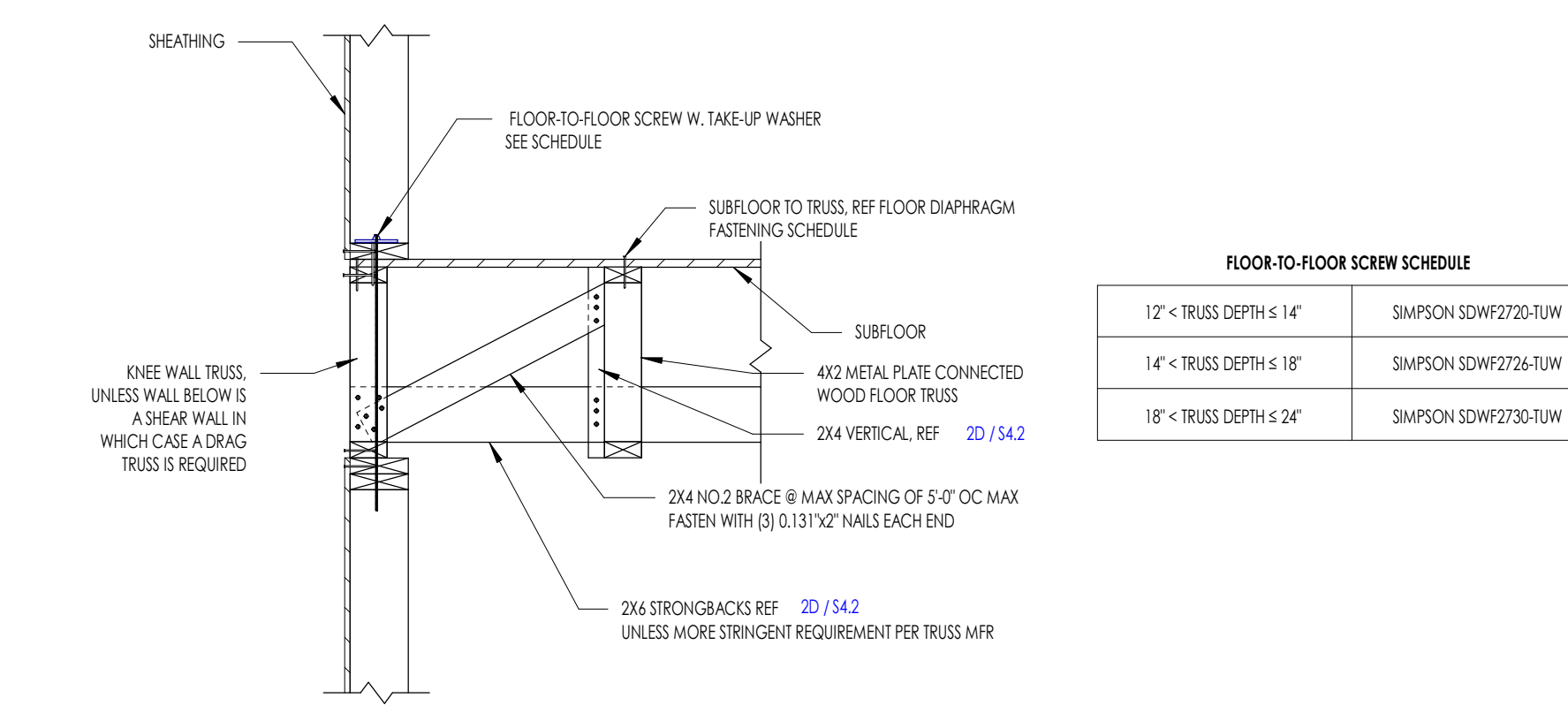
**2A** TYPICAL NAILING BUILT-UP BEAMS, GIRDERS & HEADERS  
NOT TO SCALE

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**openingdesign**  
Architect: OpeningDesign  
17 S Fairchild | FL 7  
Madison, WI 53703  
ryan@openingdesign.com | 773.425.6456

Date	Description

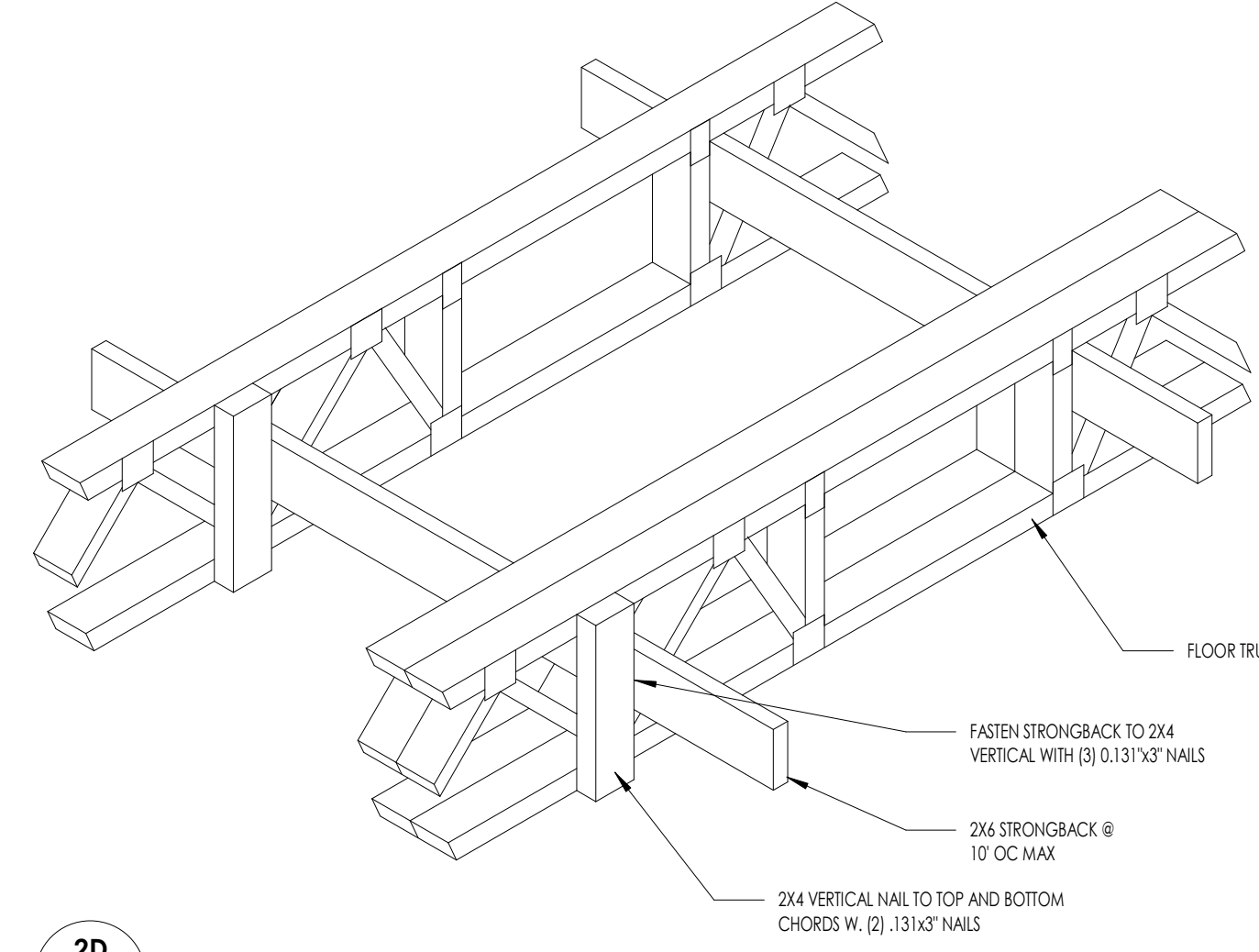




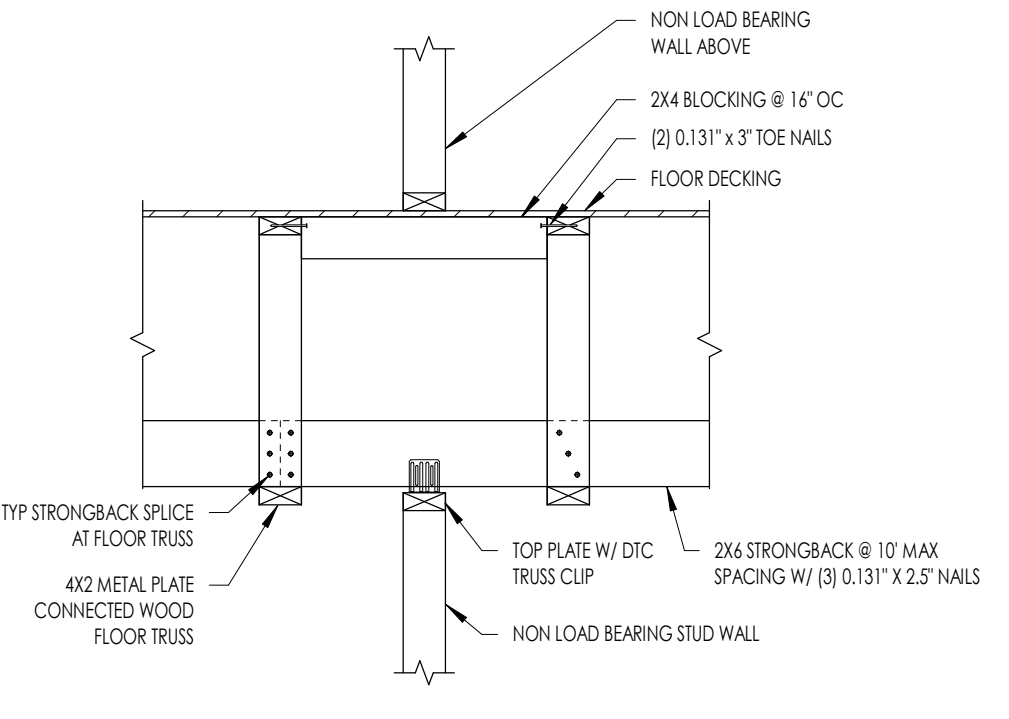
FLOOR-TO-FLOOR SCREW SCHEDULE

12' < TRUSS DEPTH ≤ 14'	SIMPSON IDWF2720-TJM
14' < TRUSS DEPTH ≤ 18'	SIMPSON IDWF2724-TJM
18' < TRUSS DEPTH ≤ 24'	SIMPSON IDWF2730-TJM

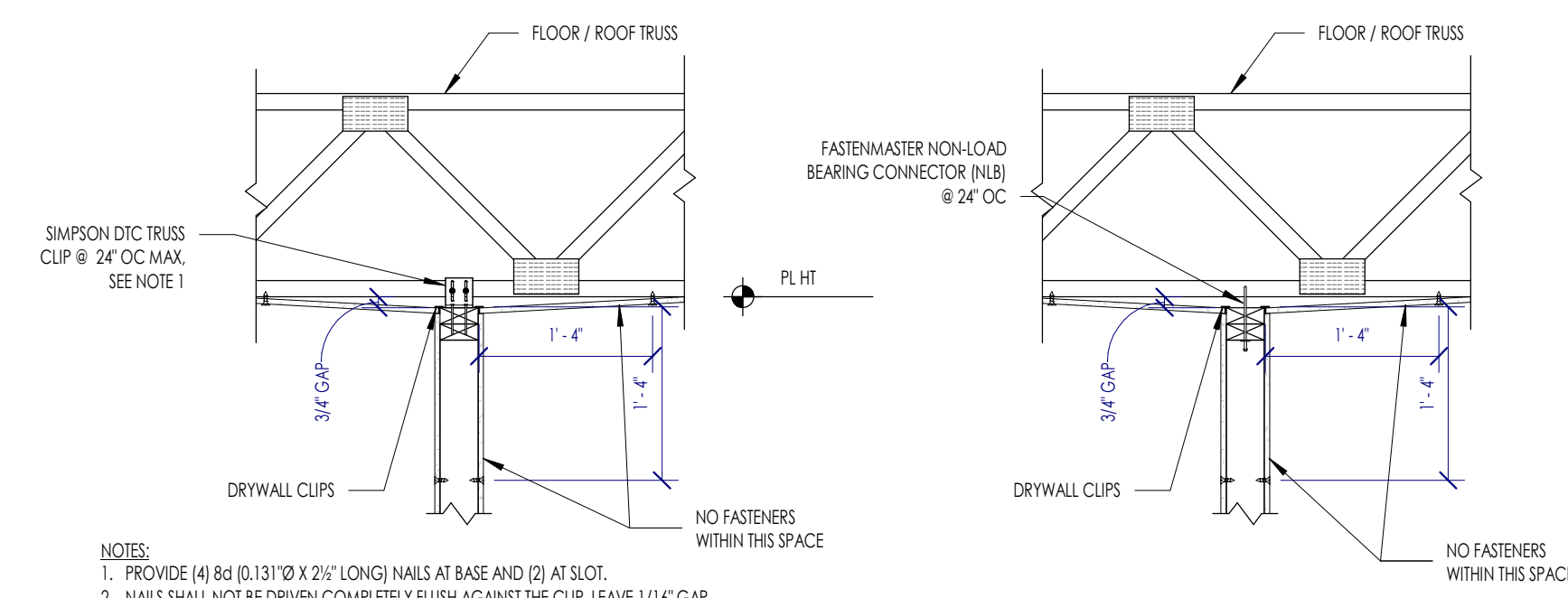
4D TYPICAL FLOOR TRUSS PARALLEL TO EXTERIOR WALL - MULTI-STORY  
NOT TO SCALE



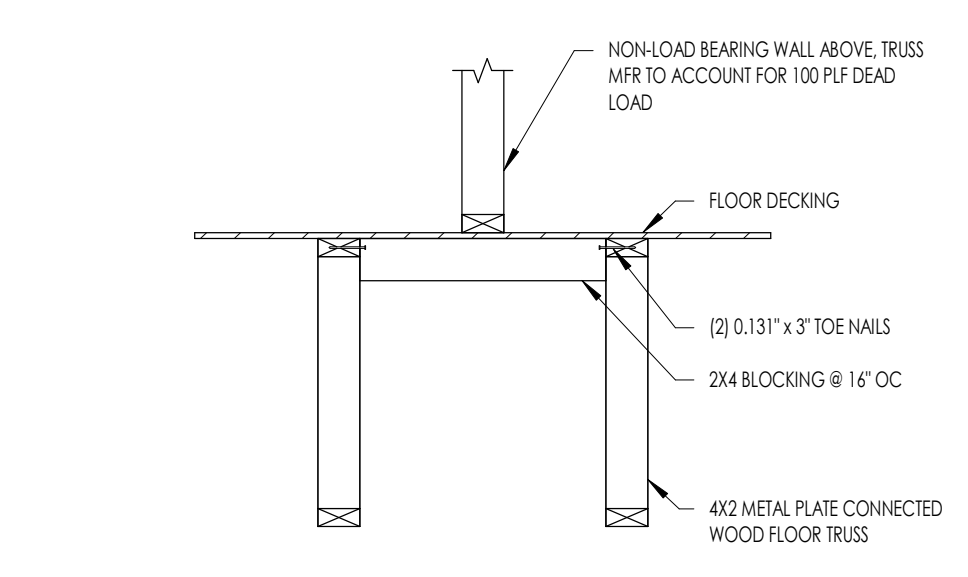
2D TYPICAL TRUSS STRONGBACK  
NOT TO SCALE



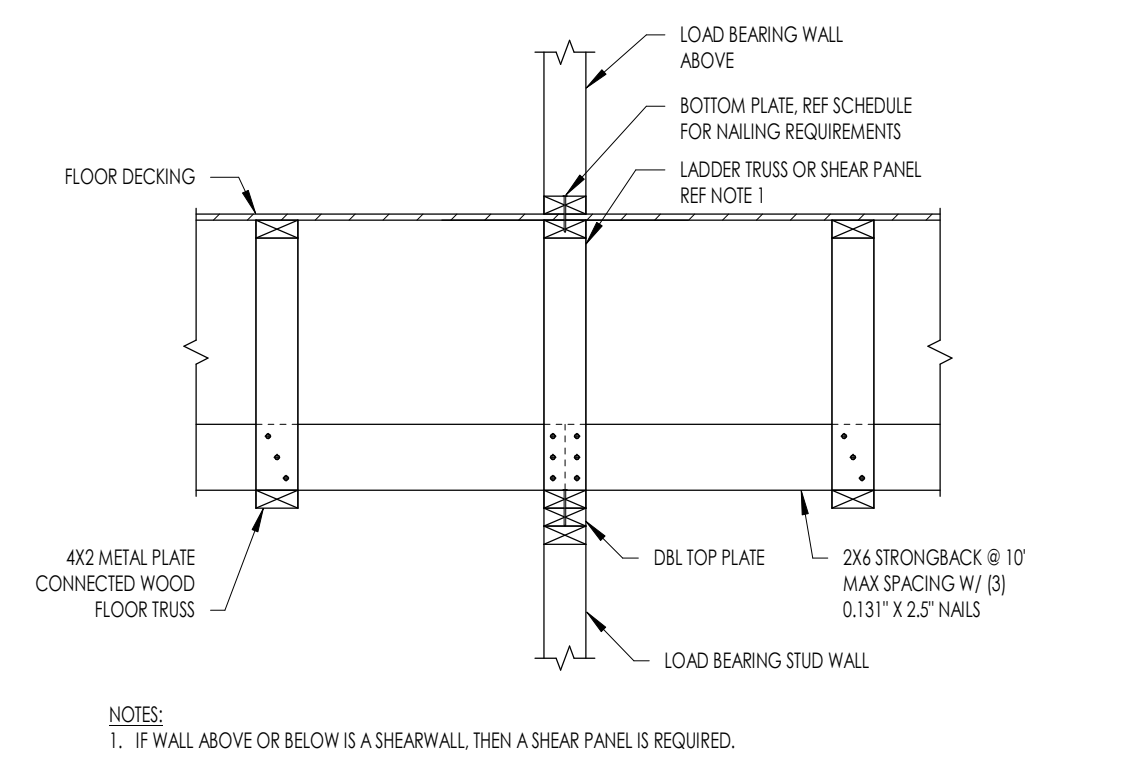
3C TYPICAL NON-LOAD BEARING WALL PARALLEL TO FLOOR TRUSSES  
NOT TO SCALE



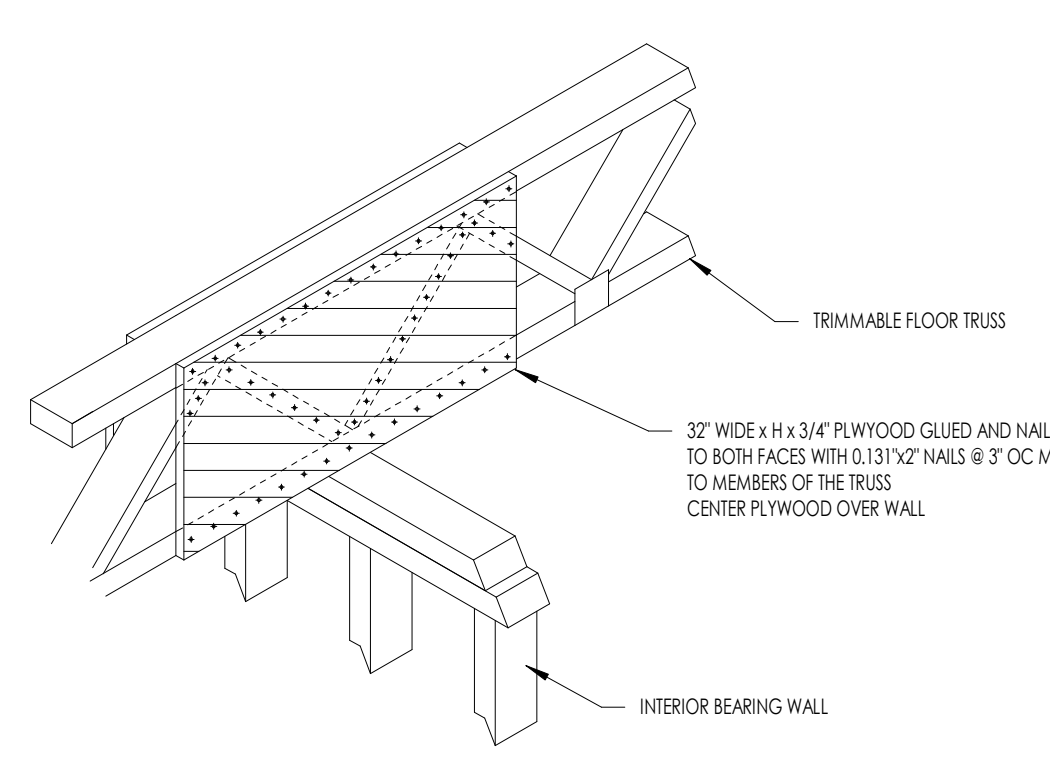
4C TYPICAL NON-LOAD BEARING WALL ATTACHMENT TO PERPENDICULAR FLOOR TRUSS  
NOT TO SCALE



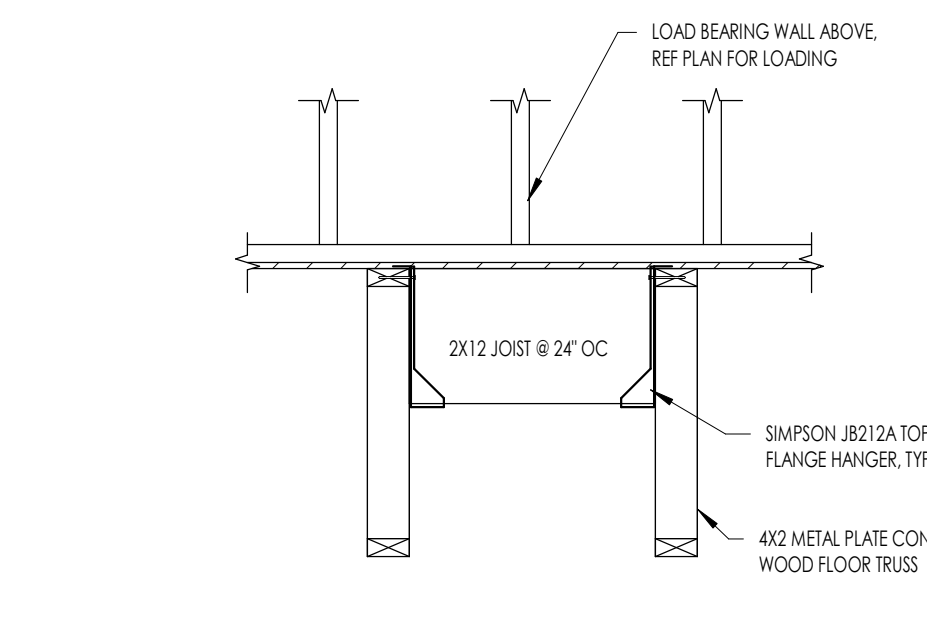
2C TYPICAL NON-LOAD BEARING WALL PARALLEL TO FLOOR TRUSS  
NOT TO SCALE



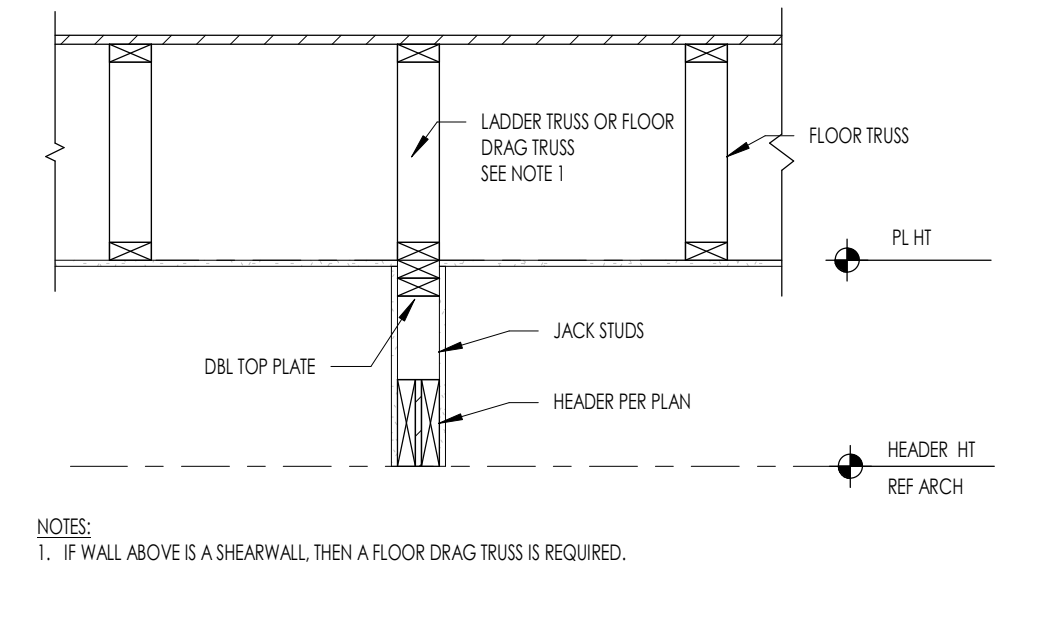
1C TYPICAL LOAD BEARING WALL PARALLEL TO FLOOR TRUSSES  
NOT TO SCALE



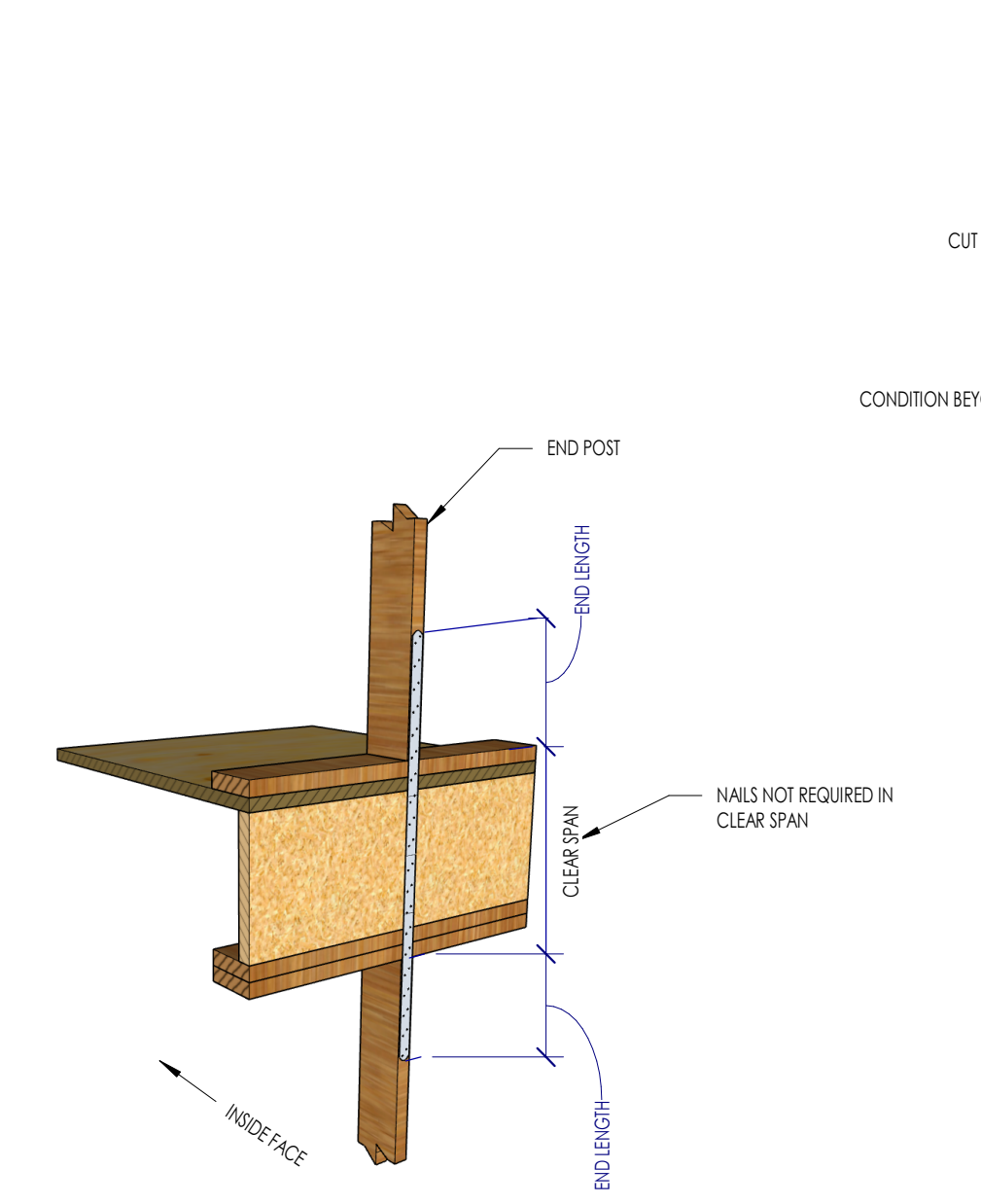
1B TYPICAL TRIMMABLE TRUSS STIFFENING AT INTERIOR SUPPORT  
NOT TO SCALE



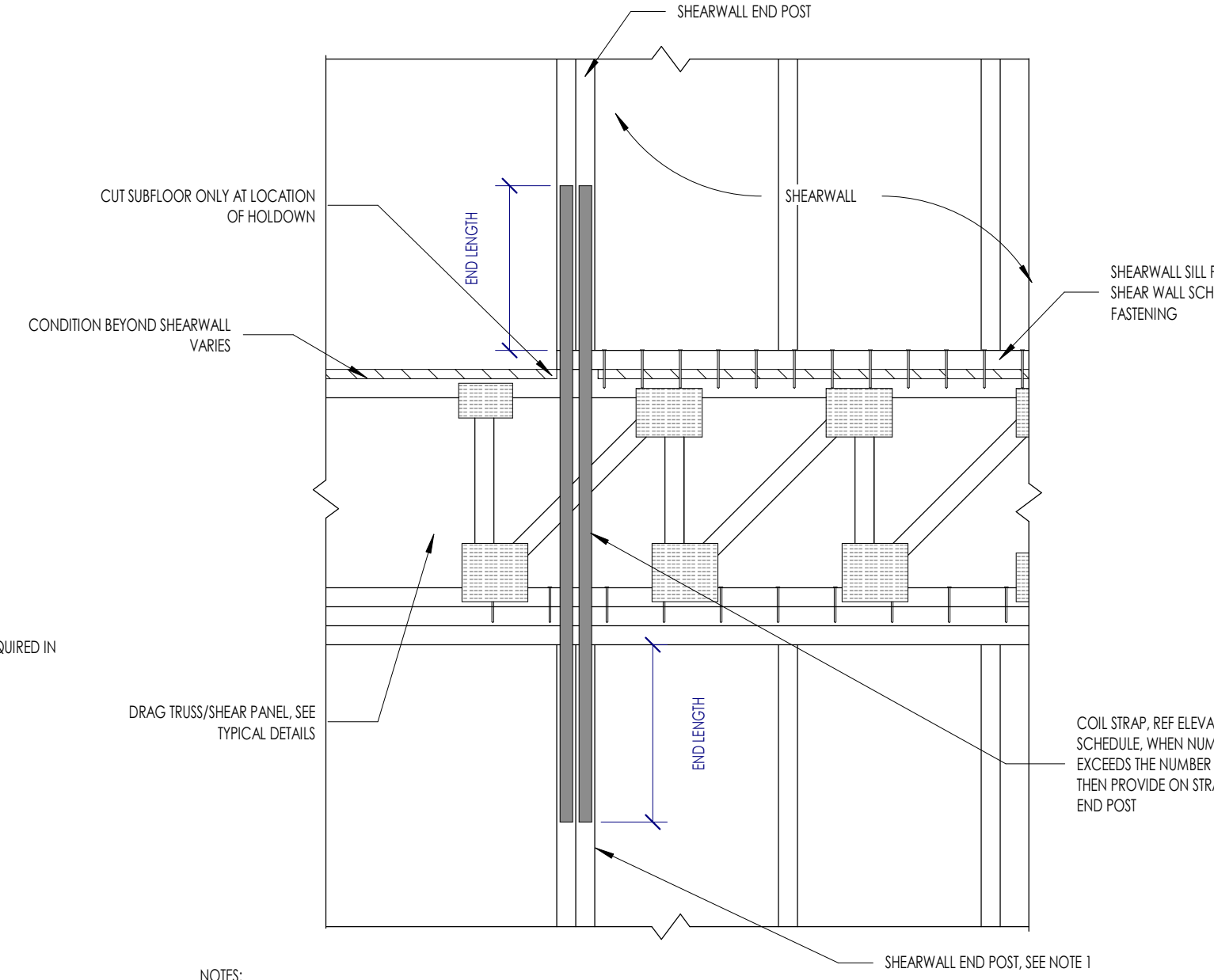
3B TYPICAL LOAD BEARING WALL PERP. TO FLOOR TRUSS  
NOT TO SCALE



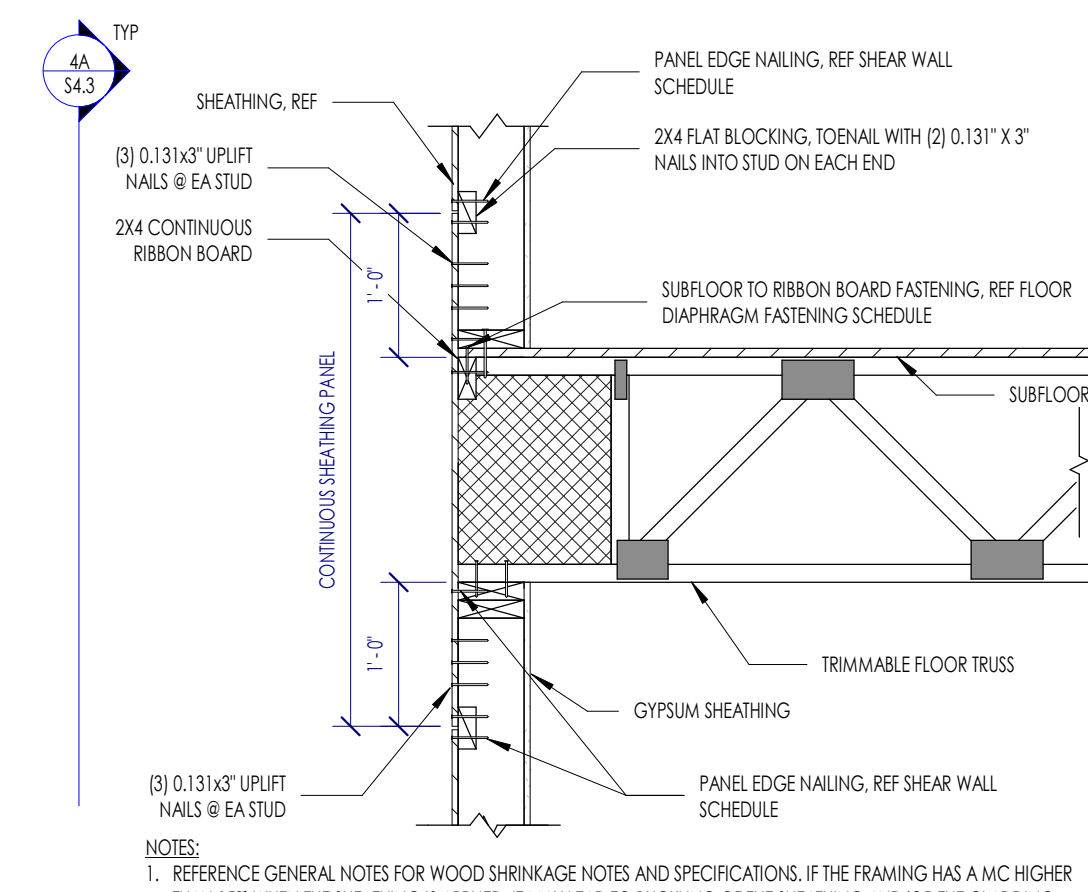
2B TYPICAL LOAD BEARING HEADER PARALLEL TO FLOOR TRUSSES  
NOT TO SCALE



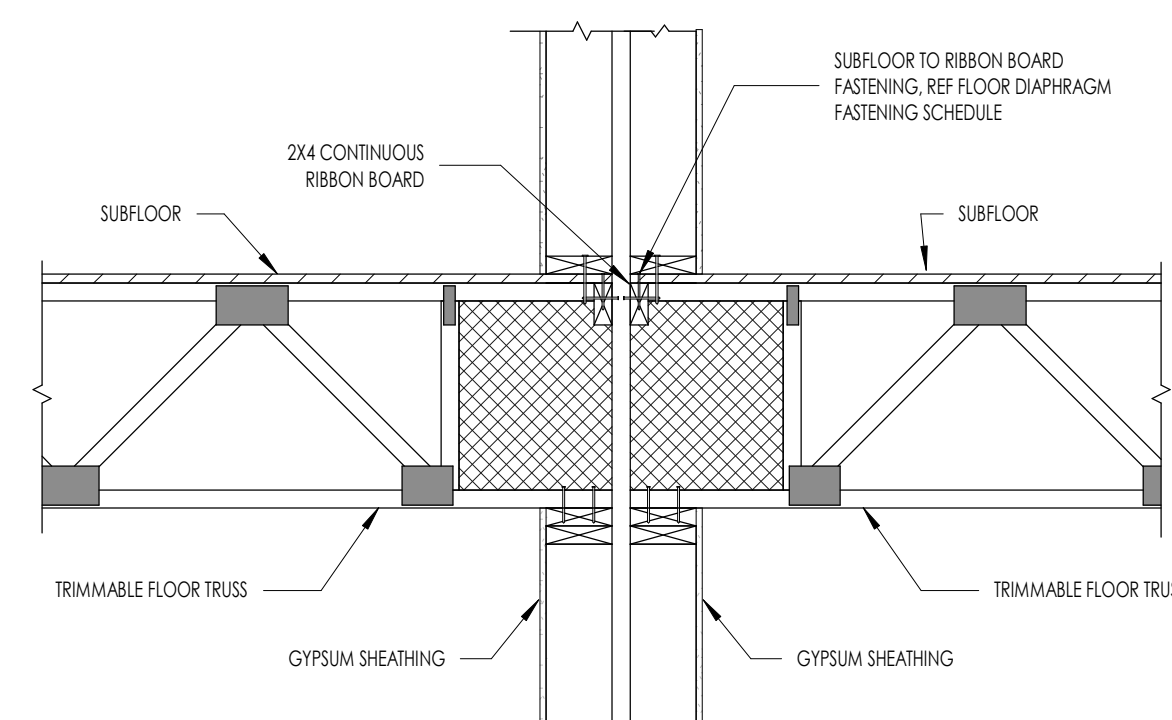
6A TYPICAL SHEARWALL HOLDDOWN AT ELEVATED FLOOR  
NOT TO SCALE



6B TYPICAL SHEARWALL HOLDDOWN AT INTERIOR SHEAR WALL  
NOT TO SCALE



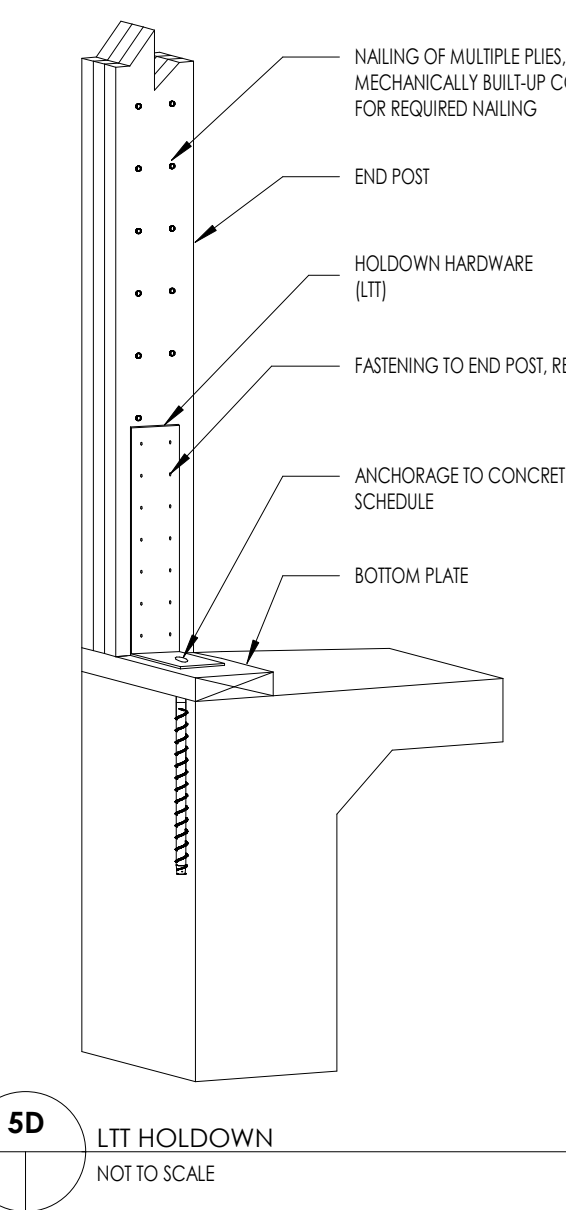
3A 061760 FLOOR - TRIMMABLE TRUSS BOTTOM CHORD BEARING ON EXTERIOR WALL  
NOT TO SCALE



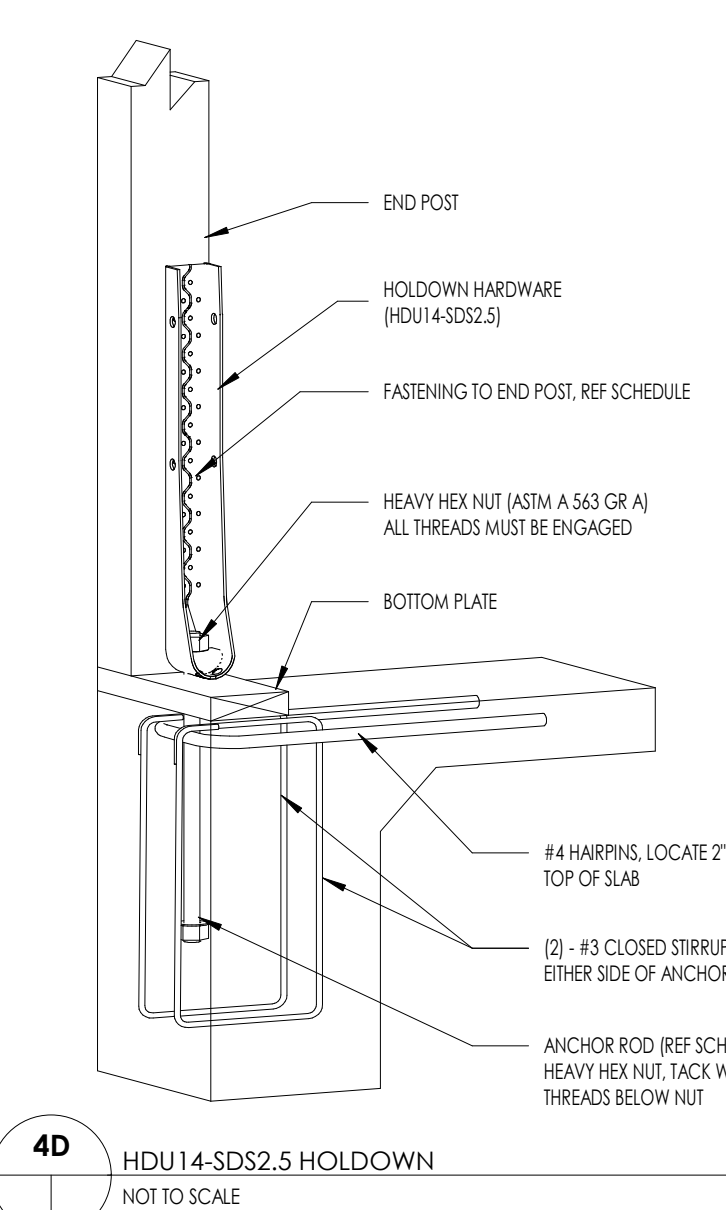
2A TYPICAL INTERIOR BOTTOM CHORD BEARING AT PARTY WALL  
NOT TO SCALE

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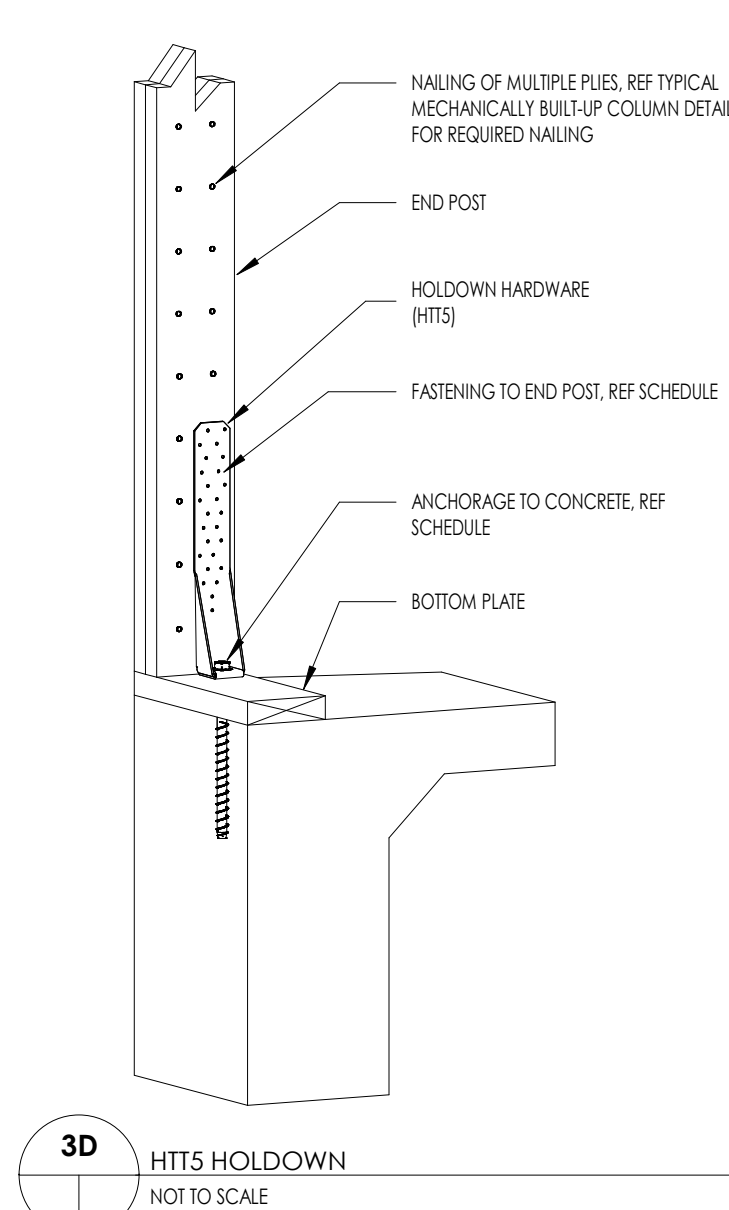
Change Log Table with columns: Date, Description



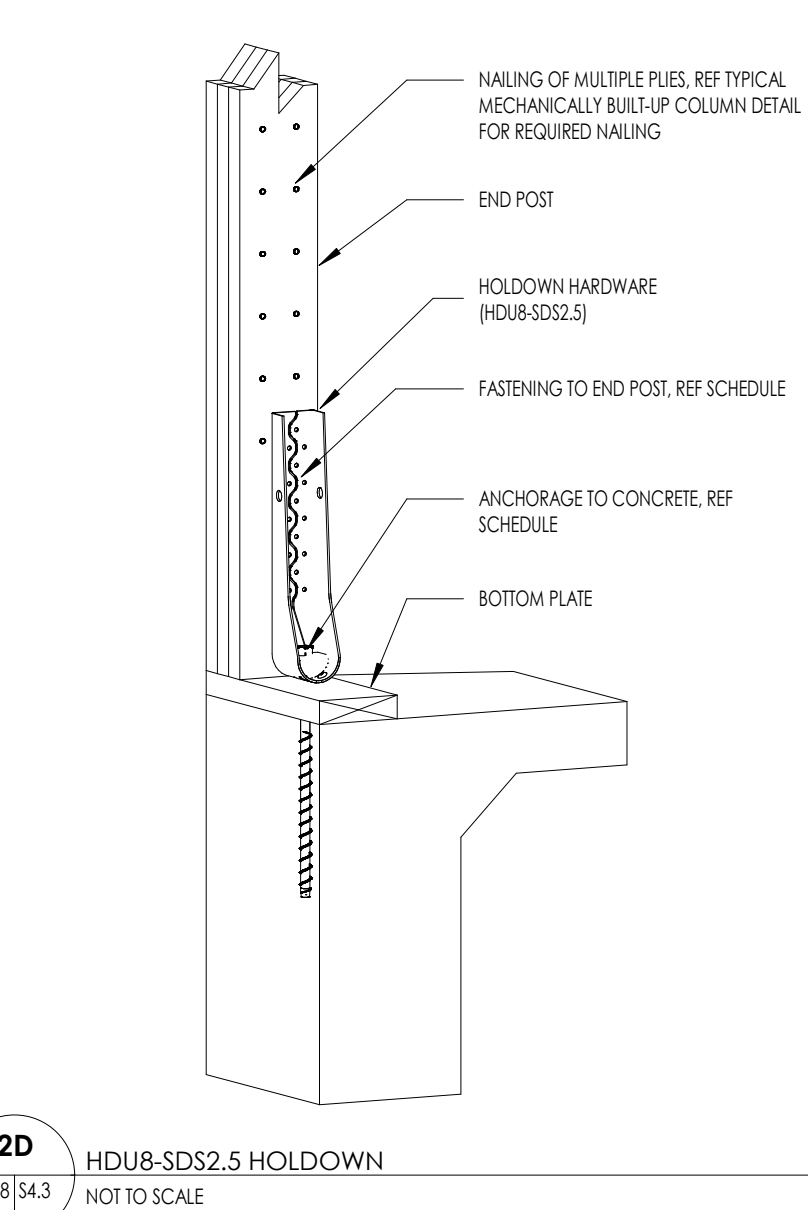
5D LIT HOLDOWN NOT TO SCALE



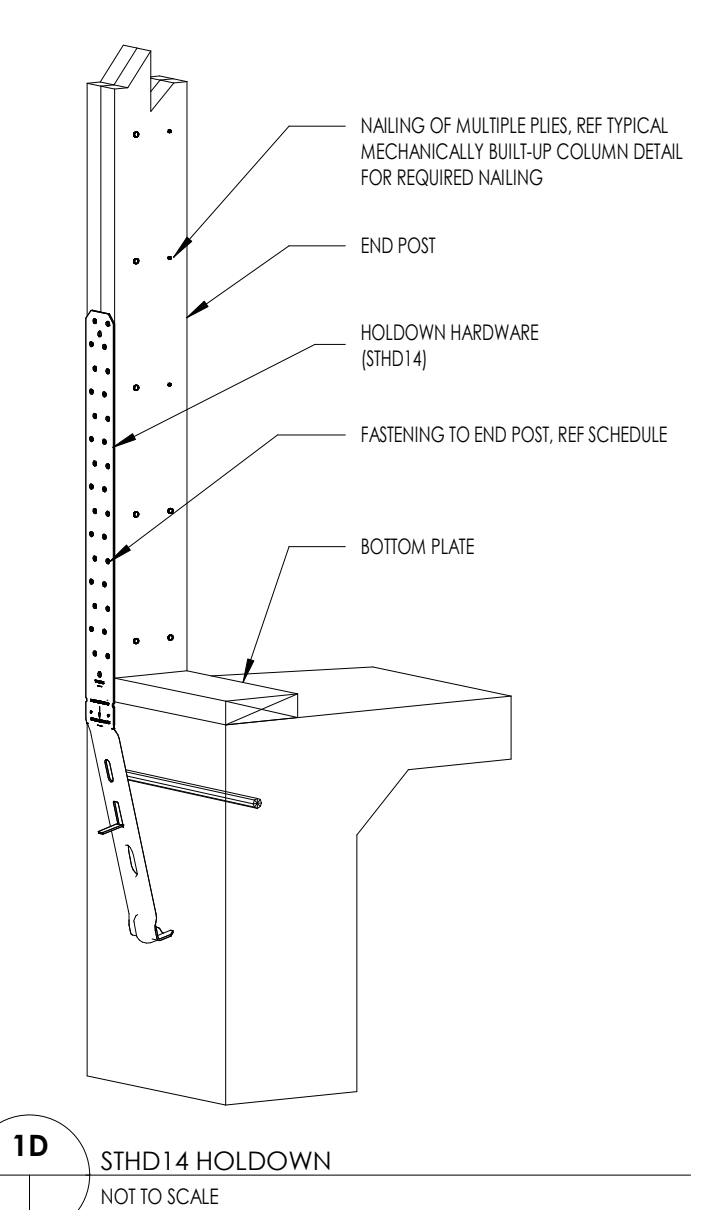
4D HDU14-SDS2.5 HOLDOWN NOT TO SCALE



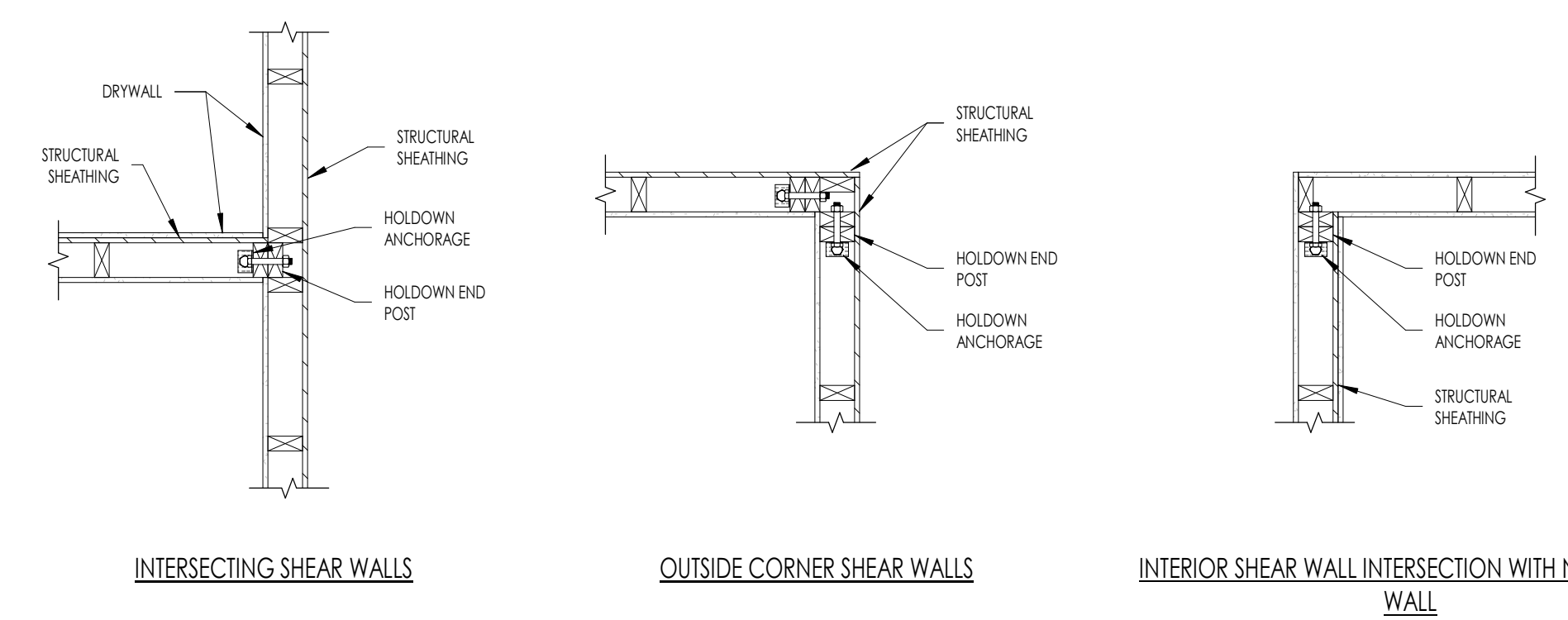
3D HTS HOLDOWN NOT TO SCALE



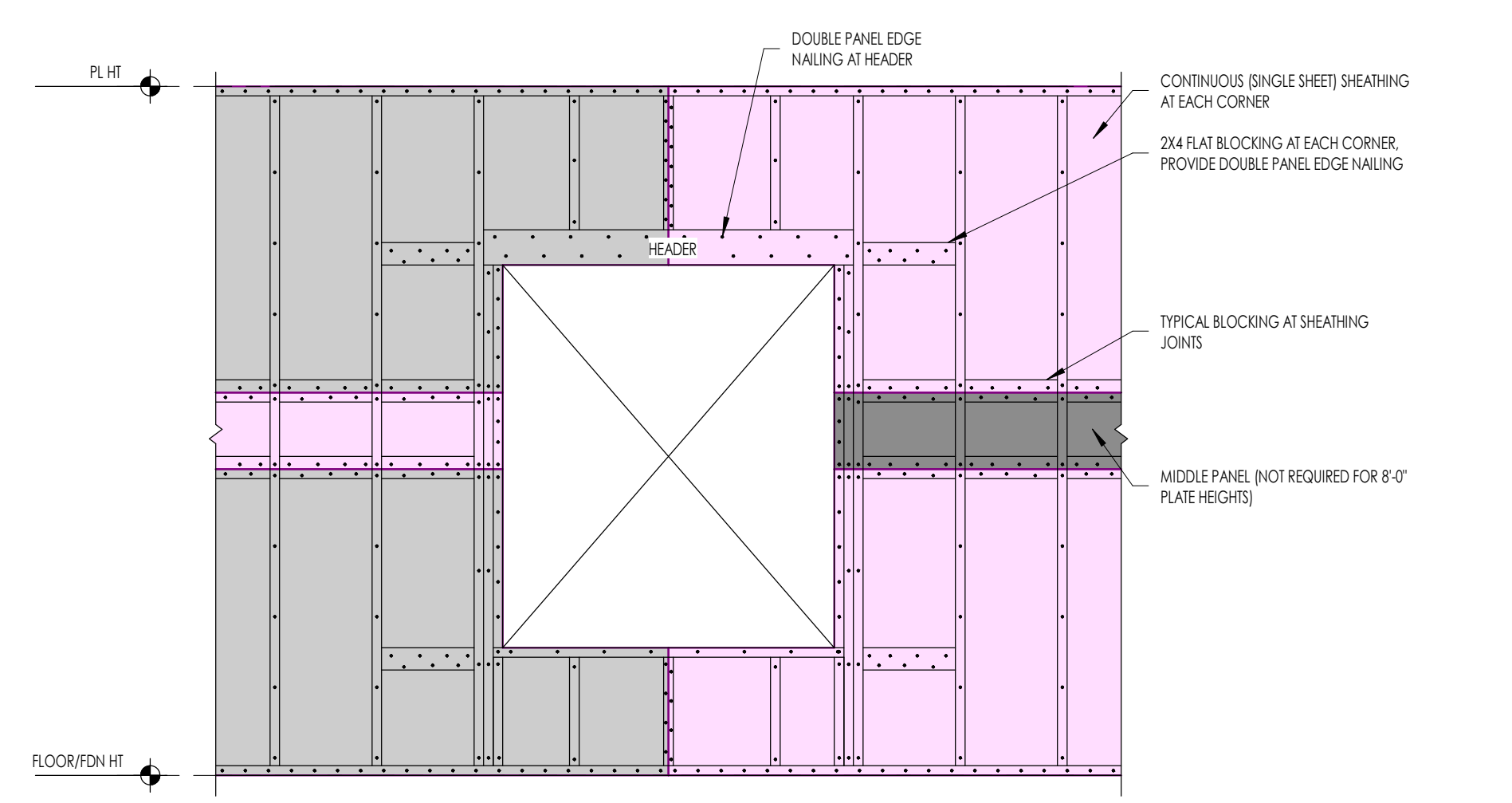
2D HDU8-SDS2.5 HOLDOWN NOT TO SCALE



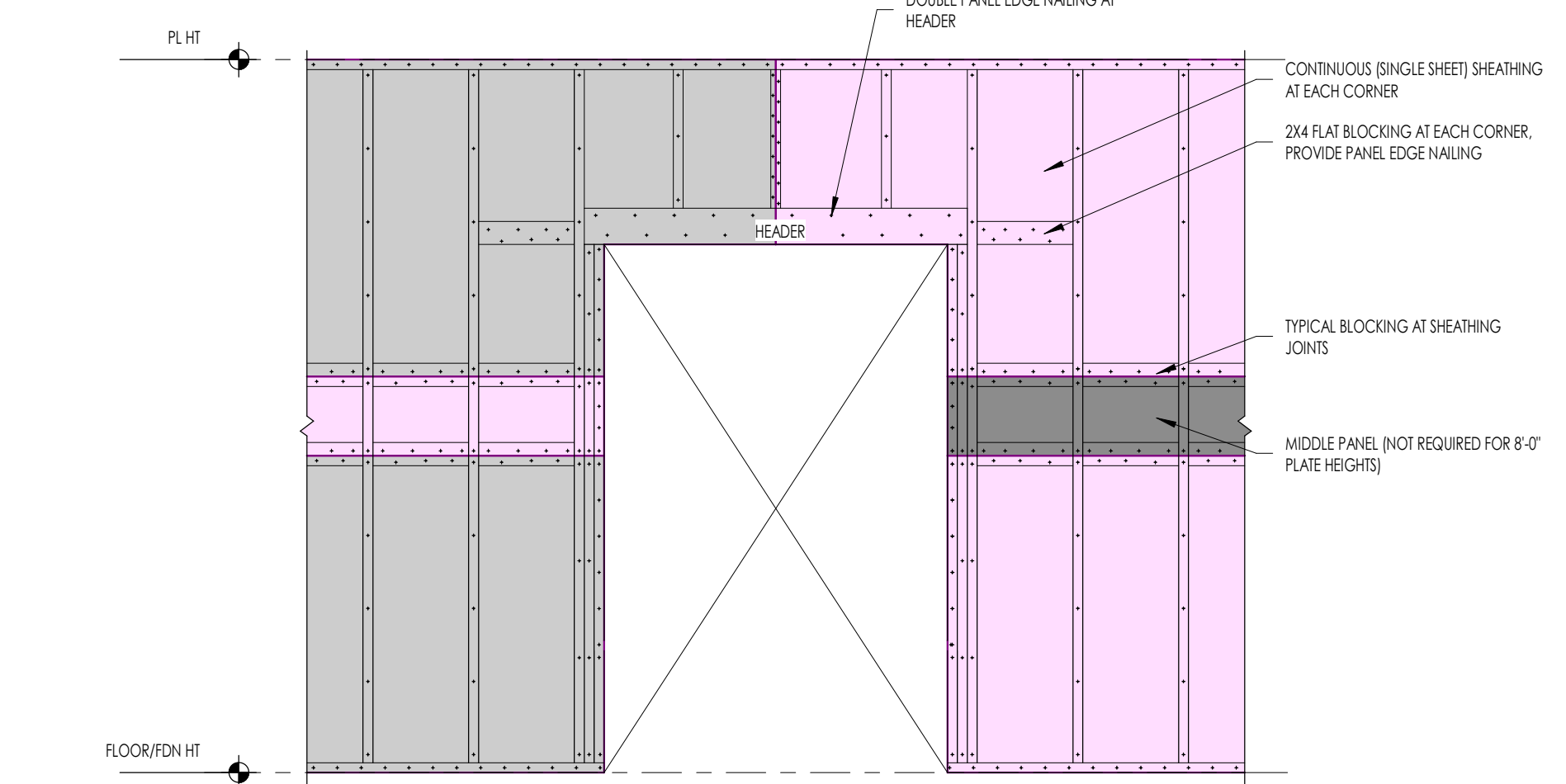
1D SHD1.4 HOLDOWN NOT TO SCALE



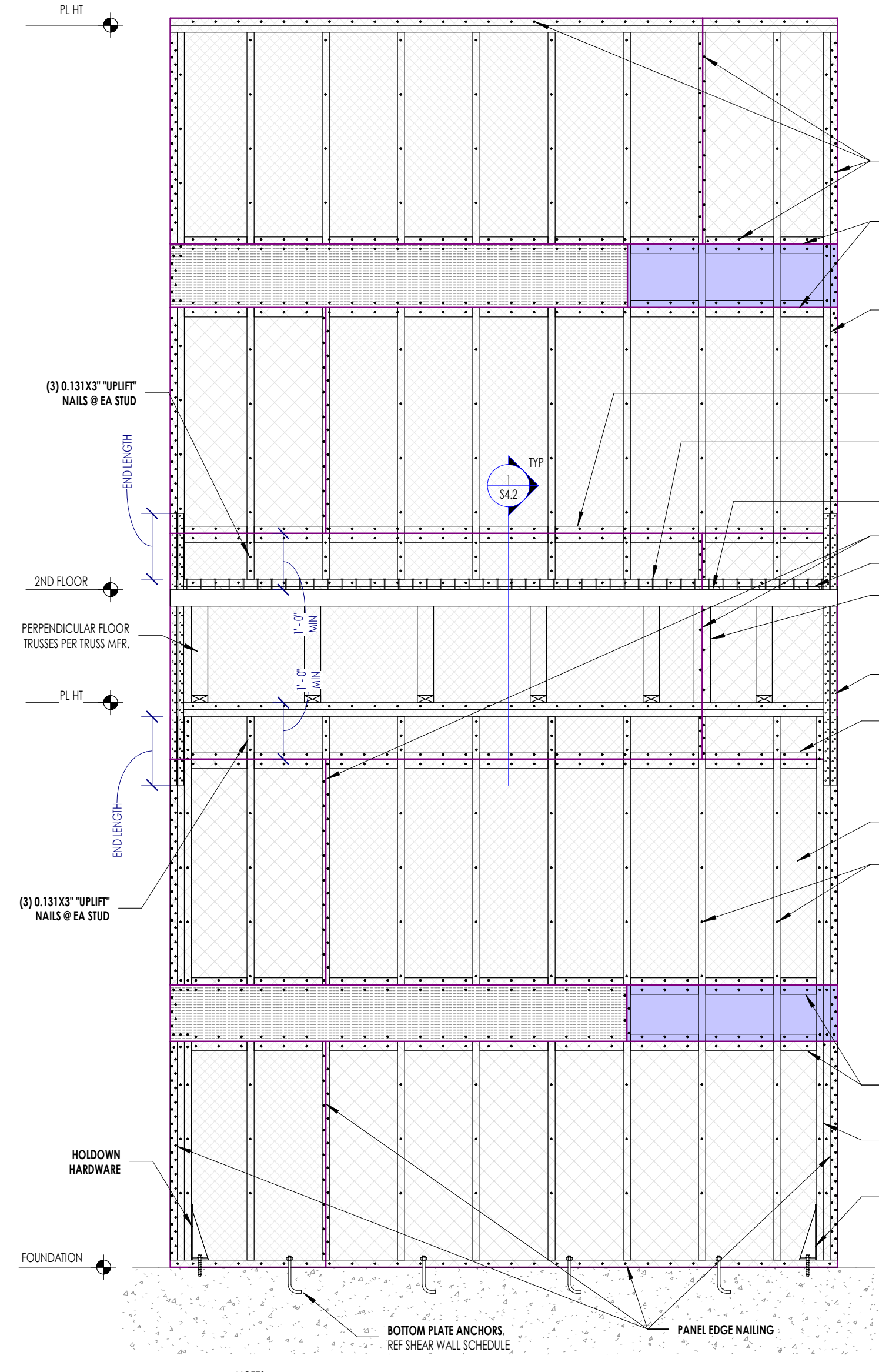
6C SHEAR WALL - END POST CONFIGURATIONS NOT TO SCALE



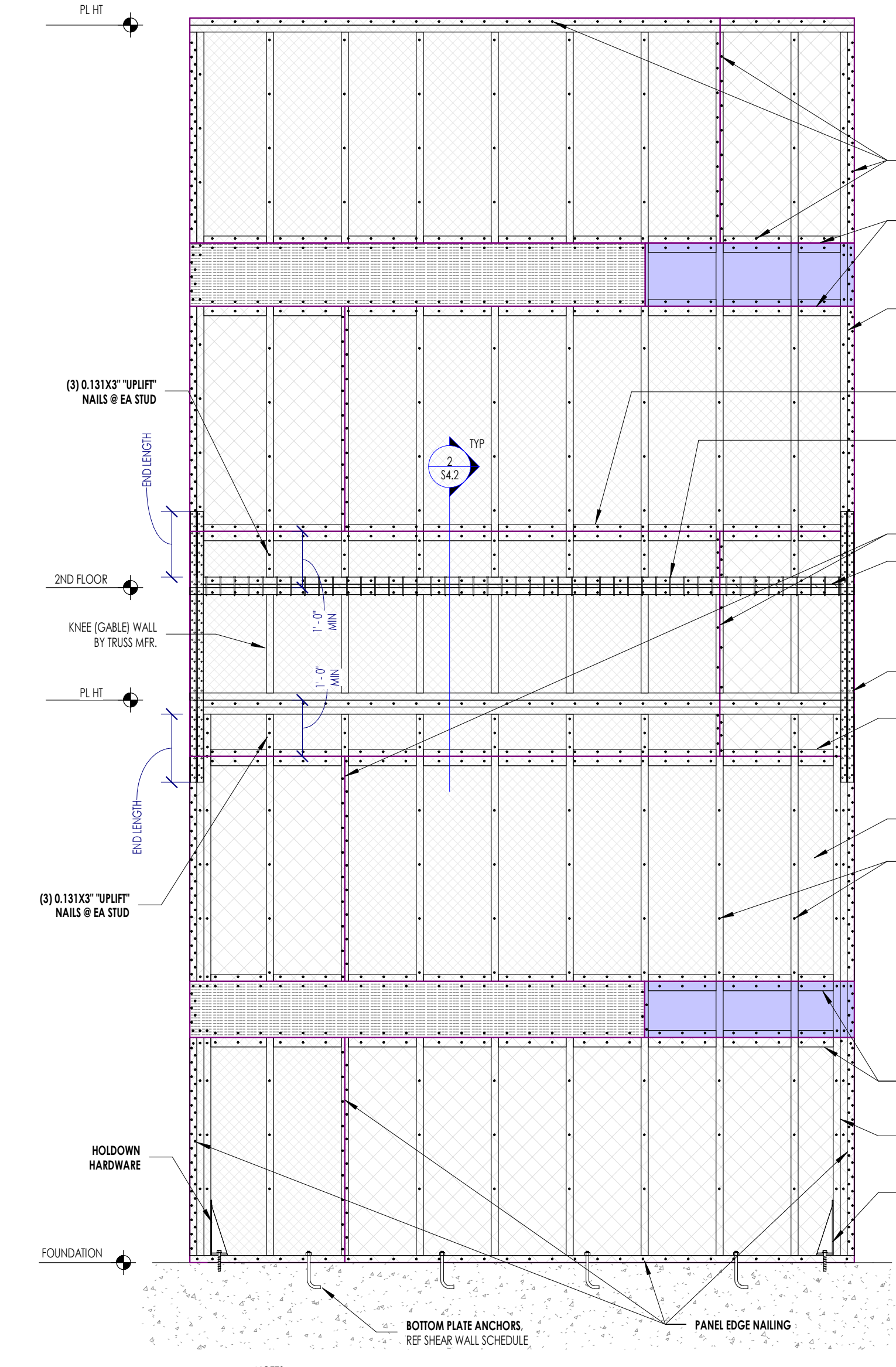
6B SHEAR WALL - FORCE TRANSFER AROUND OPENING NOT TO SCALE



6A SHEAR WALL - FORCE TRANSFER AROUND OPENING (DOOR) NOT TO SCALE



4A TYPICAL MULTIPLE STORY SHEARWALL FRAMING AND FASTENING, TRUSSES PERPENDICULAR NOT TO SCALE



2A TYPICAL MULTIPLE STORY SHEARWALL FRAMING AND FASTENING, TRUSSES PARALLEL NOT TO SCALE

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openingdesign ARCHITECT

Table with 2 columns: Date, Description



RENOVATION  
Wranglers

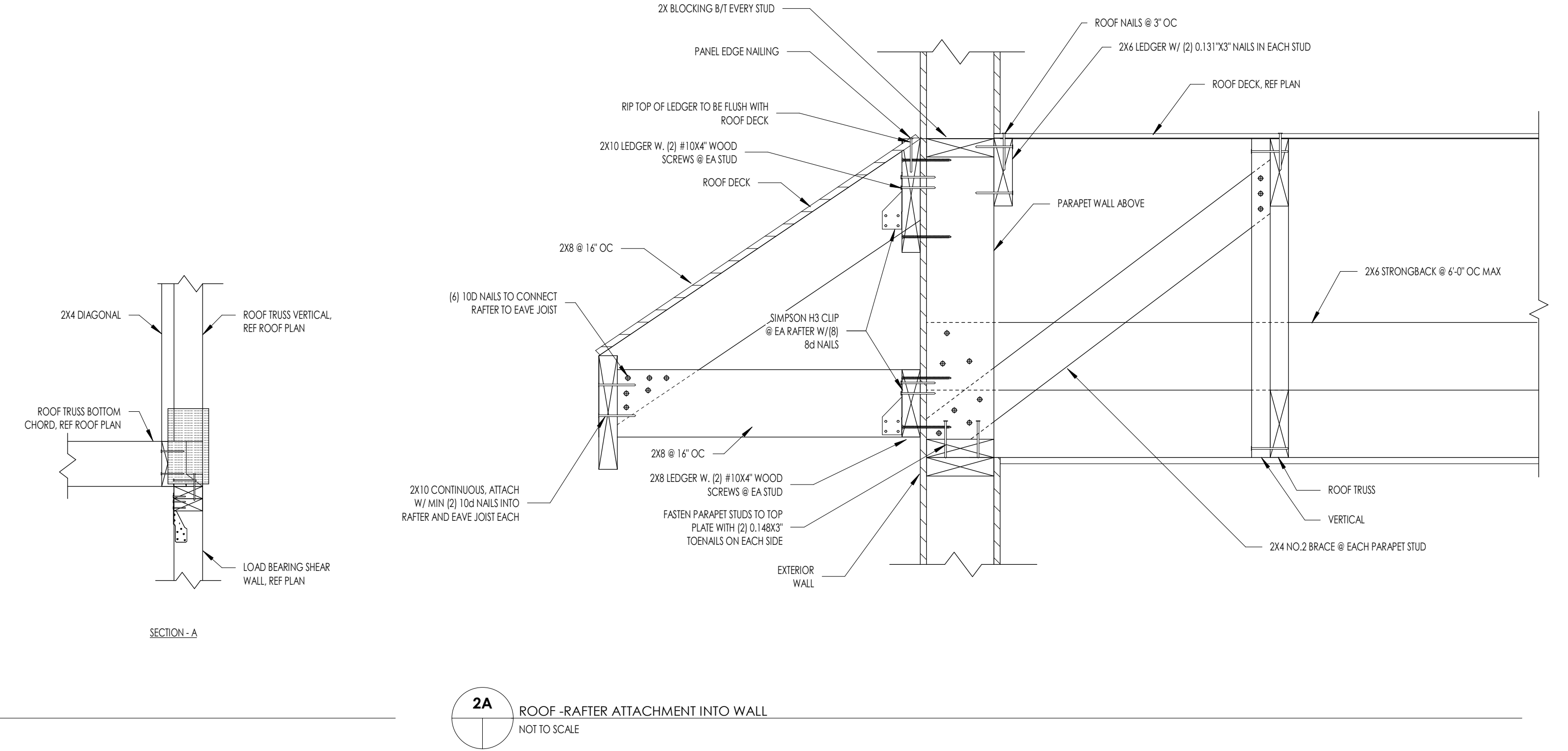
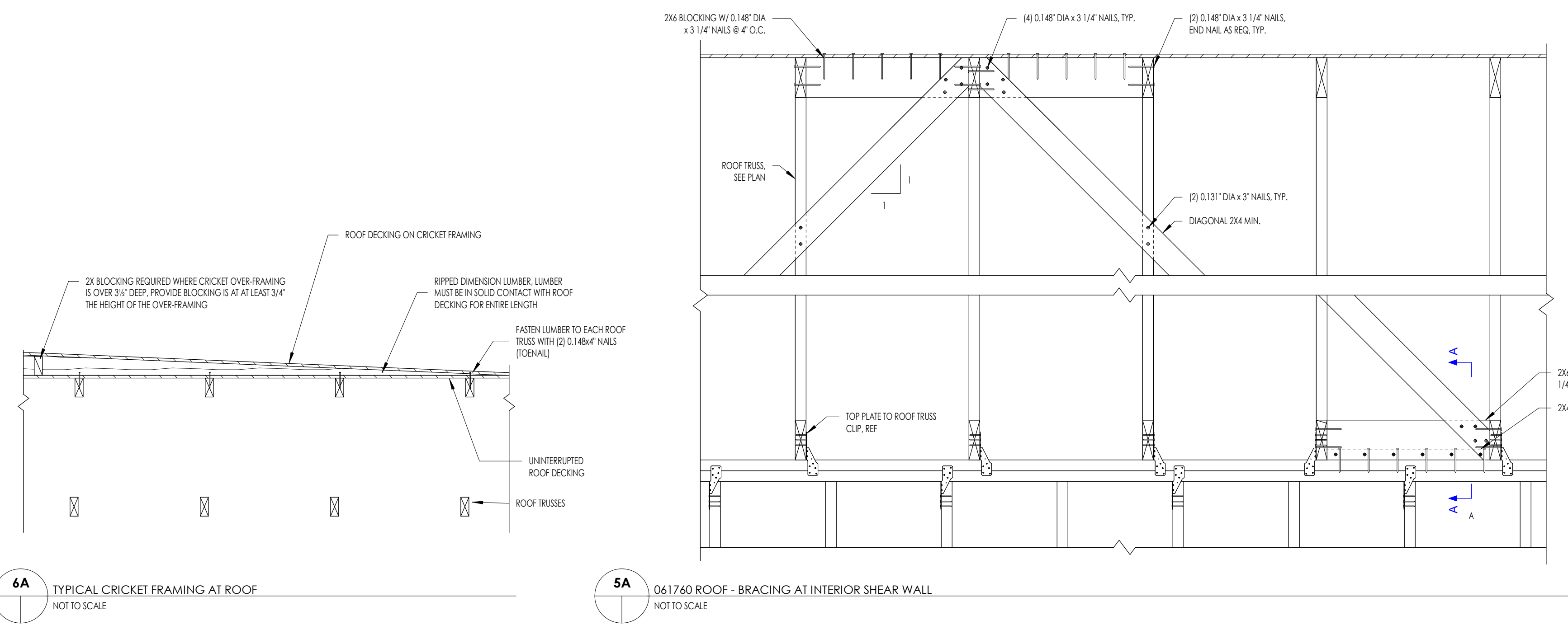
Owner: Renovation Wranglers  
102 E 26th St  
Bryan, TX 77803  
Kateneason@time.com | 979.450.9969

LKB ARCHITECTURE  
Architect of Record: LKB Architecture  
2929 Allen Pkwy Suite 200  
Houston, TX 77019  
isa@lkbarchitecture.com | 713.425.3076

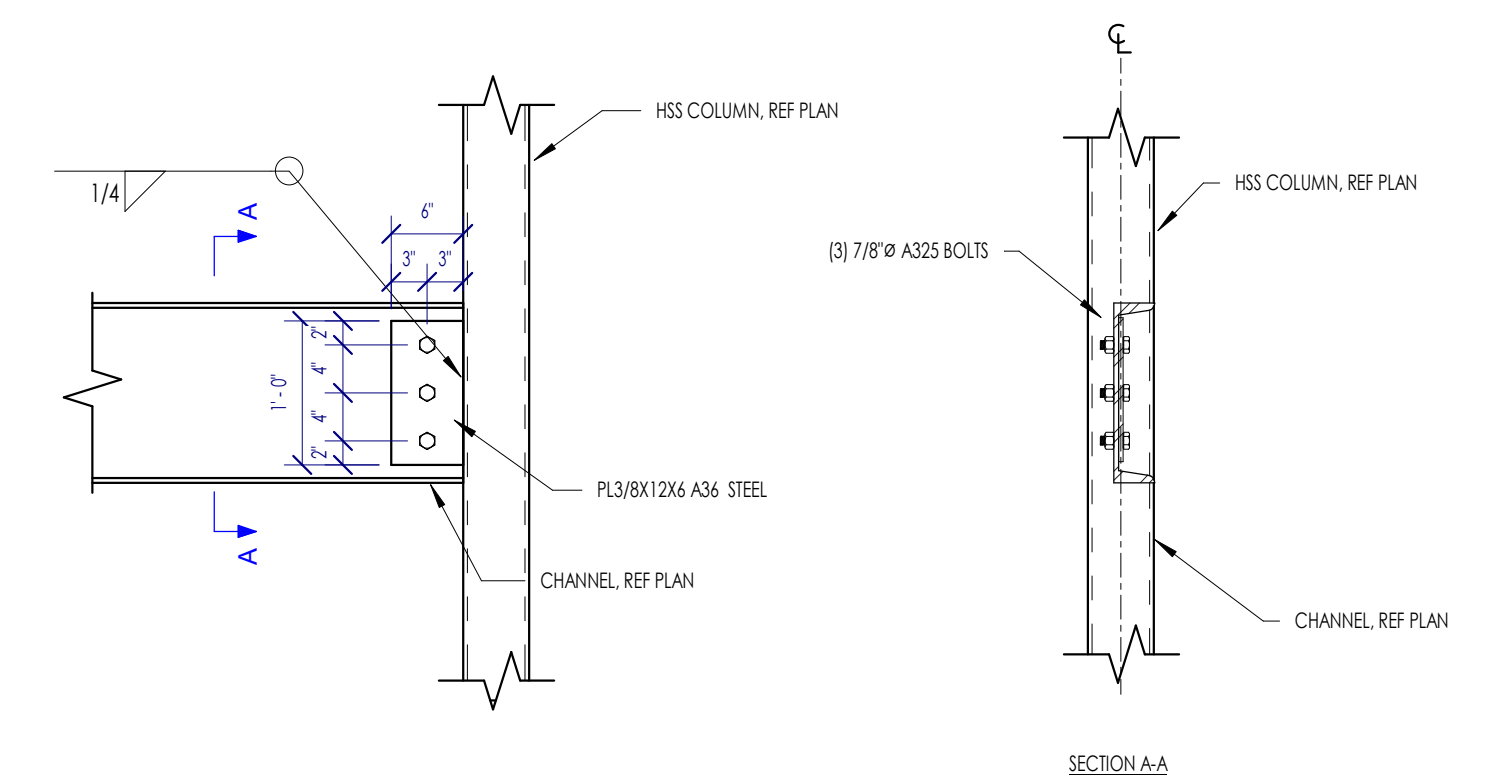
DUDLEY  
Structural: Dudley  
6102 Imperial Loop Drive  
College Station, TX 77845  
(979) 777-0720

amc ENGINEERS  
MEP: AMC Engineers  
508 E Jackson St # 552  
Bumet, TX 78611  
info@amcengineers.com

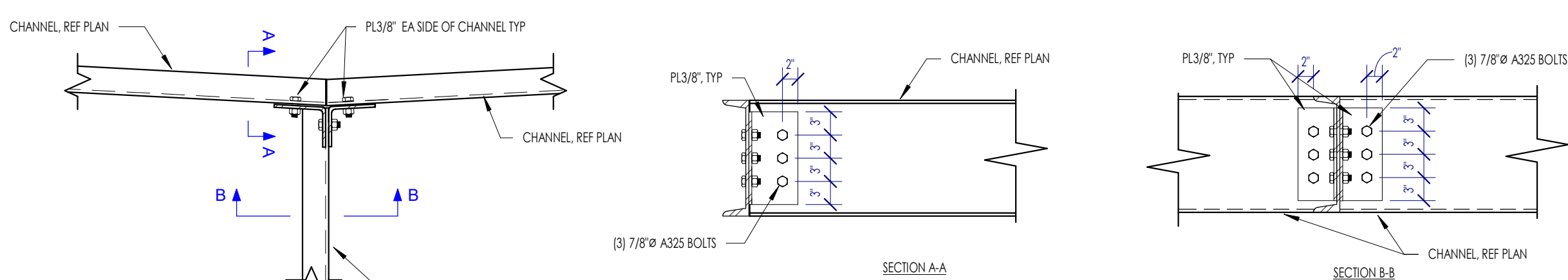
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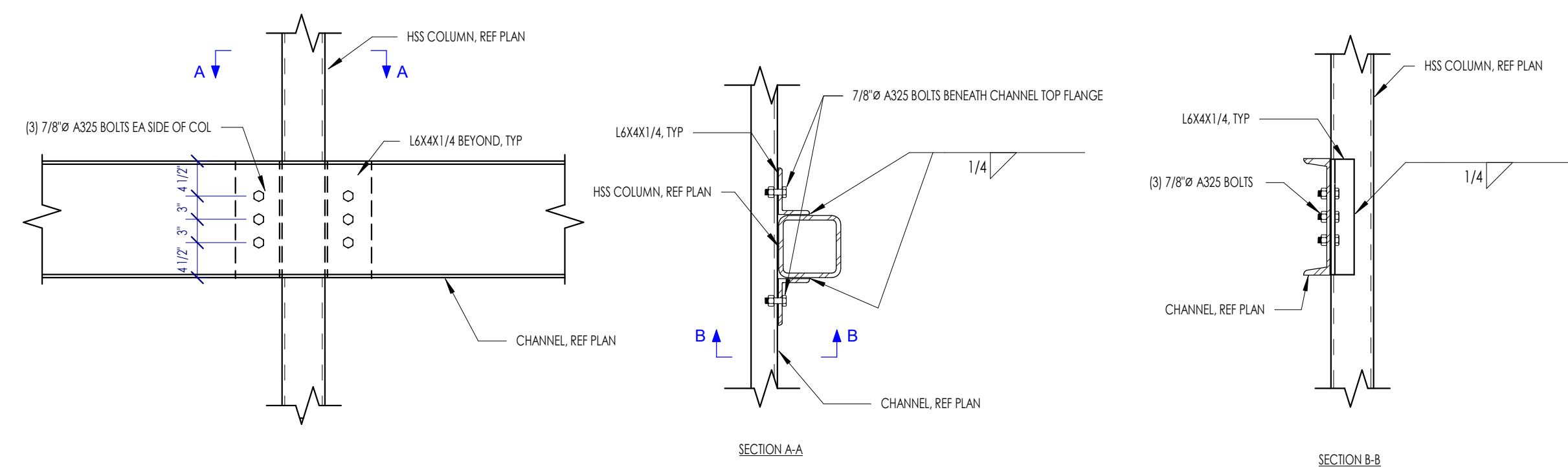


**1C** CHANNEL TO HSS COLUMN CONNECTION - ALIGNED  
NOT TO SCALE

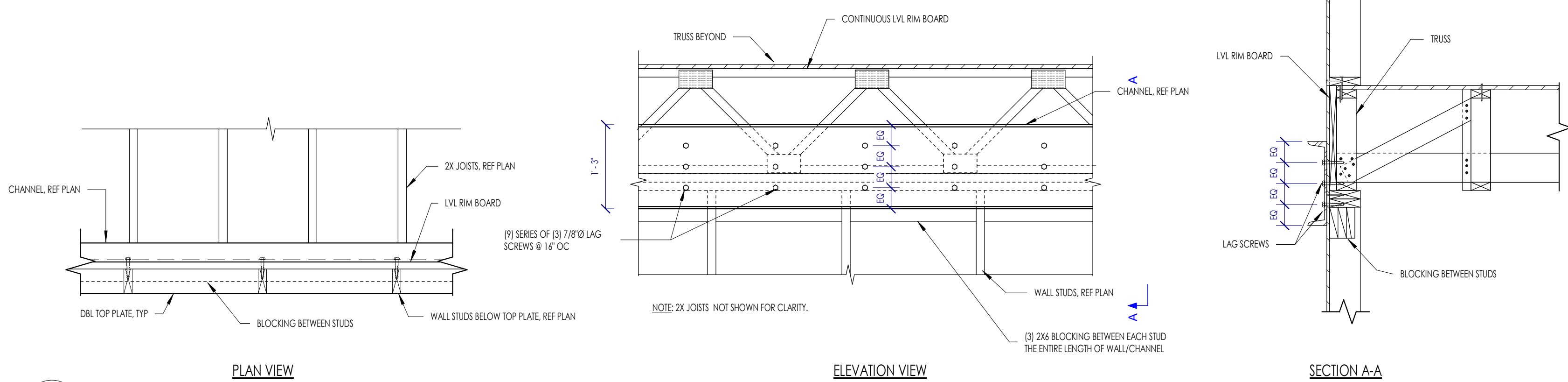


NOTE: CONNECTING PLATES AND BOLTS SHOWN IN PLAN VIEW FOR CLARITY.

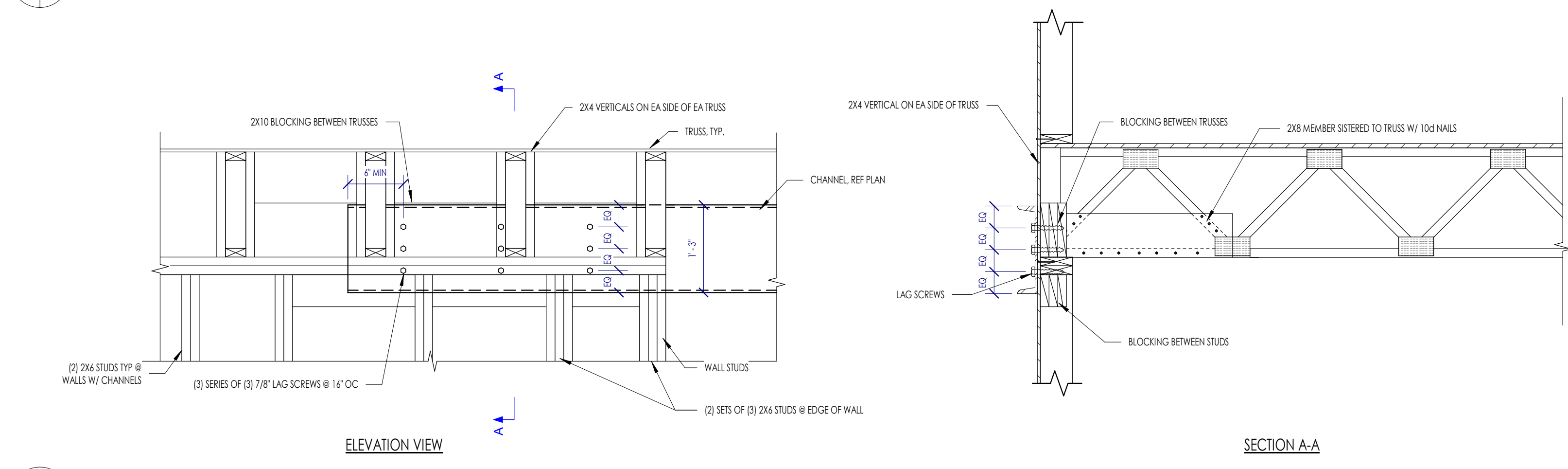
**6B** TYPICAL CHANNEL CONNECTION AT BALCONY  
NOT TO SCALE



**3B** CHANNEL TO HSS COLUMN CONNECTION - ECCENTRIC  
NOT TO SCALE



**6A** TYPICAL CHANNEL TO WALL STUD CONNECTION  
NOT TO SCALE



**3A** TYPICAL CHANNEL TO WALL STUD CONNECTION Copy 2  
NOT TO SCALE

Date	Description