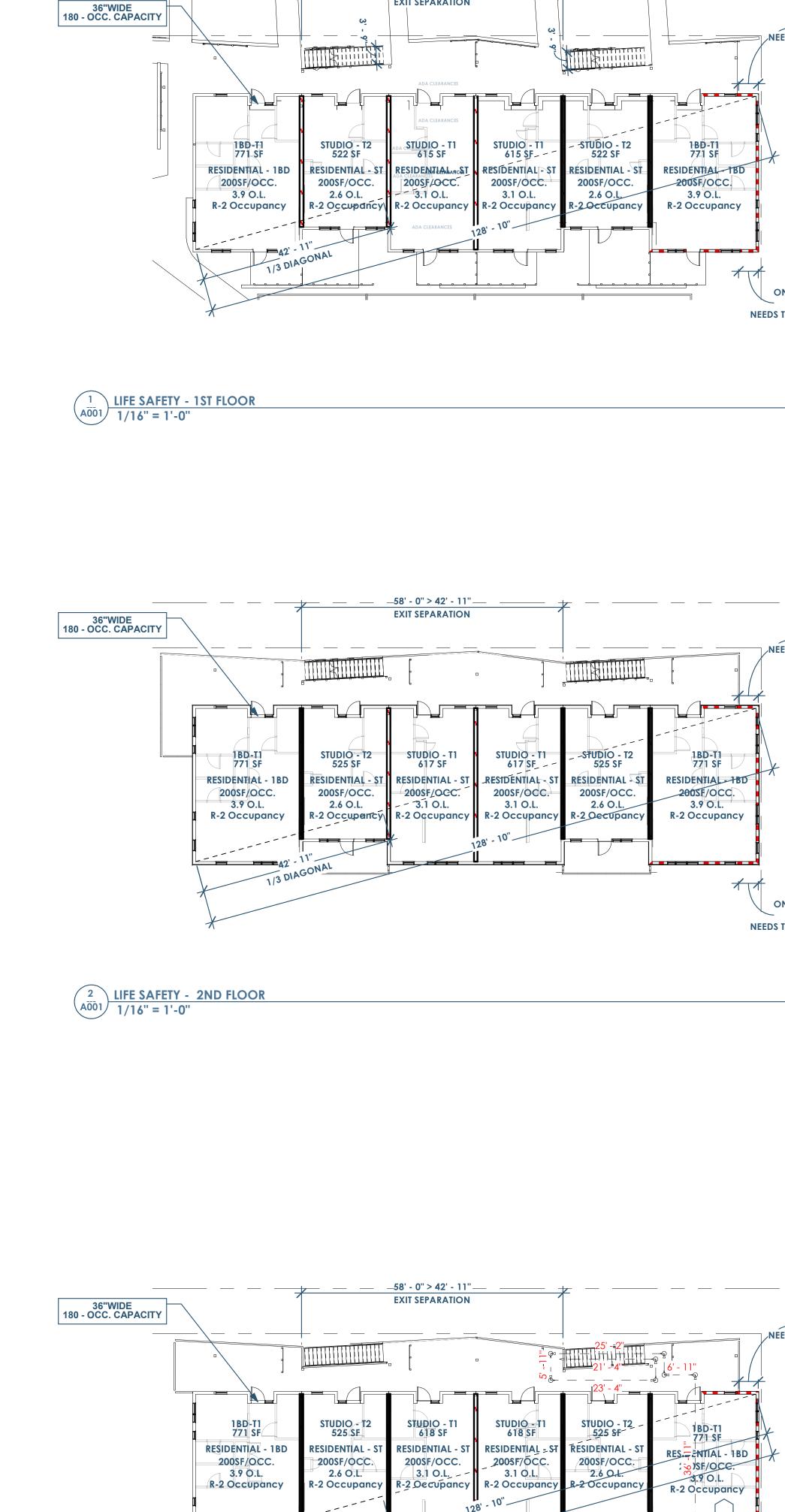

_____ F

EXIT SEPARATION



13 DIAGONAL

42'

4 A001 LIFE SAFETY - 3RD FLOOR 1/16" = 1'-0"

6

	4		

3

ONLY 1ST 4' - 4" NEEDS TO BE 1HR RATED

ONLY THE 1ST 4' - 4" NEEDS TO BE 1HR RATED

ONLY 1ST 4' - 4"

NEEDS TO BE 1HR RATED

ONLY THE 1ST 4' - 4" NEEDS TO BE 1HR RATED

ONLY 1ST 4' - 4" NEEDS TO BE 1HR RATED

12' - 4"

____ <u>`</u>

ROUTE ONLY THE 1ST 4' - 4" NEEDS TO BE 1HR RATED

RATED - 4 HR RATED - 3 HR RATED - 2 HR RATED - 1.5 HR RATED - 1 HR RATED - 0.5 HR

A001 LIFE SAFETY LEGEND

EGRES	s data
EXIT ROUTE	DISTANCE
ROUTE 1	141' - 4"
Route A	50' - 0"

MARCO POLO - 101 W 33RD STREET - CITY OF BRYAN TOWNSITE, BLOCK 96, LOT 3-5 & PT OF 6 & PT OF ALLEY - BRYAN, TX 77803

PRELIMINARY - NOT FOR CONSTRUCTION

CODE INFORMATION

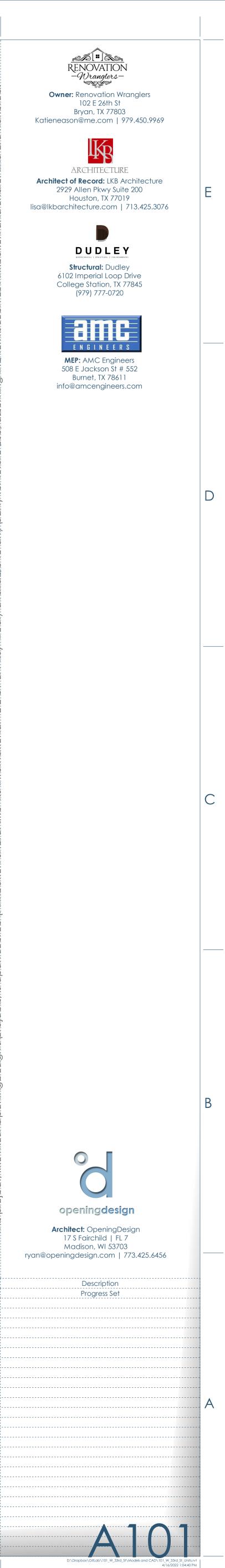
						Area	
Number	Level	Name	Occupancy	Area	Function of Space	Allowance Per Occupant	Occupan Load
B112	1ST FLOOR	1BD-T1	R-2	771 SF	RESIDENTIAL - 1BD	200	3.9
B117	1ST FLOOR	18D-T1	R-2	771 SF	RESIDENTIAL - 1BD	200	3.9
B118	2ND FLOOR	18D-T1	R-2	771 SF	RESIDENTIAL - 1BD	200	3.9
B123	2ND FLOOR	18D-T1	R-2	771 SF	RESIDENTIAL - 1BD	200	3.9
B124	3RD FLOOR	1BD-T1	R-2	771 SF	RESIDENTIAL - 1BD	200	3.9
B129	3RD FLOOR	1BD-T1	R-2	771 SF	RESIDENTIAL - 1BD	200	3.9
B113	1ST FLOOR	STUDIO - T2	R-2	522 SF	RESIDENTIAL - ST	200	2.6
B114	1ST FLOOR	STUDIO - T1	R-2	615 SF	RESIDENTIAL - ST	200	3.1
B115	1ST FLOOR	STUDIO - T1	R-2	615 SF	RESIDENTIAL - ST	200	3.1
B116	1ST FLOOR	STUDIO - T2	R-2	522 SF	RESIDENTIAL - ST	200	2.6
B119	2ND FLOOR	STUDIO - T2	R-2	525 SF	RESIDENTIAL - ST	200	2.6
B120	2ND FLOOR	STUDIO - T1	R-2	617 SF	RESIDENTIAL - ST	200	3.1
B121	2ND FLOOR	STUDIO - T1	R-2	617 SF	RESIDENTIAL - ST	200	3.1
B122	2ND FLOOR	STUDIO - T2	R-2	525 SF	RESIDENTIAL - ST	200	2.6
B125	3RD FLOOR	STUDIO - T2	R-2	525 SF	RESIDENTIAL - ST	200	2.6
B126	3RD FLOOR	STUDIO - T1	R-2	618 SF	RESIDENTIAL - ST	200	3.1
B127	3RD FLOOR	STUDIO - T1	R-2	618 SF	RESIDENTIAL - ST	200	3.1
B128	3RD FLOOR	STUDIO - T2	R-2	525 SF	RESIDENTIAL - ST	200	2.6
2: 18				11468 SF			57.3
rand total	: 18			11468 SF			57.3

CODE & LIFE SAFETY

Date

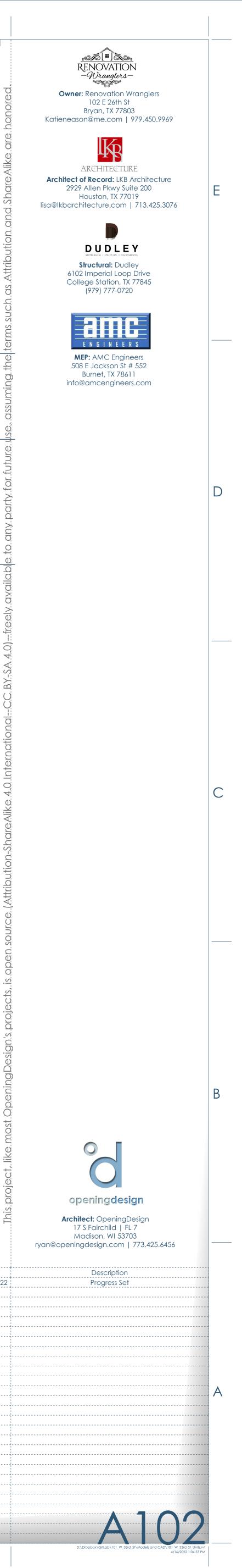


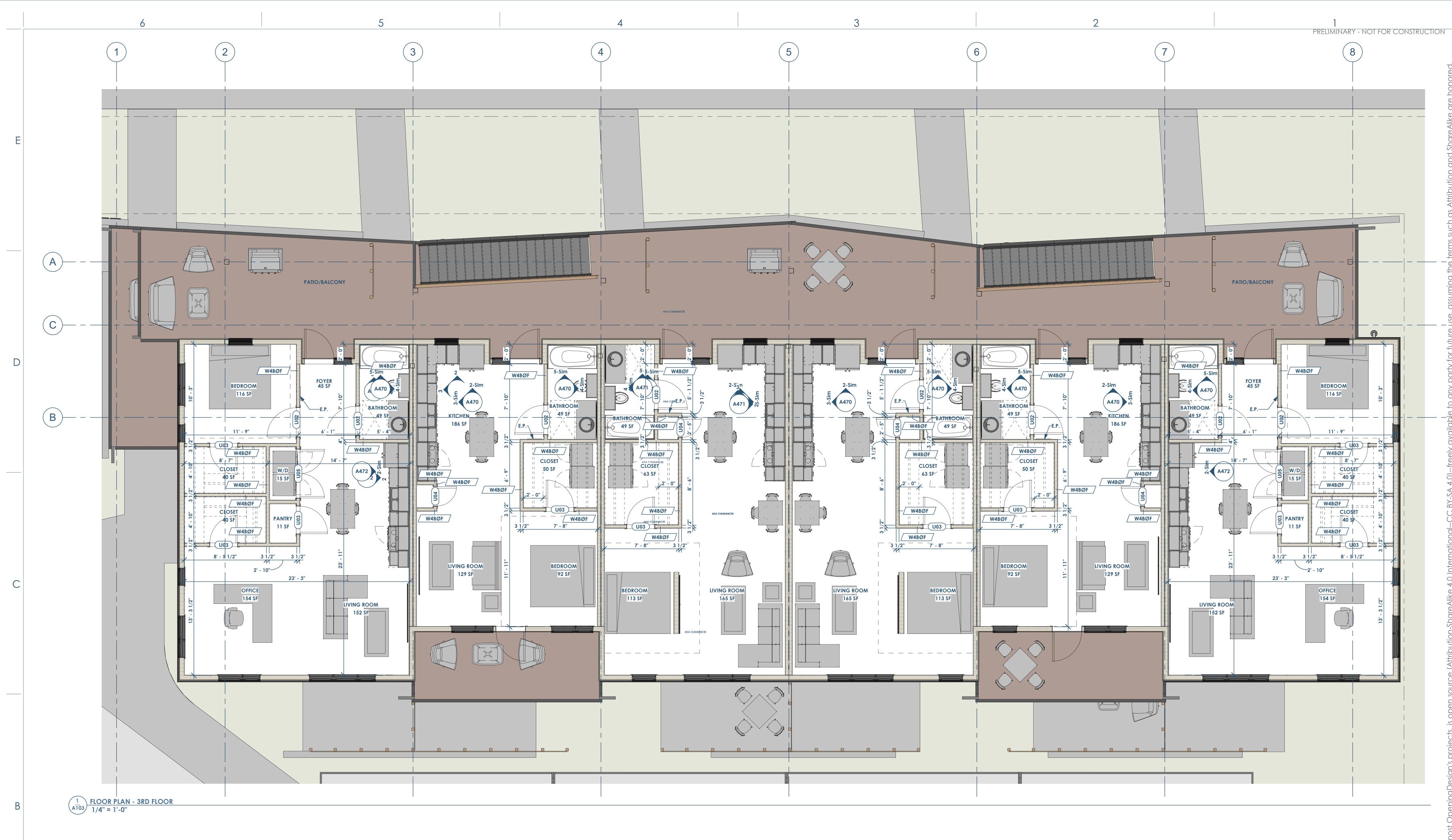


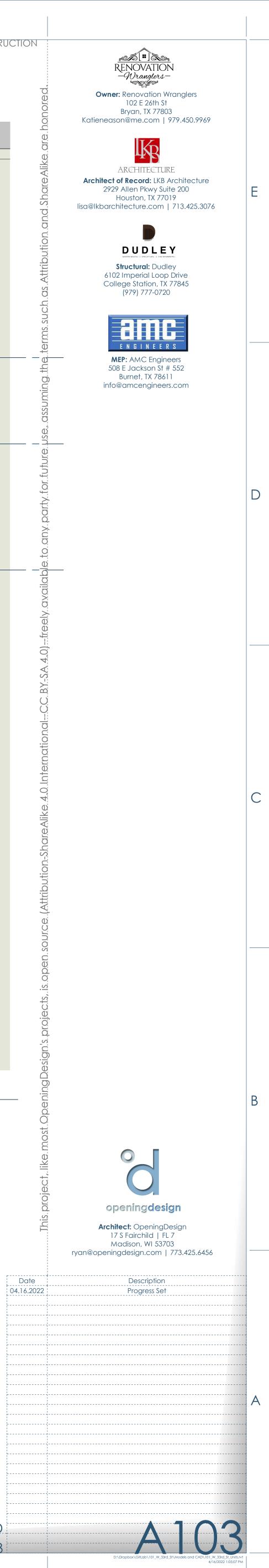


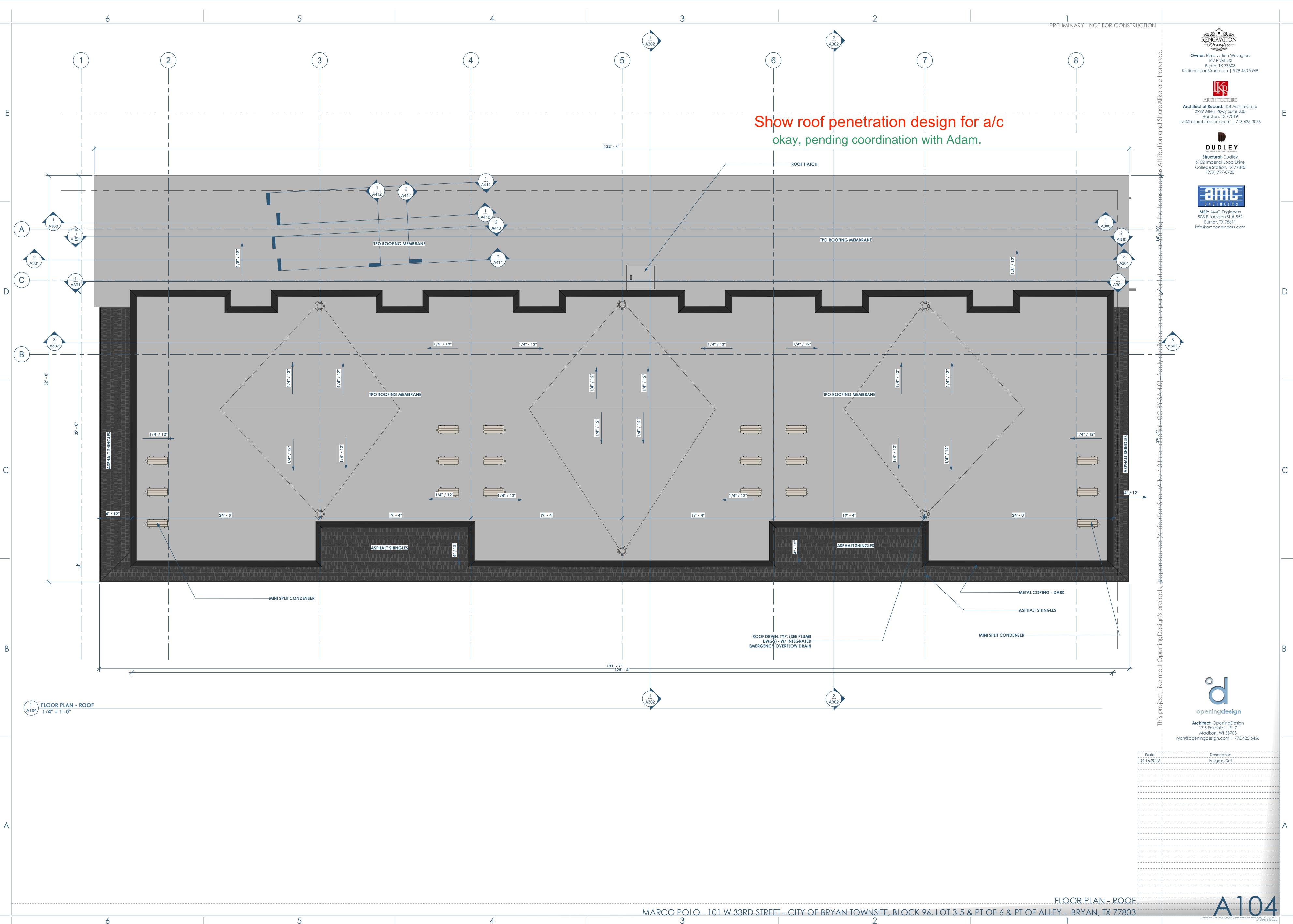
4/16/2022 1:04:40 PM













I am not familiar with treated Cedar this is a mistak Can this be a 4x4 cedar treated? If so I will look at price difference	VS TREATED 4X4
yes.4x4 cedar works if it's No. 1 grade, if not I would use a 5x5 No. 2 grade	TREATED 4X4 EDGES - EX INSU
-	

MARCO POLO - 101 W 33RD STREET - CITY OF BRYAN TOWNSITE, BLOCK 96, LOT 3-5 & PT OF 6 & PT OF ALLEY - BRYAN, TX 77803

 \mathbf{O}

BUILDING ELEVATION - NORTH/WEST

Date





_____ _

6

2 A201 3/16" = 1'-0"

MARCO POLO - 101 W 33RD STREET - CITY OF BRYAN TOWNSITE, BLOCK 96, LOT 3-5 & PT OF 6 & PT OF ALLEY - BRYAN, TX 77803

2 \1

A300¹/30

A301

A302

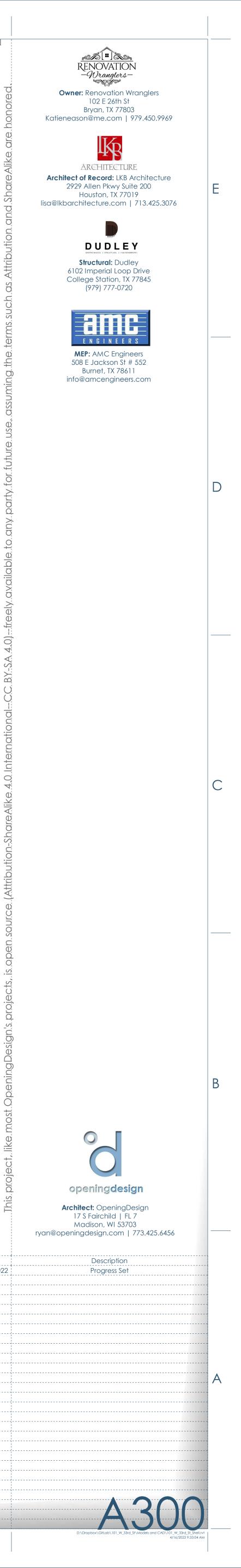
0' - 0'' - <u>1S</u>T F<u>LOOR</u>

BUILDING ELEVATION - SOUTH/EAST

Date



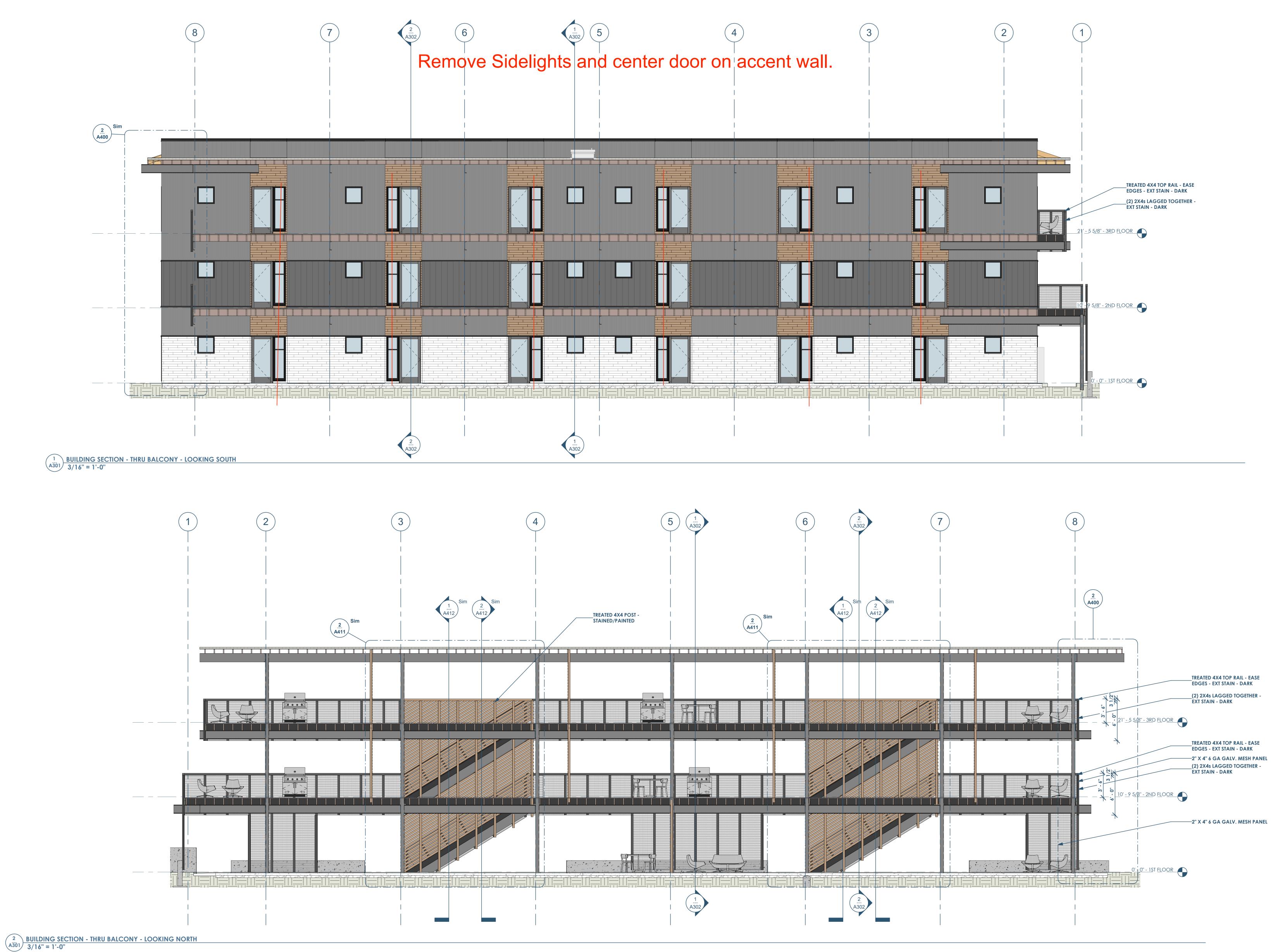






1 BUILDING SECTION - THRU BALCONY - LOOKING SOUTH (A301) 3/16" = 1'-0"

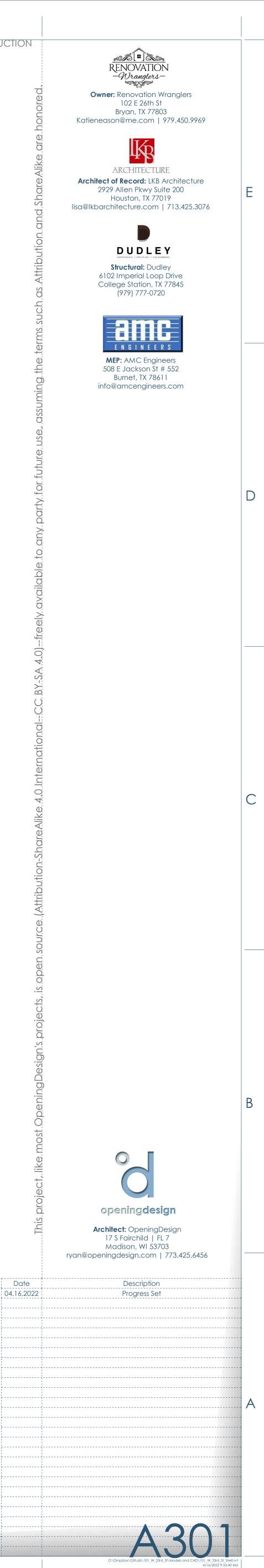
6



6

BUILDING SECTIONS

Date





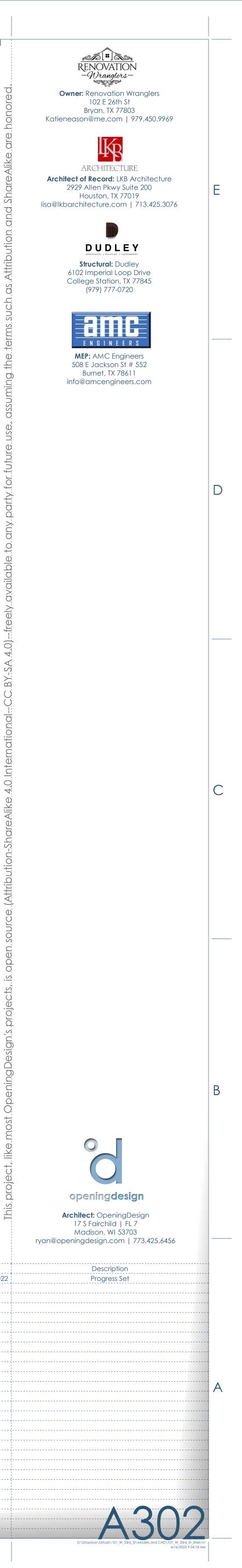


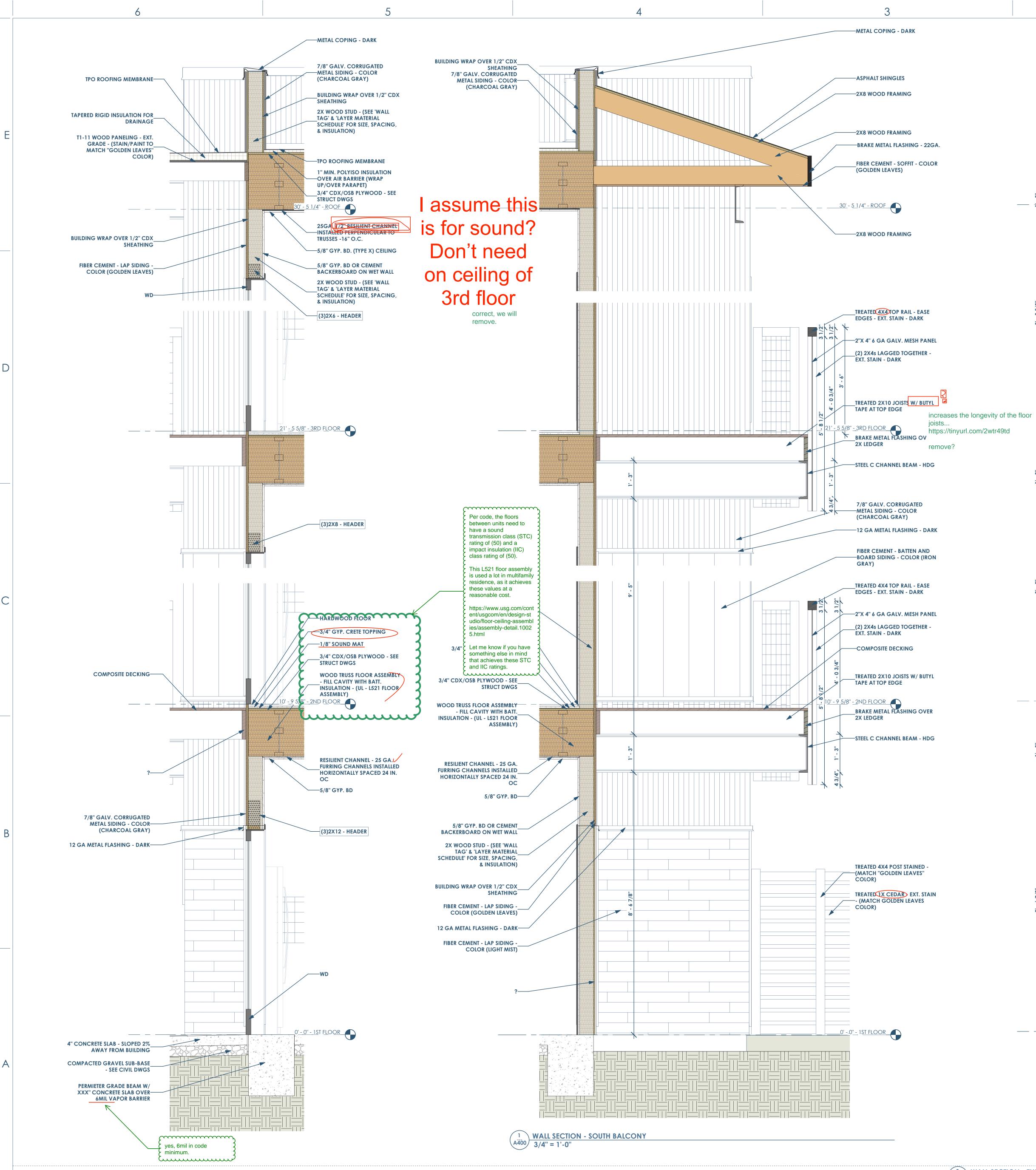
BUILDING SECTION - EAST/WEST - LOOKING SOUTH 3/16" = 1'-0"

6

6

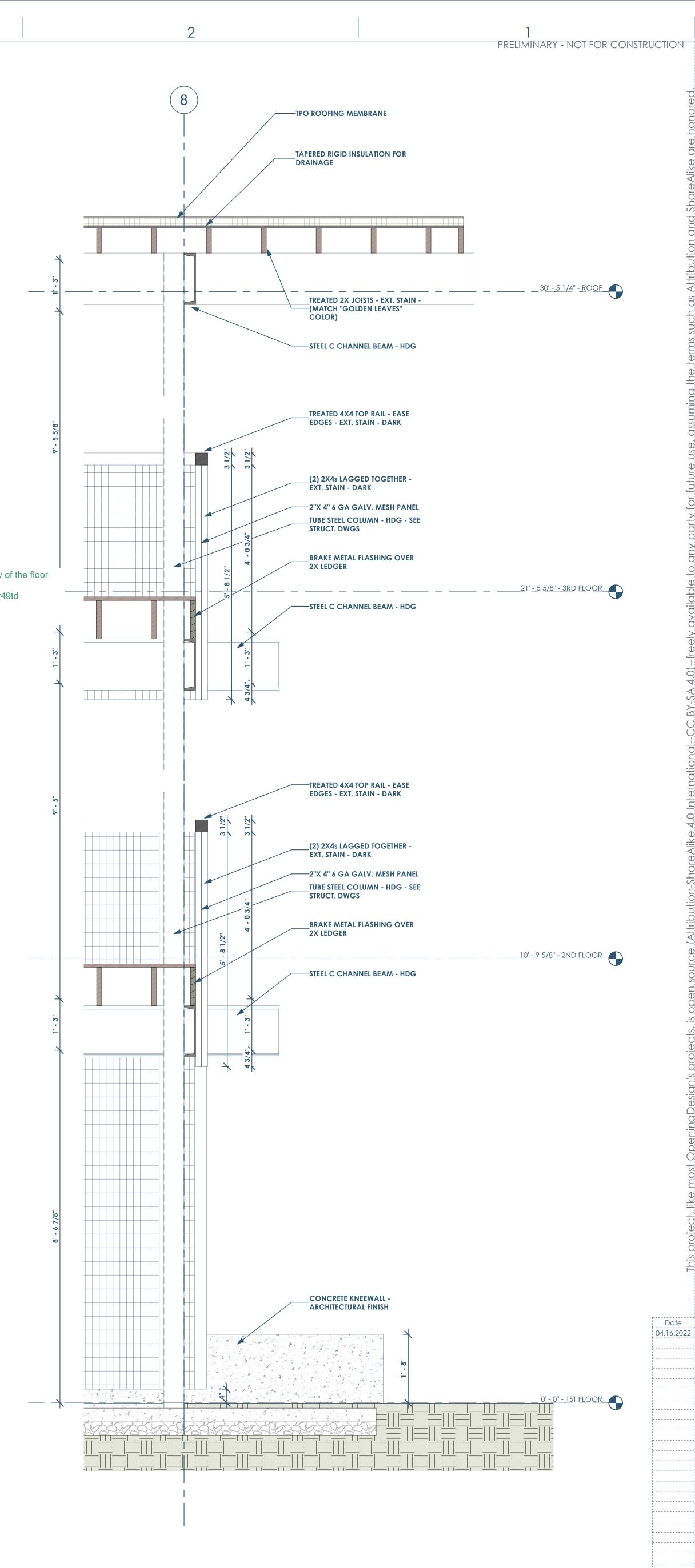
 \mathbf{O}



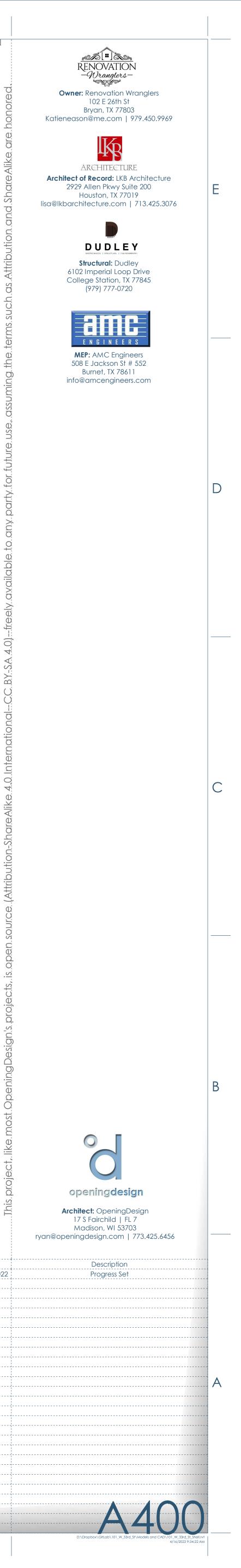


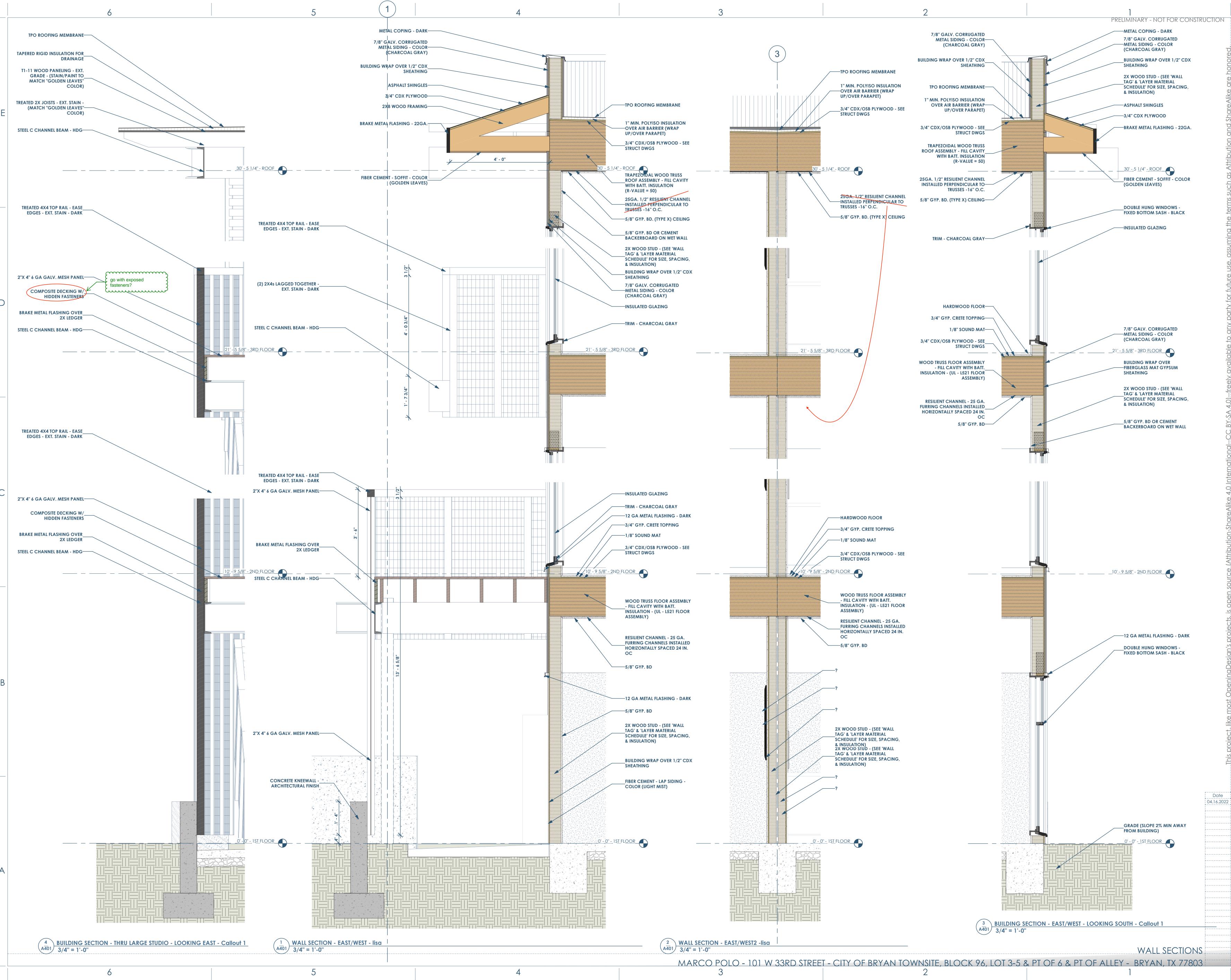
BUILDING SECTION - THRUSMALL STUDIO - LOOKING EAST - Callout 1 3/4" = 1'-0"

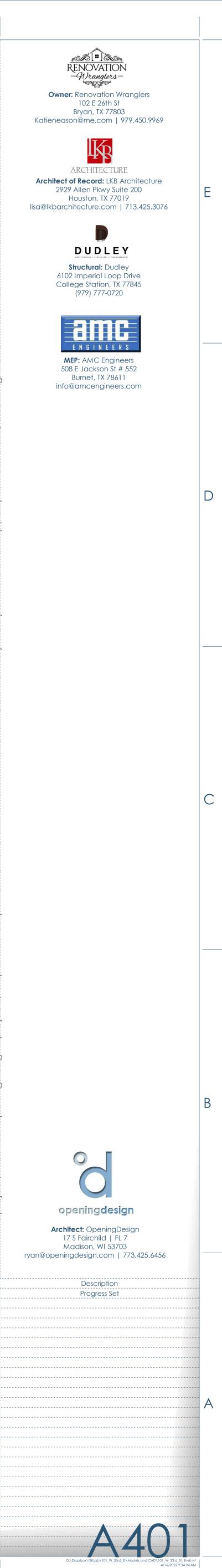
MARCO POLO - 101 W A MAIL SECTION - THRU BALCONY - IIS A TOWNSITE, BLOCK 96, LOT 3-5 & PT OF 6 & PT OF ALLEY - BRYAN, TX 77803

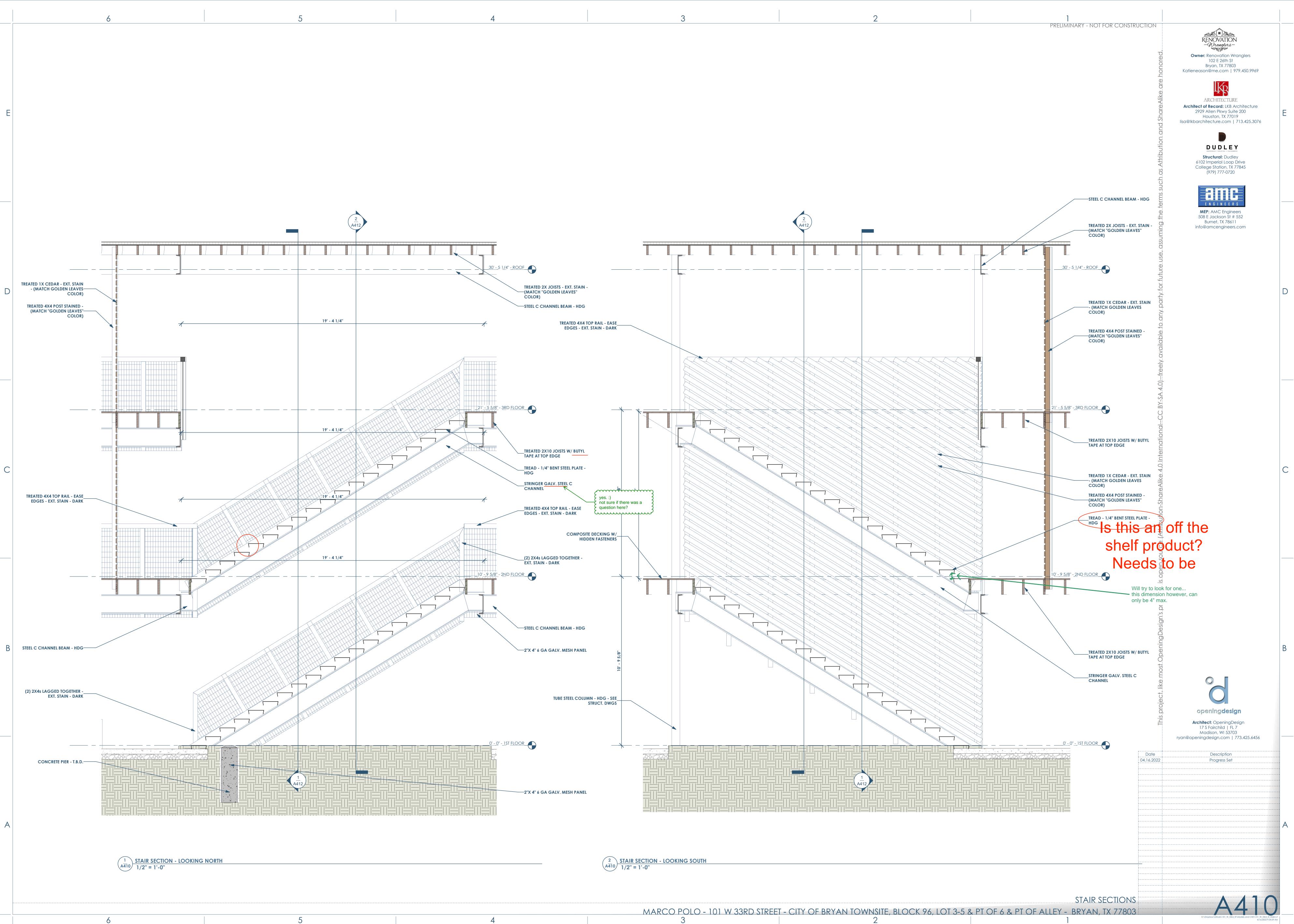


WALL SECTIONS



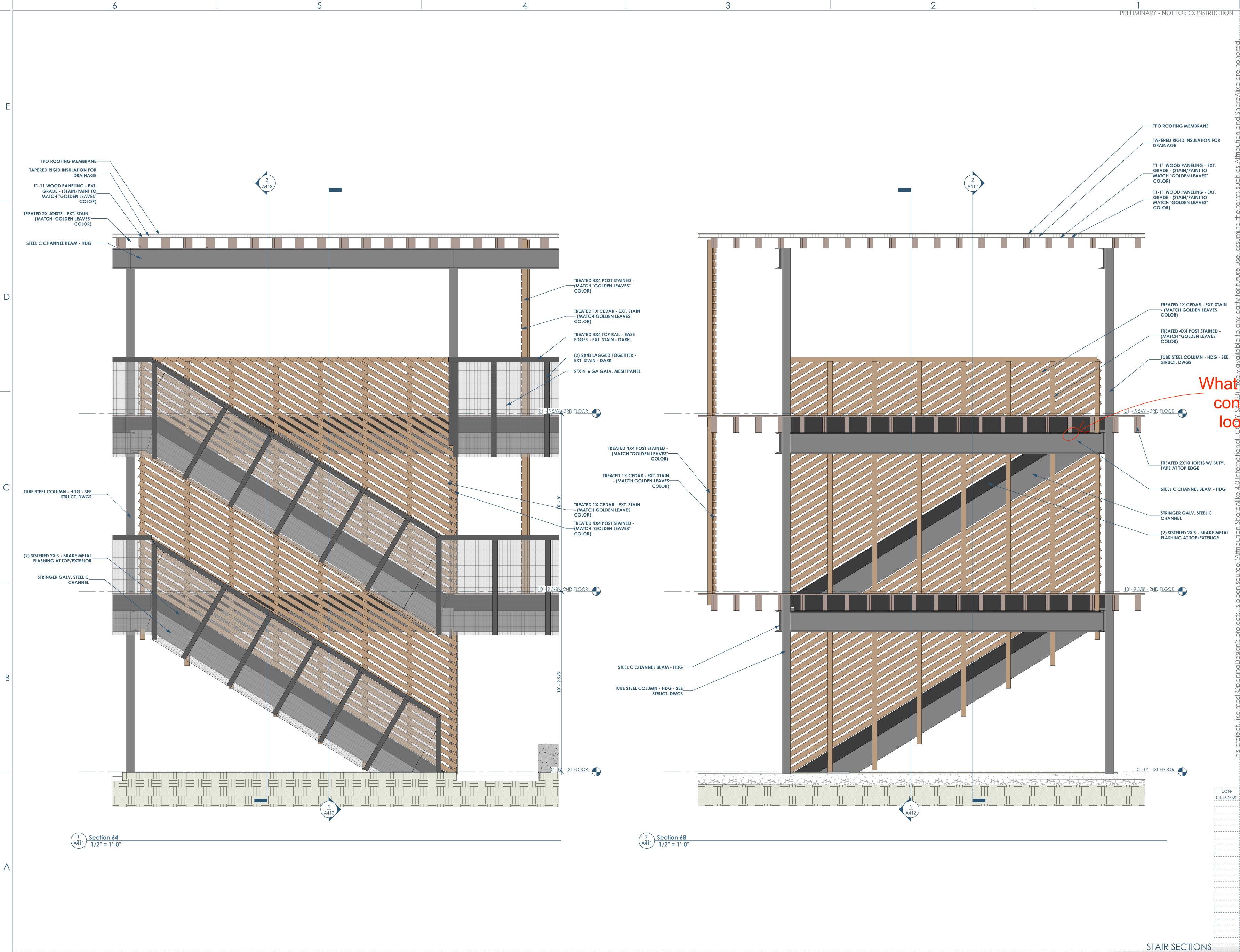




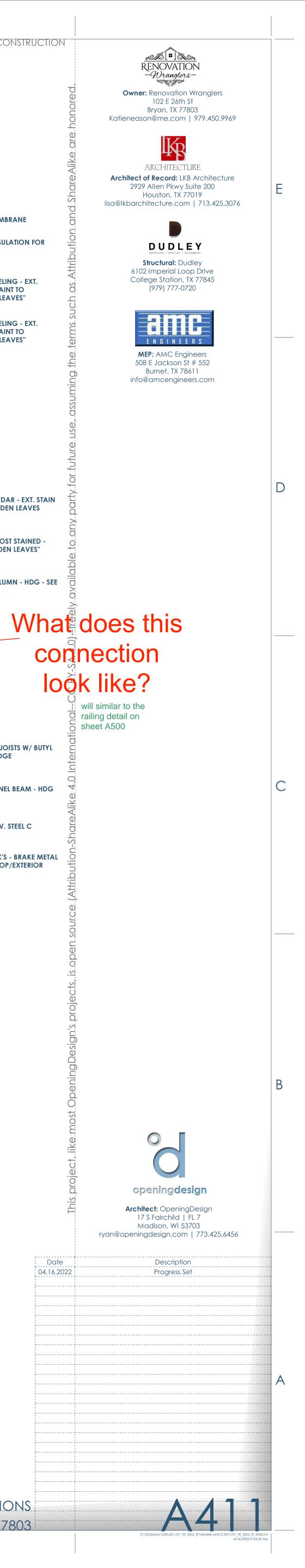


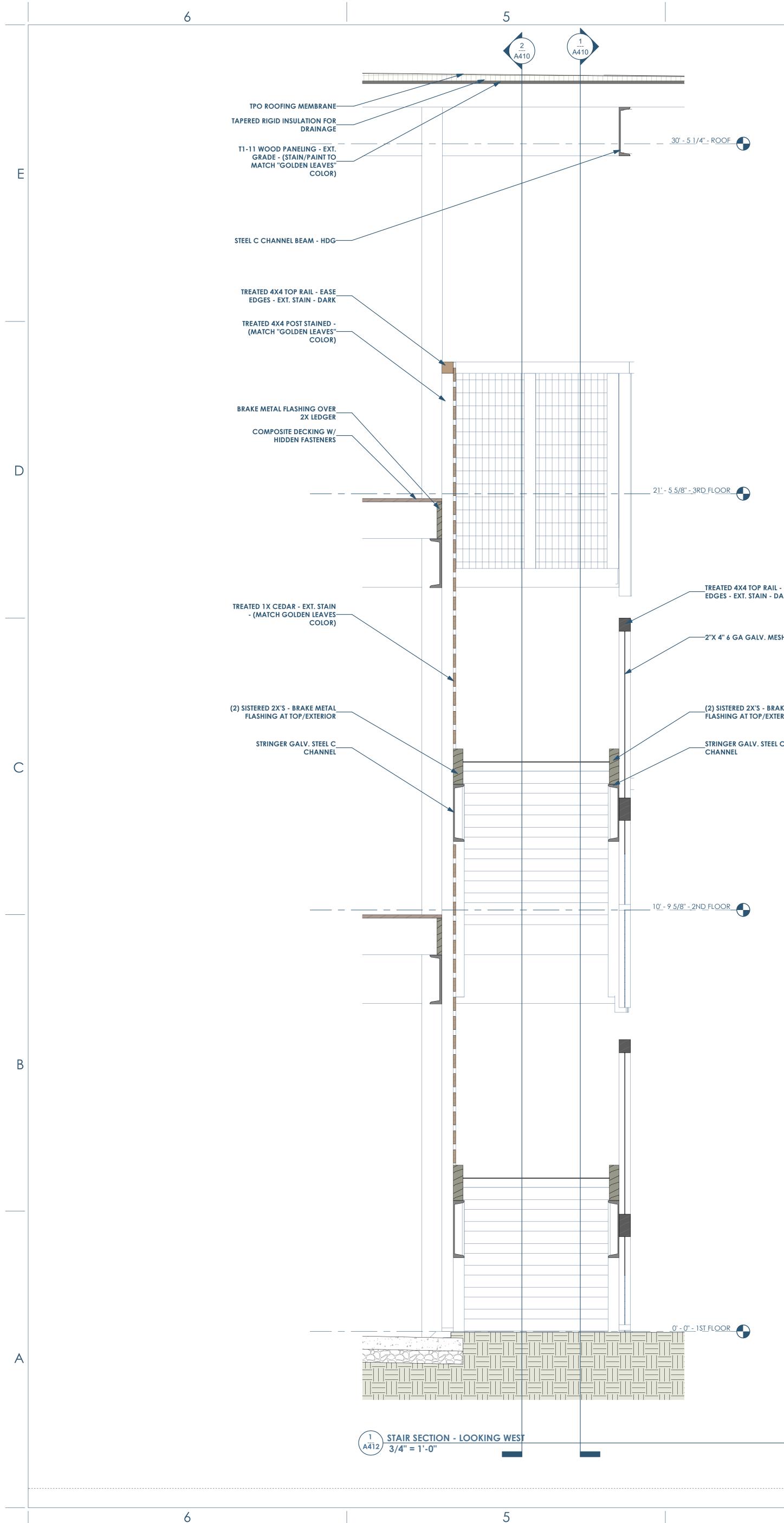
 Marco Polo - 101 W 33rd Stree
3











4	3

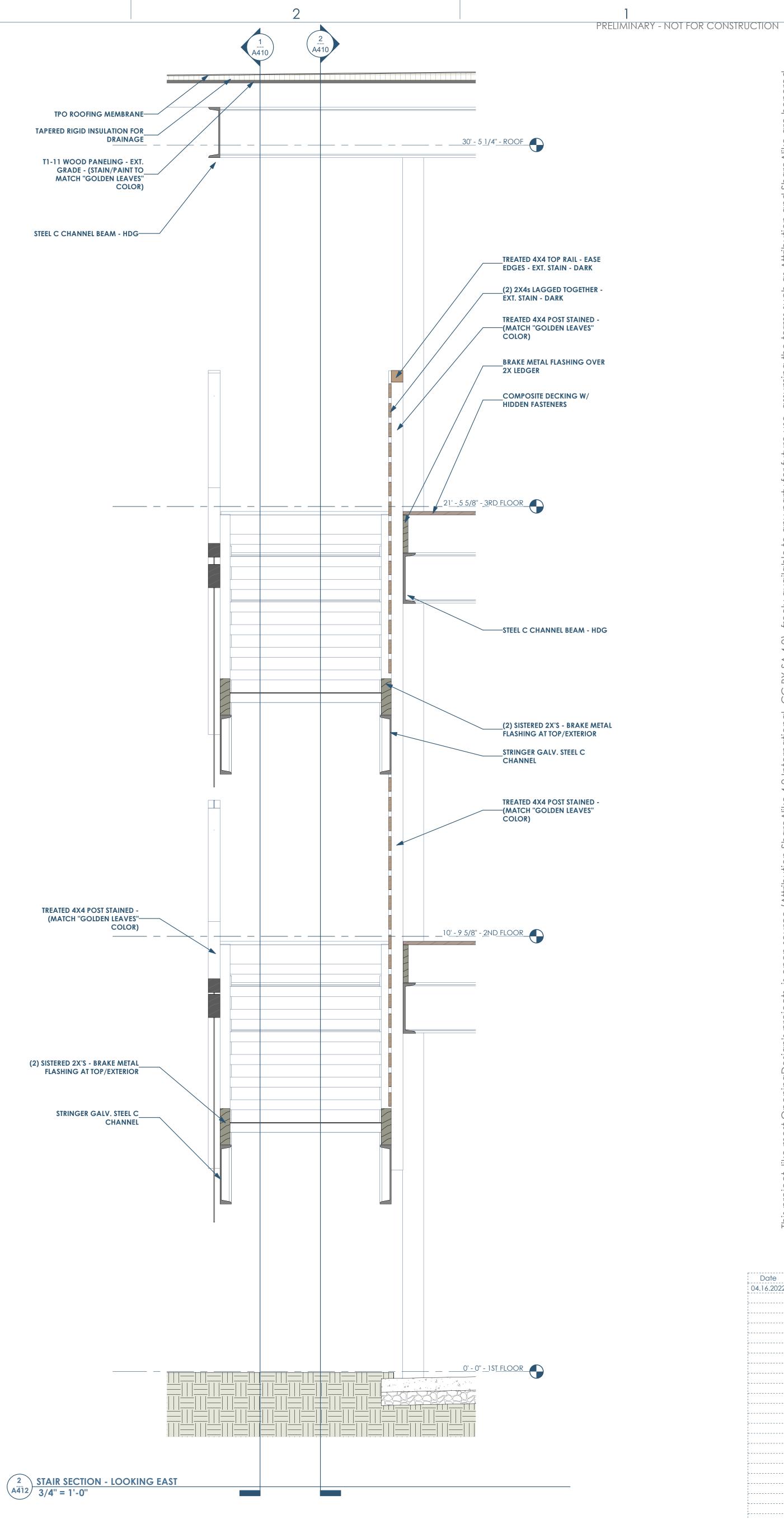
			т

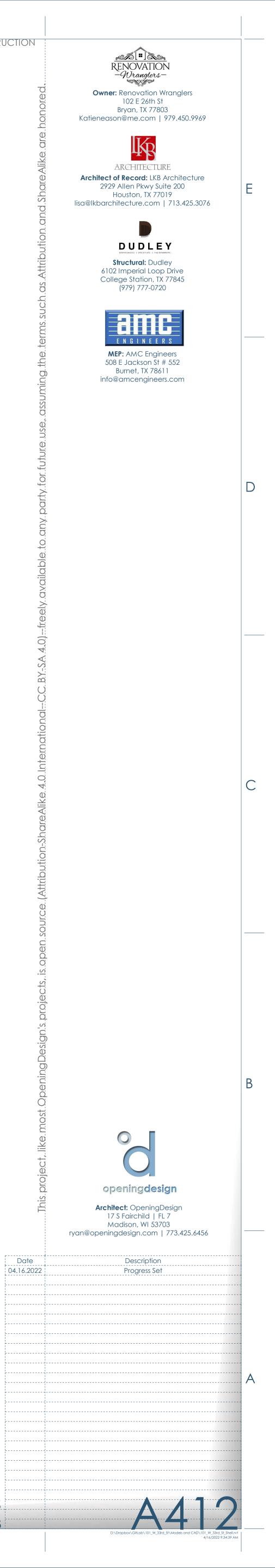


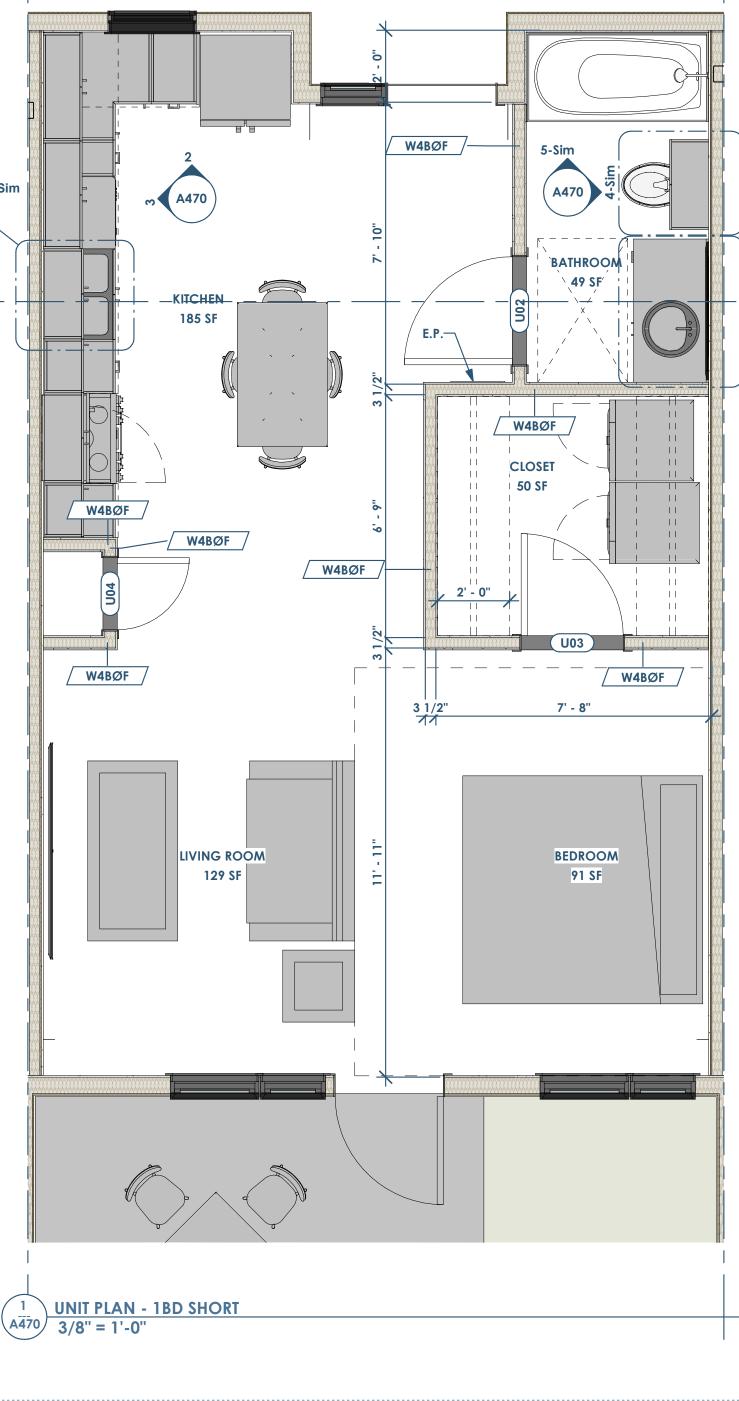
TREATED 4X4 TOP RAIL - EASE EDGES - EXT. STAIN - DARK

-2"X 4" 6 GA GALV. MESH PANEL

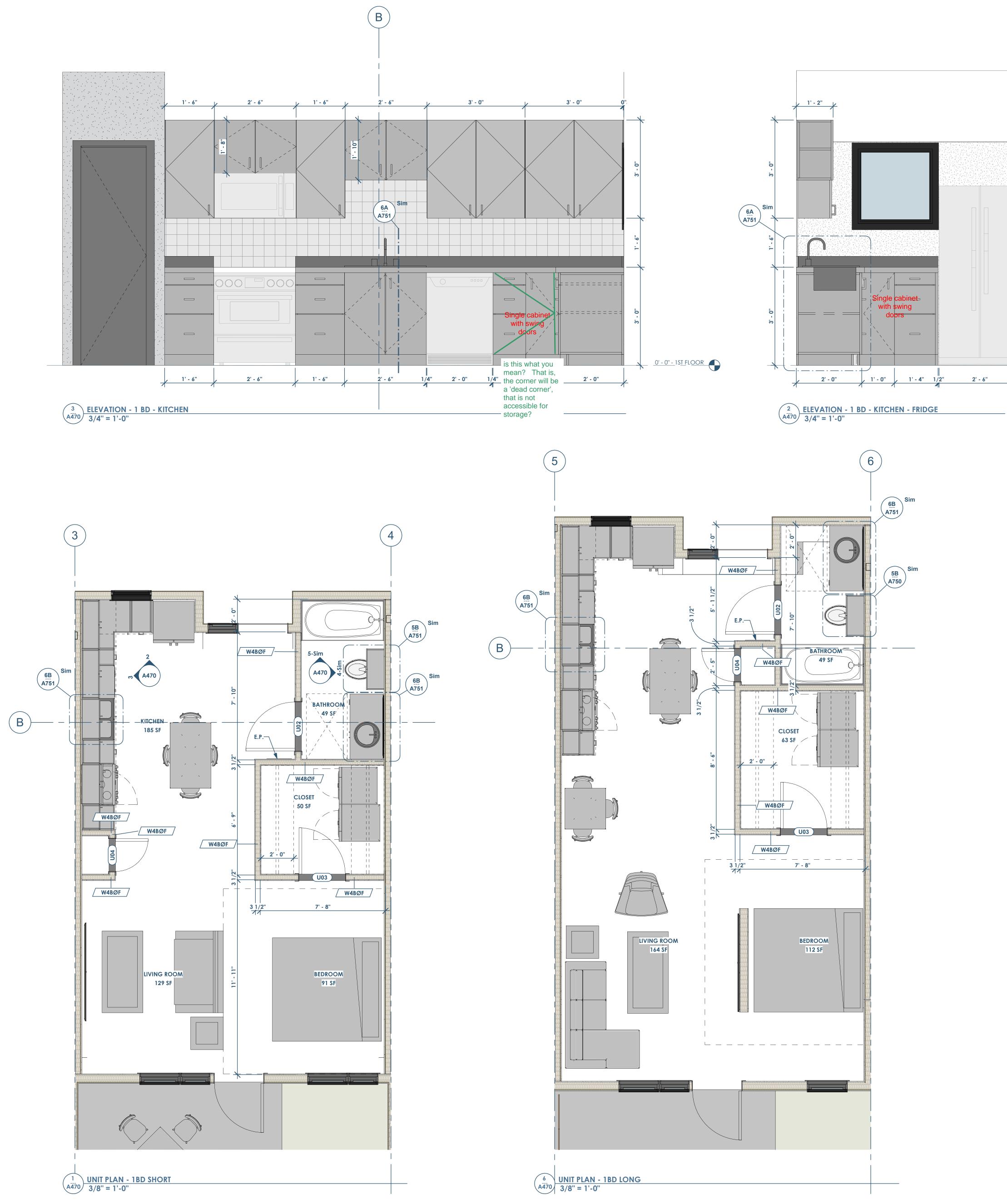
(2) SISTERED 2X'S - BRAKE METAL FLASHING AT TOP/EXTERIOR STRINGER GALV. STEEL C





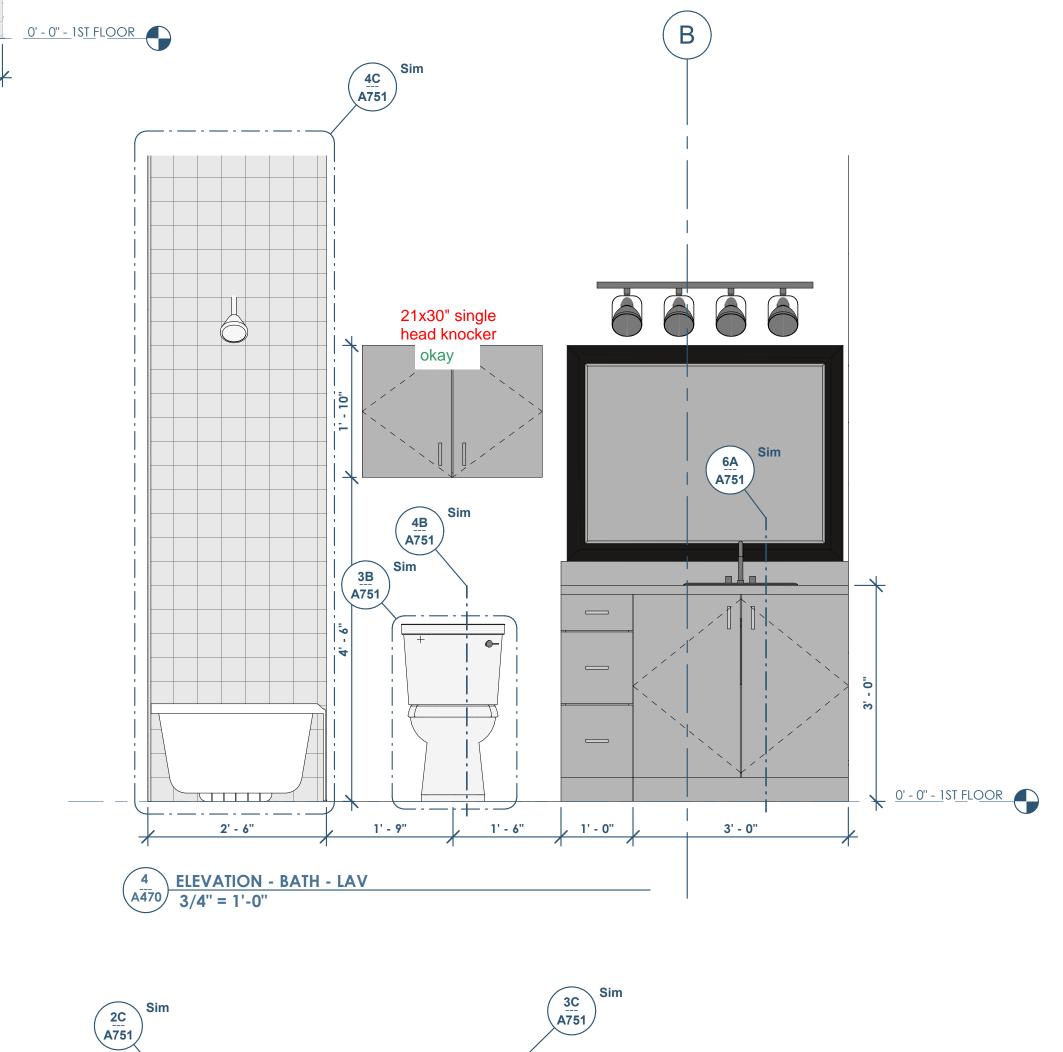


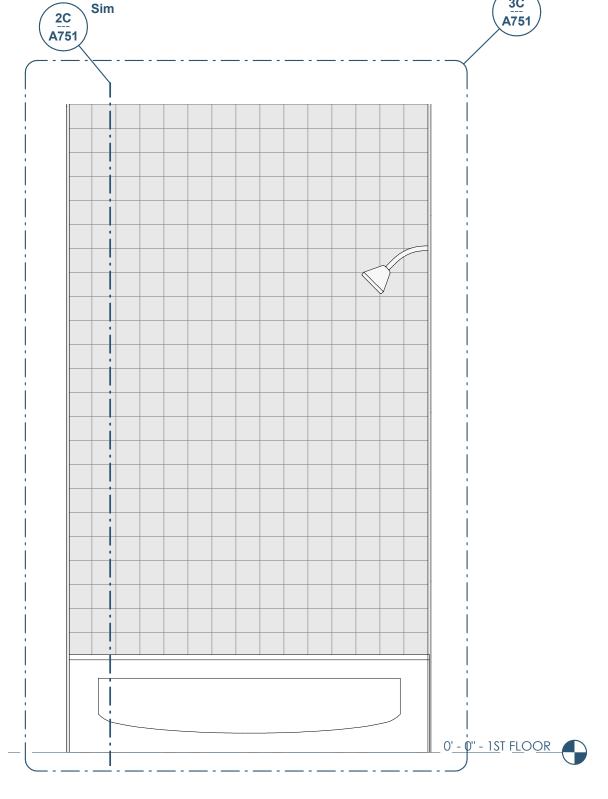




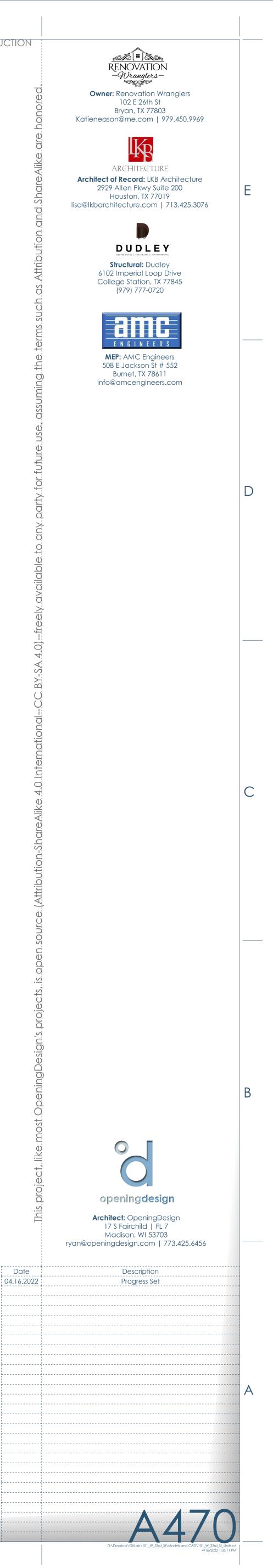


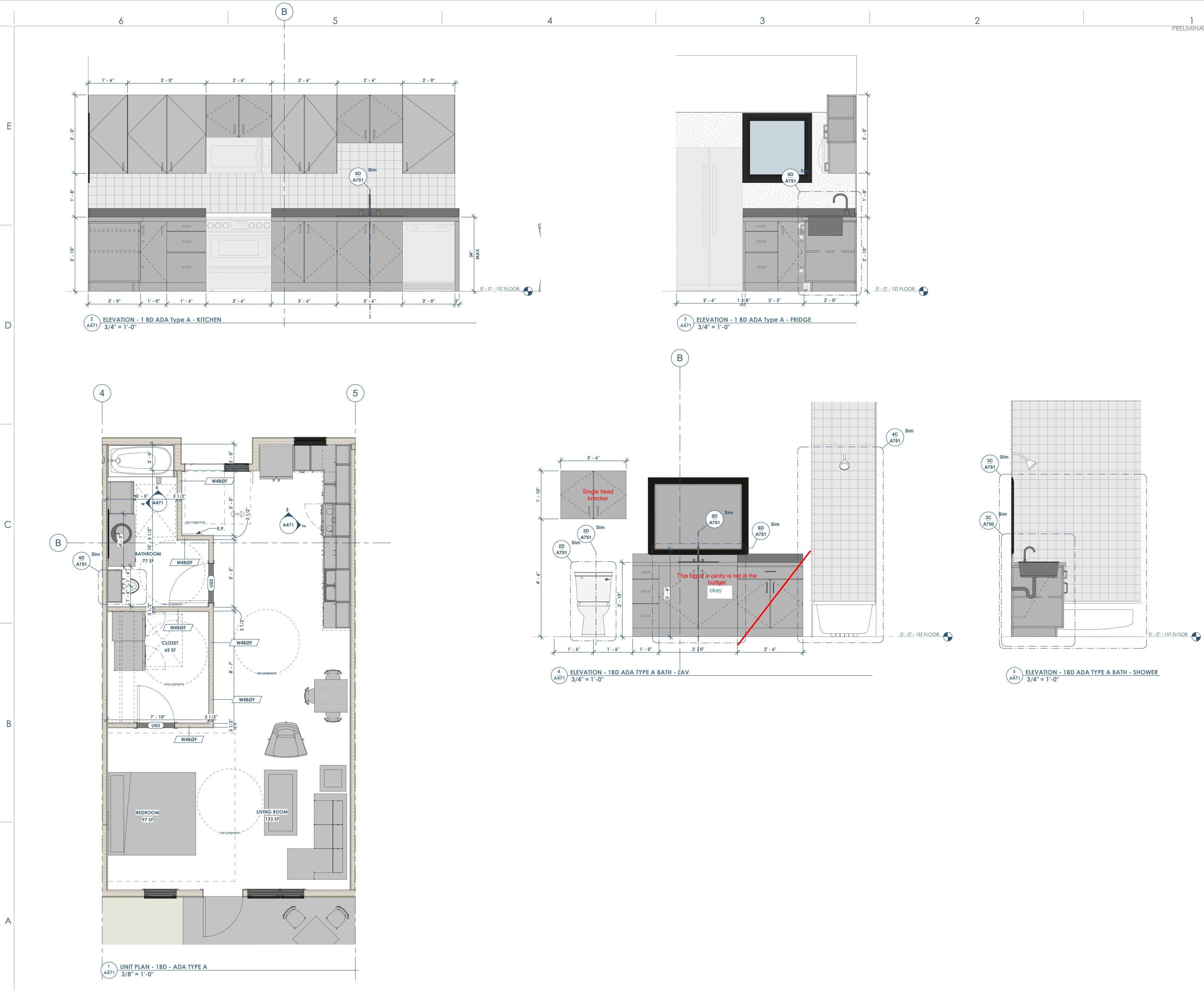


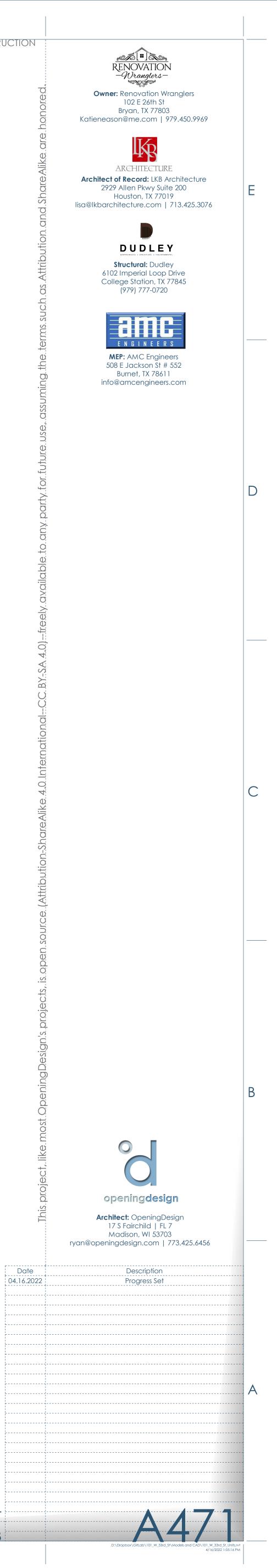


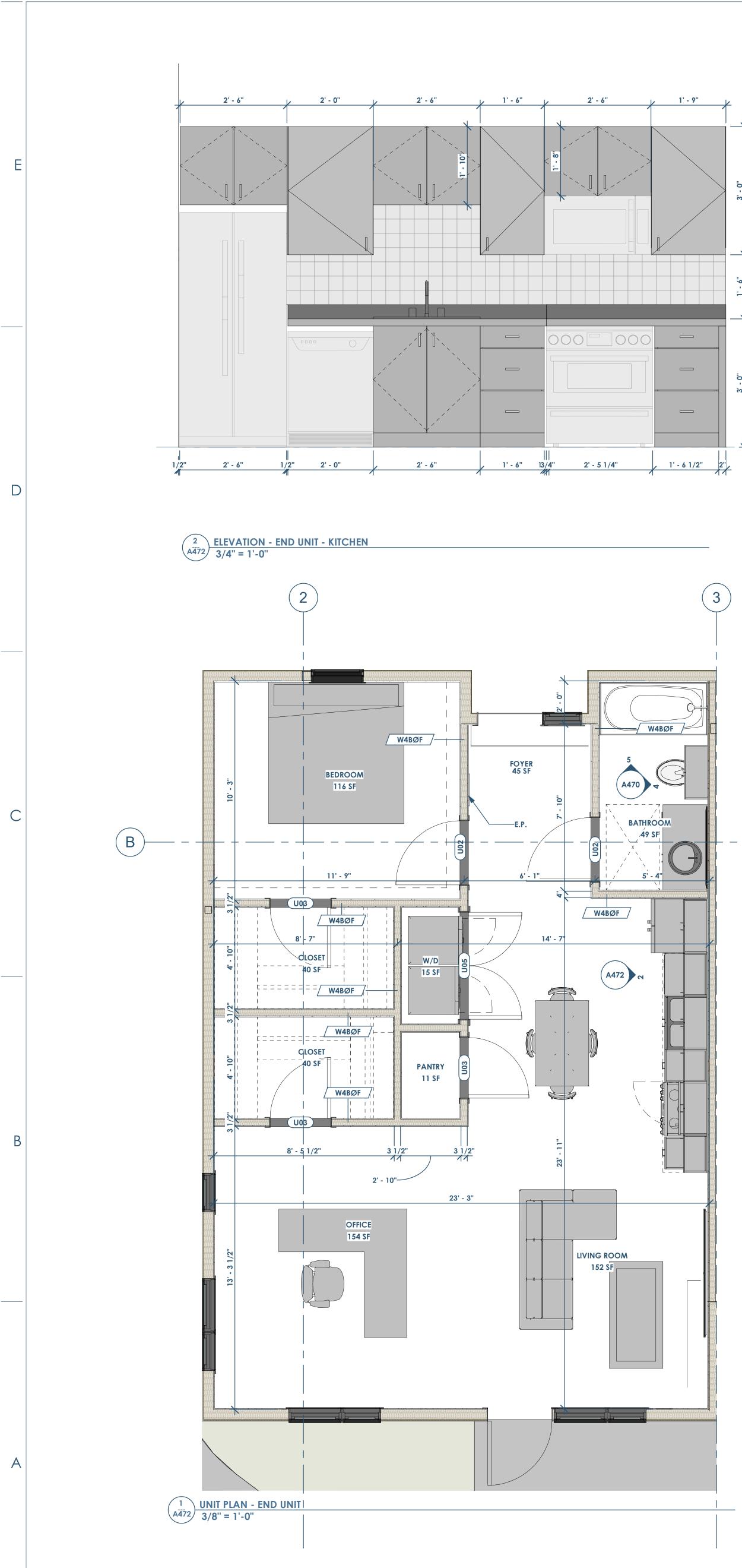


A470 ELEVATION - BATH - SHOWER 3/4" = 1'-0"

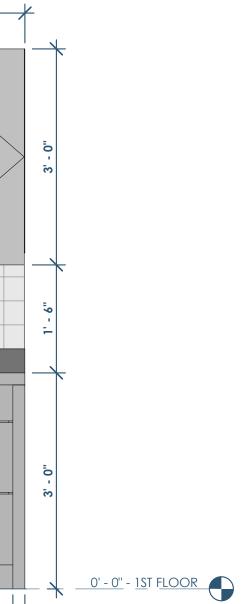




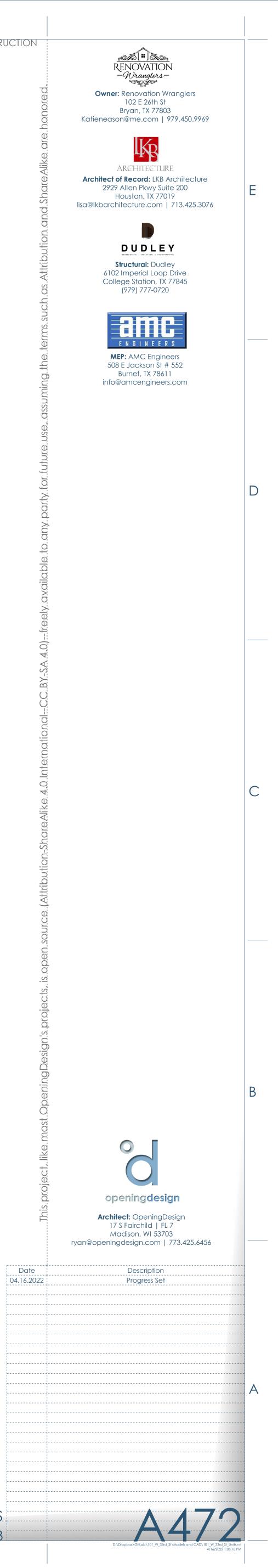


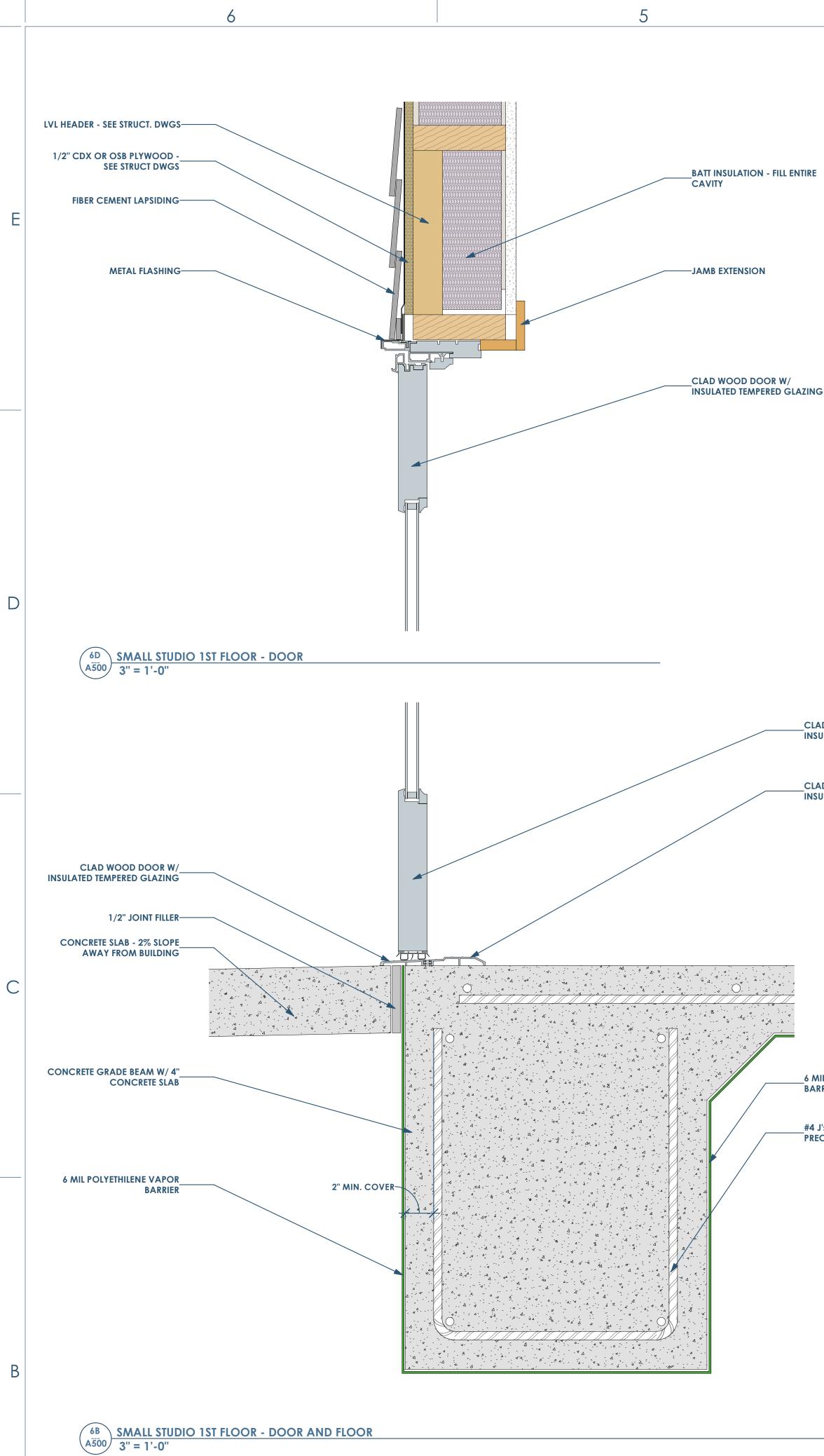


4	



END UNITS 101 W 33RD STREET - BRYAN, TX 77803





32 38

in. 40000

CAVITY

-2X WOOD STUD

SEE STRUCT DWGS

EXPOSED FASTENERS

-METAL DRIP FLASHING

UP/OVER PARAPET)

SEE STRUCT DWGS

DWGS

ASSEMBLY)

OC

—5/8" GYP. BD

-2X WOOD STUD

OC

CAVITY





2X10 LEDGER - STAIN BEFORE INSTALL **STRONG-DRIVE SDWS TIMBER** SCREW (EXTERIOR GRADE)

TPO ROOFING MEMBRANE

SIMPSON HUC46 JOIST HANGER WITH CONCEALED FLANGES ----HDG - UNPAINTED TAPERED RIGID INSULATION -FOR DRAINAGE

PLYTANIUM T1-11 SYP PLYWOOD PANEL SIDING - 4" GROOVES ---EXTERIOR GRADE

2X10 JOISTS SIMPSON HUC46 JOIST HANGER WITH CONCEALED FLANGES --

STRONG-DRIVE SDWS TIMBER SCREW (EXTERIOR GRADE)

METAL FLASHING-

HDG - UNPAINTED

CLAD WOOD DOOR W/ INSULATED TEMPERED GLAZING FIBER CEMENT LAPSIDING CLAD WOOD DOOR W/

INSULATED TEMPERED GLAZING RAINAGE WRAP - ASTM 2273 1/2" CDX OR OSB PLYWOOD -SEE STRUCT DWGS

2X WOOD STUD-

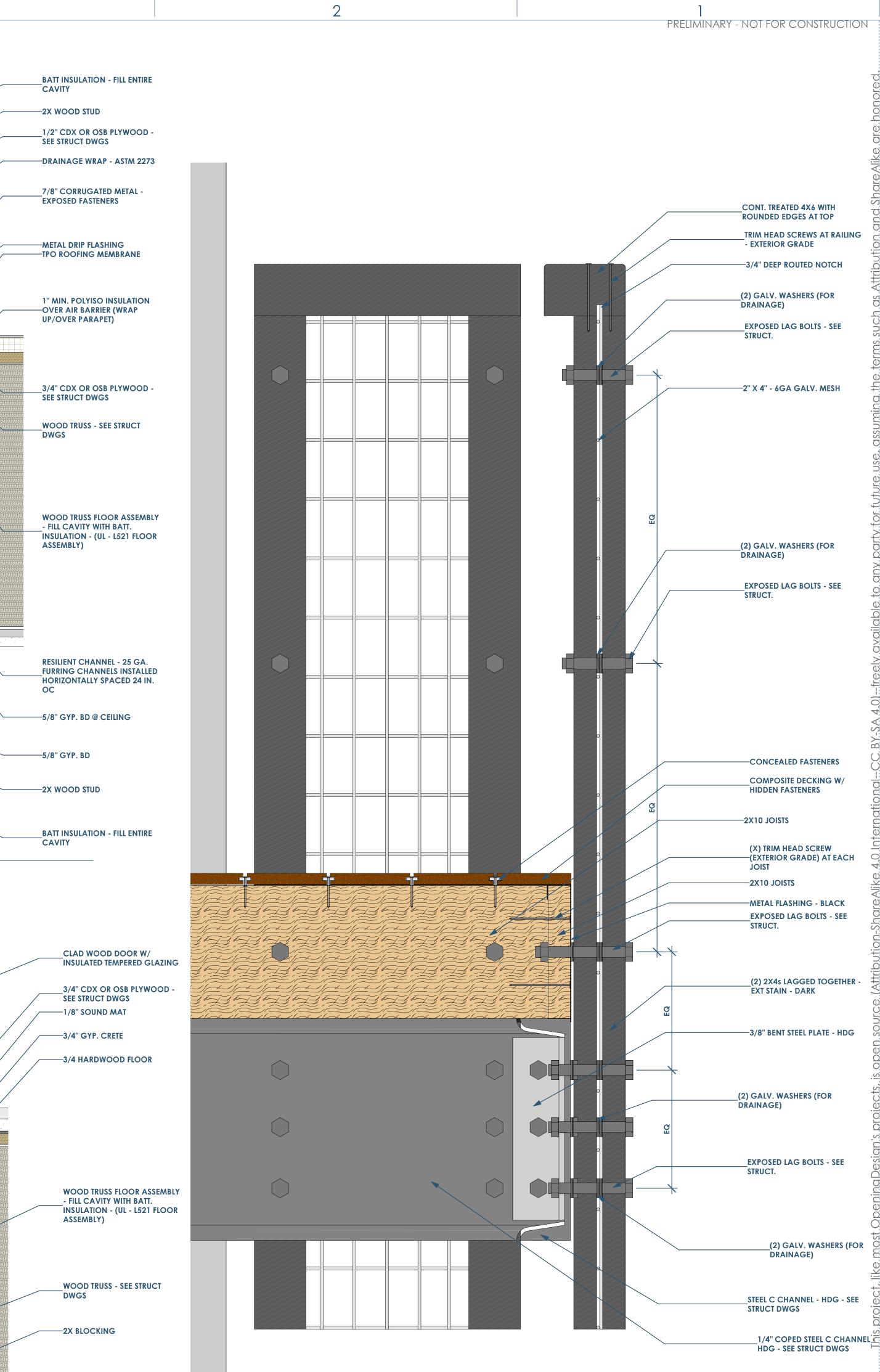
4C A500 SMALL STUDIO 3RD FLOOR - ROOF OVER BALCONY 3" = 1'-0"

METAL FLASHING _6 MIL POLYETHILENE VAPOR 2X10 LEDGER - STAIN BEFORE INSTALL CLAD WOOD DOOR W/ BARRIER INSULATED TEMPERED GLAZING SIMPSON HUC46 JOIST HANGER #4 J's @ 24" O.C. - GROUTED IN WITH CONCEALED FLANGES --_____3/4" CDX OR OSB PLYWOOD -HDG - UNPAINTED PRECAST KEYS AND CORES SEE STRUCT DWGS CONCEALED FASTENERS 2X10 JOISTS COMPOSITE DECKING W/ HIDDEN FASTENERS WOOD TRUSS FLOOR ASSEMBLY - FILL CAVITY WITH BATT. INSULATION - (UL - L521 FLOOR ASSEMBLY) WOOD TRUSS - SEE STRUCT DWGS SIMPSON HUC46 JOIST HANGER WITH CONCEALED FLANGES --HDG - UNPAINTED 2X BLOCKING STRONG-DRIVE SDWS TIMBER SCREW (EXTERIOR GRADE) METAL FLASHING-LVL RIM BOARD - SEE STRUCT. DWGS

> FIBER CEMENT LAPSIDING DRAINAGE WRAP - ASTM 2273-1/2" CDX OR OSB PLYWOOD -SEE STRUCT DWGS

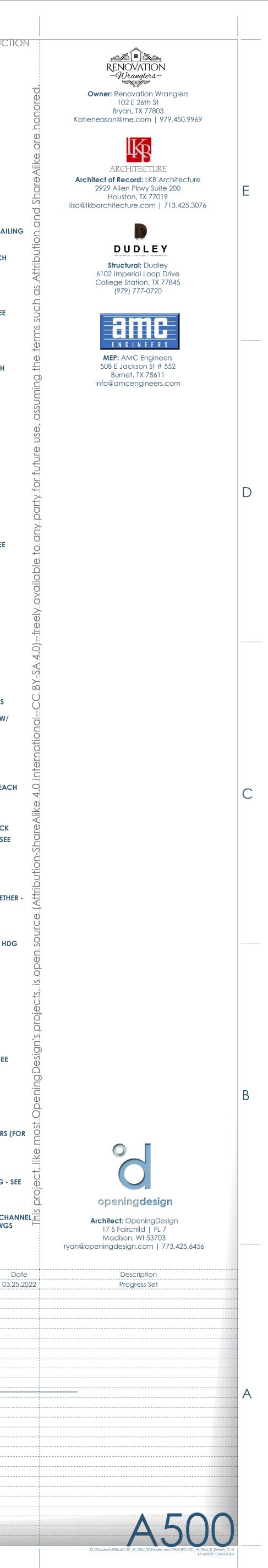
> > 6A A500 SMALL STUDIO 2ND FLOOR - DOOR AND BALCONY 3" = 1'-0"

3



RESILIENT CHANNEL - 25 GA. FURRING CHANNELS INSTALLED HORIZONTALLY SPACED 24 IN. A500 3" = 1'-0"

Date





BASE TRIM, AS SCHEDULED

2X BOTTOM PLATE

FIRE RATED SEALANT SEE TYP. TOP PLATE DETAIL XX/XXX FOR FASTENER

AT RIM BOARD AT EXT. WALL: (?)

?D FASTENERS AT EACH TRUSS

SEE TYP. TOP PLATE DETAIL XX/XXX FOR FASTENER

AT TRUSS INTO TOP PLATE: (?) ?D_

FASTENERS AT EACH TRUSS

RESILIENT CHANNEL - 25 GA. -

PERPENDICULAR TO TRUSSES

24" O.C. INSTALLED

5/8" GYP. BD OR CEMENT BACKERBOARD ON WET WALL SEAMS

PANEL

(2) 2X TOP PLATE STAGGER

BATT INSULATION - FILL ENTIRE

__2X WOOD STUDS BEYOND 16" O.C.

—1/2" CDX/OSB WALL SHEATHING

BACKERBOARD ON WET WALL

5/8" GYP. BD OR CEMENT

-2X BOTTOM PLATE

____3/4" OSB/CDX FLOOR

2X BRIDGING PER TRUSS MANU.

-WOOD TRUSSES - STRUCT. DWGS

TRUSS'S INTEGRATED SHEAR

ATTENUATION MAT

SHEATHING

REQUIREMENTS

2X WOOD STUDS BEYOND 16"_

WALL CAVITY - R20

PERPENDICULAR TO TRUSSES 5/8" GYP. BD. (PAINTED) AT

CEILING'

5/8" GYP. BD OR CEMENT

BACKERBOARD ON WET WALL

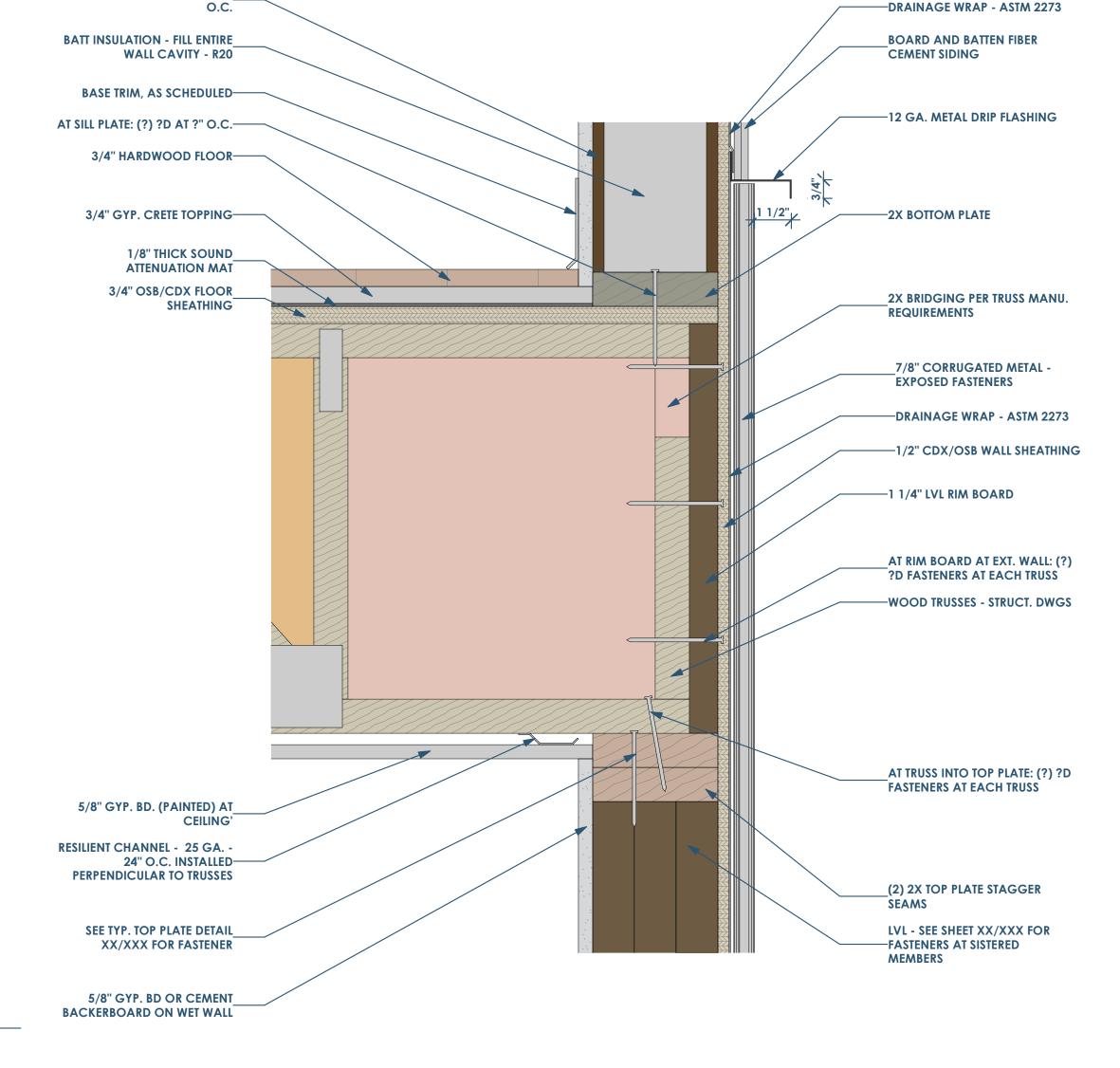
-1/2" CDX/OSB WALL SHEATHING 2X WOOD STUDS BEYOND 16"

0.C. BATT INSULATION - FILL ENTIRE WALL CAVITY - R20

1
A501SECTION DETAIL - PARTY WALL AND FLOOR TRUSS3" = 1'-0"

6





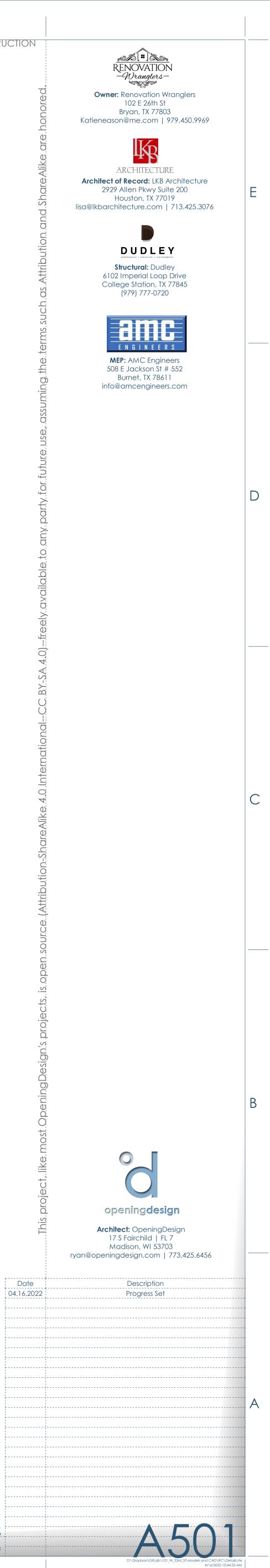
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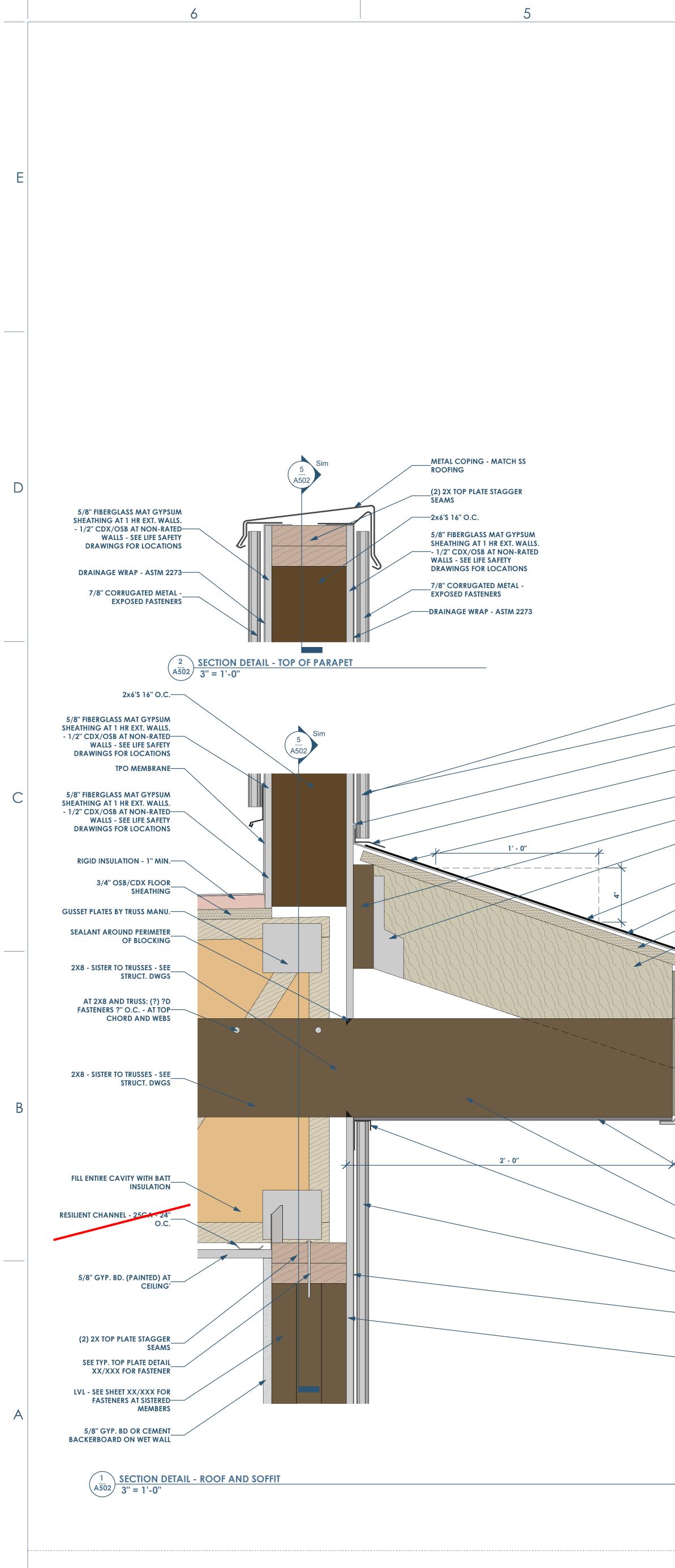
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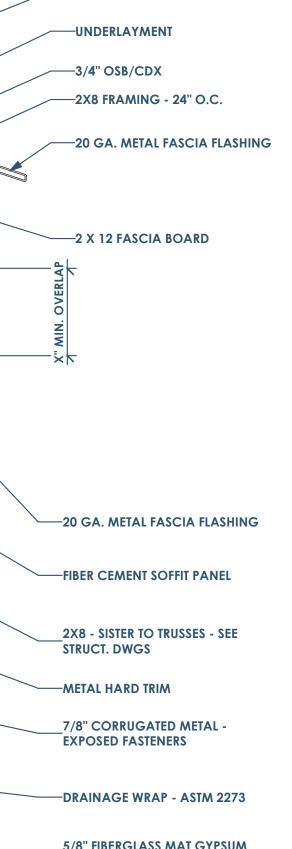
LARGE SCALE DETAILS **NAME OF PROJECT** - **ADDRESS OF PROJECT**

Date

· -----







_7/8" CORRUGATED METAL -

7/8" CORRUGATED METAL -

-DRAINAGE WRAP - ASTM 2273

EXPOSED FASTENERS

EXPOSED FASTENERS

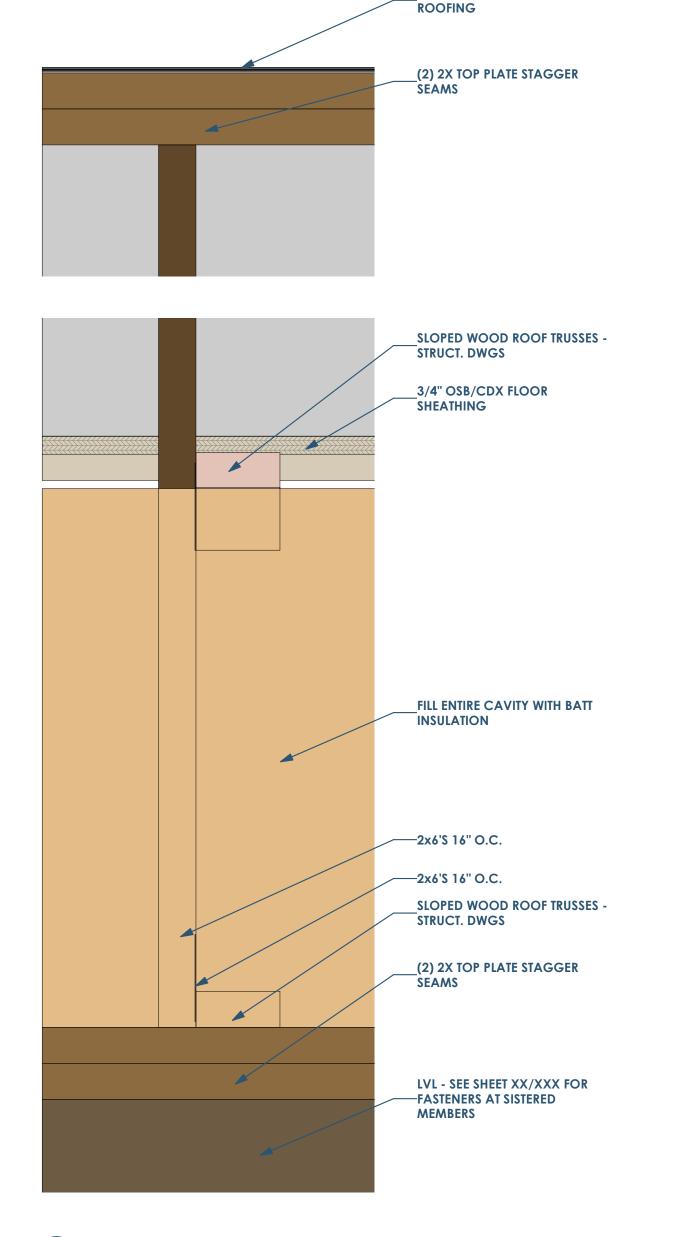
-METAL DRIP FLASHING

-2X LEDGER BOARD

-JOIST HANGER

-CLASS A ASPHALT SHINGLES

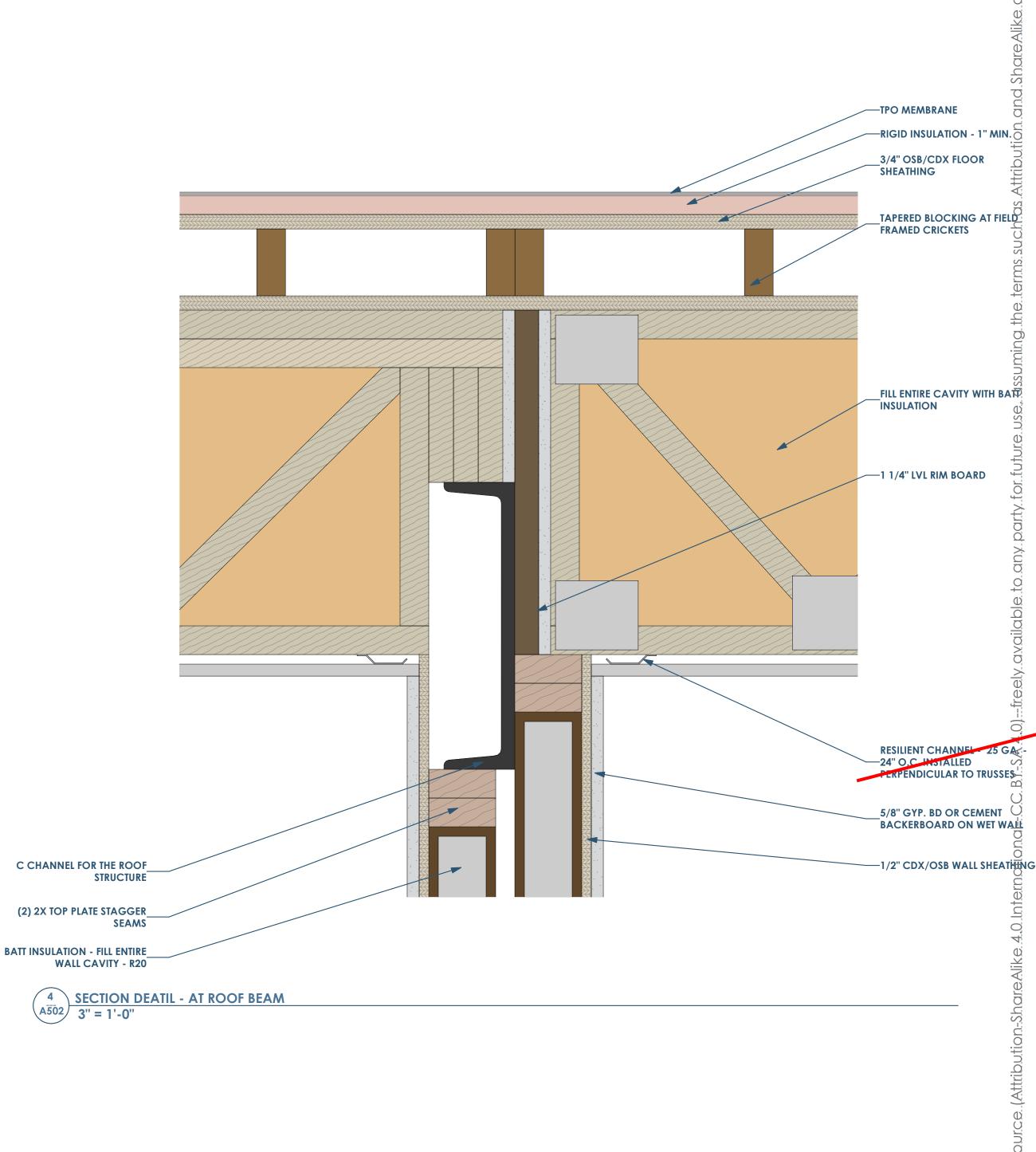
5/8" FIBERGLASS MAT GYPSUM SHEATHING AT 1 HR EXT. WALLS. —- 1/2" CDX/OSB AT NON-RATED WALLS - SEE LIFE SAFETY DRAWINGS FOR LOCATIONS

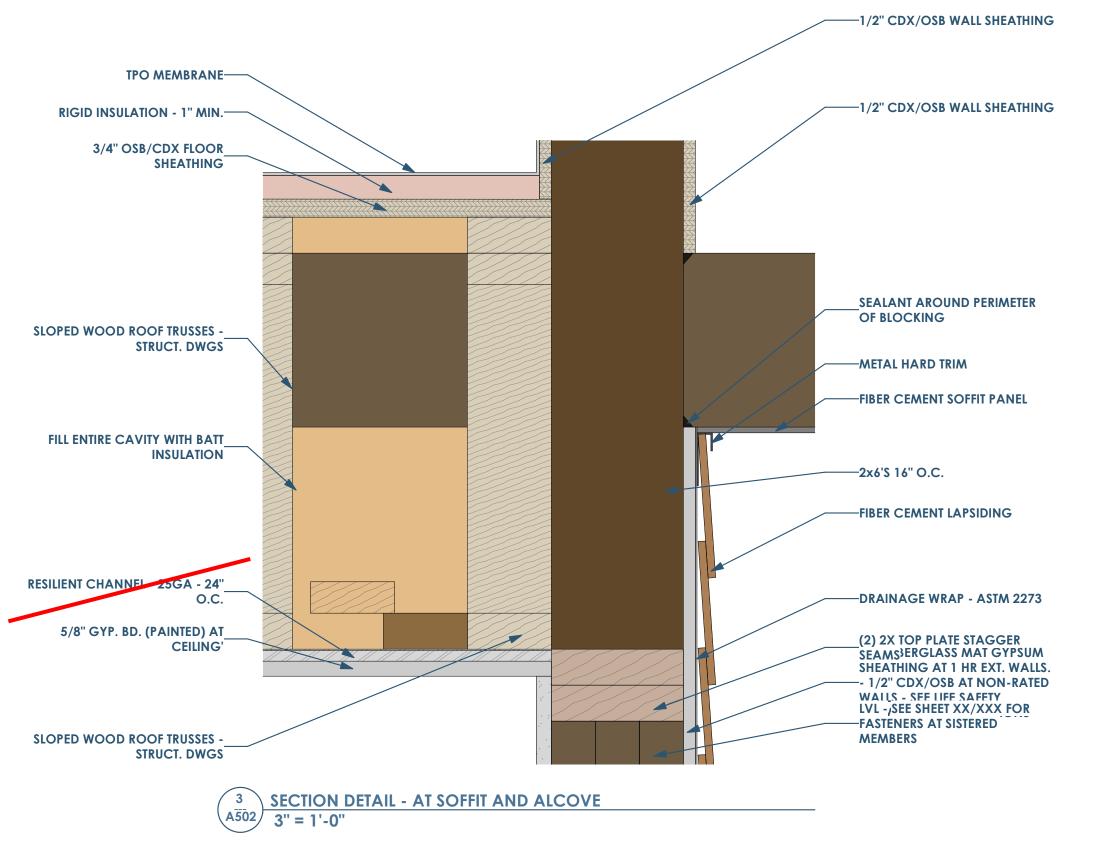


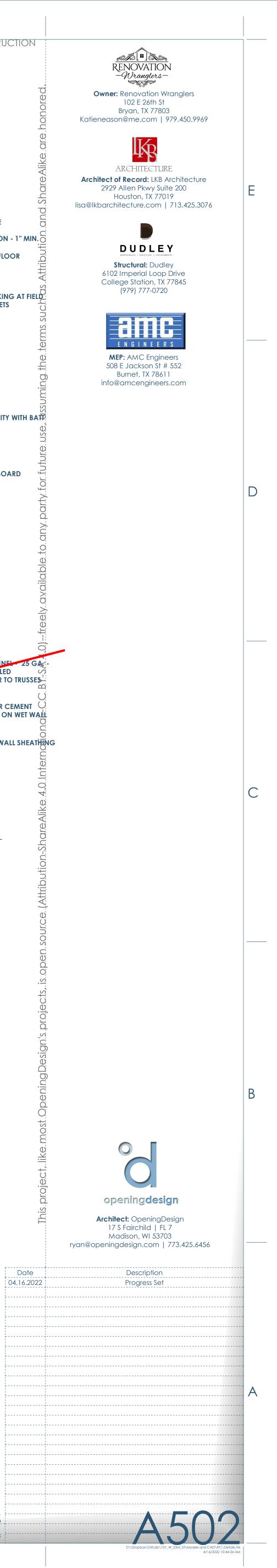
2

METAL COPING - MATCH SS

5 A502 004 - DETAILS (3" = 1'-0") - Dependent 1 3" = 1'-0"







SIMPLIFIED WINDOW SCHEDULE								
PHASE	TYPE							
CREATED	MARK	TYPE	COUNT	WIDTH	HEIGHT	TYPE COMMENTS		
ZAVE_2	W2	_Type Name_	1	2' - 0"	2' - 0"	Vinyl-clad wood awning window		
ZAVE_2	W1	_Type Name_	1	3' - 0''	4' - 0''			
ZAVE_2	W2	_Type Name_	1	2' - 0''	2' - 0"	Vinyl-clad wood awning window		
ZAVE_2	W3	_Type Name_	1	2' - 1 5/8"	6' - 4 7/8"	Vinyl-clad wood double hung window		
ZAVE_2	W4	_Type Name_	1			vinyl clad wood gliding window		
ZAVE 2	WO	_Type Name_	1			@master-bath; electric fresh air skylight		

5

					DOC	OR SCHED	ULE - TYPE		
Count	Type Mark	Туре	Width	Height	Fire Rating	Door Type	Door Material	Frame Type	Frame Material
24	U02	U02 - SINGLE - FLUSH - 3-0 x 6-8	3' - 0''	6' - 8''		B1	WD	A	WD
30	U03	U02 - SINGLE - FLUSH - 2-10 x 6-8	2' - 10''	6' - 8''		B1	WD	A	WD
11	U04	U04 - SINGLE - FLUSH - 2-0 x 6-8	2' - 0''	6' - 8''		B1	WD	A	WD
6	U05	U05 - DOUBLE - FLUSH - 5-0 x 6-8	5' - 0''	6' - 8''	OHR	B2	WD	A	WD

D

С

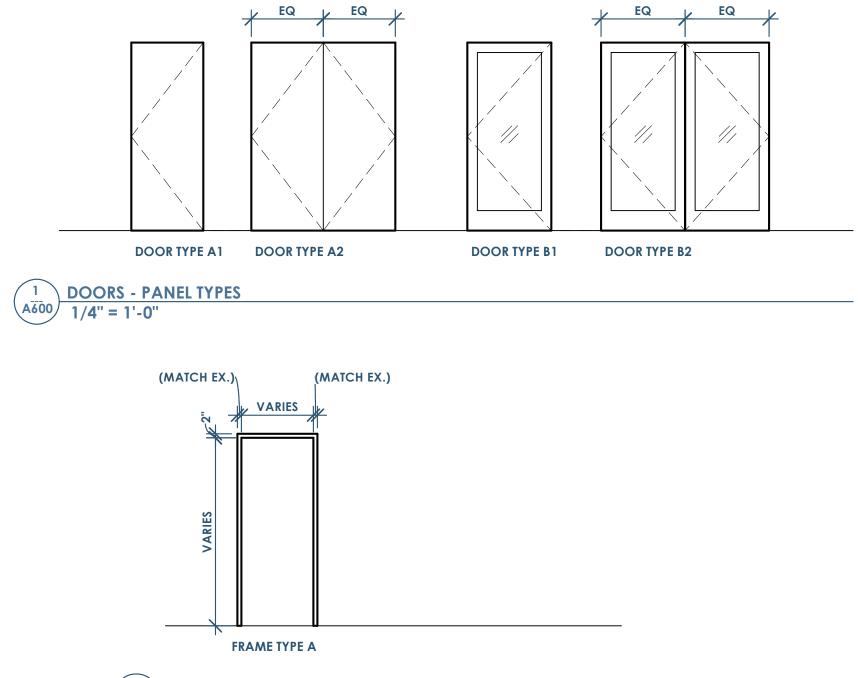
P

6

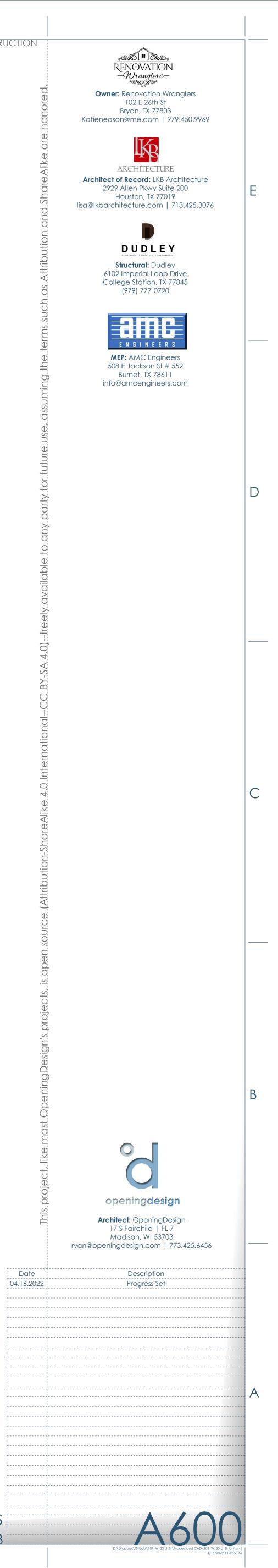
Λ	
4	

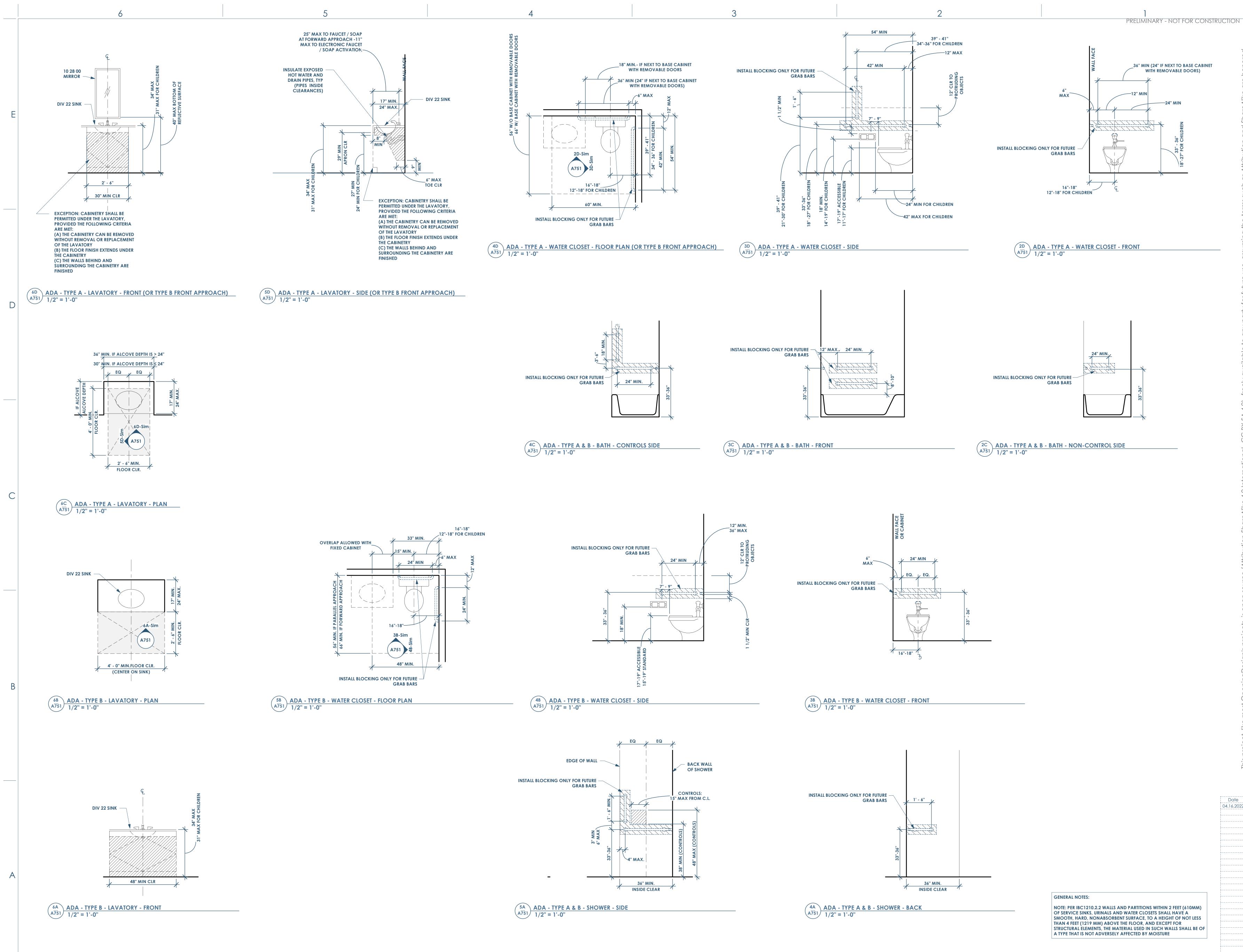
Type Comments DS_Hardware_ Set_(SFT)

4

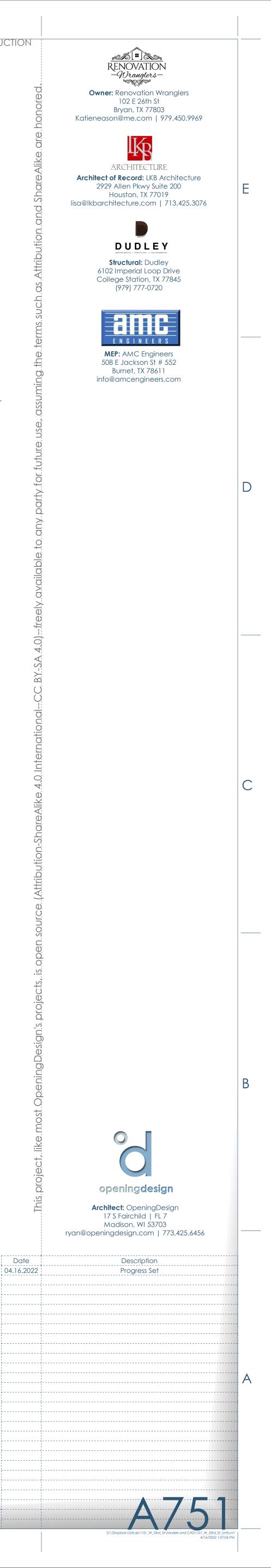




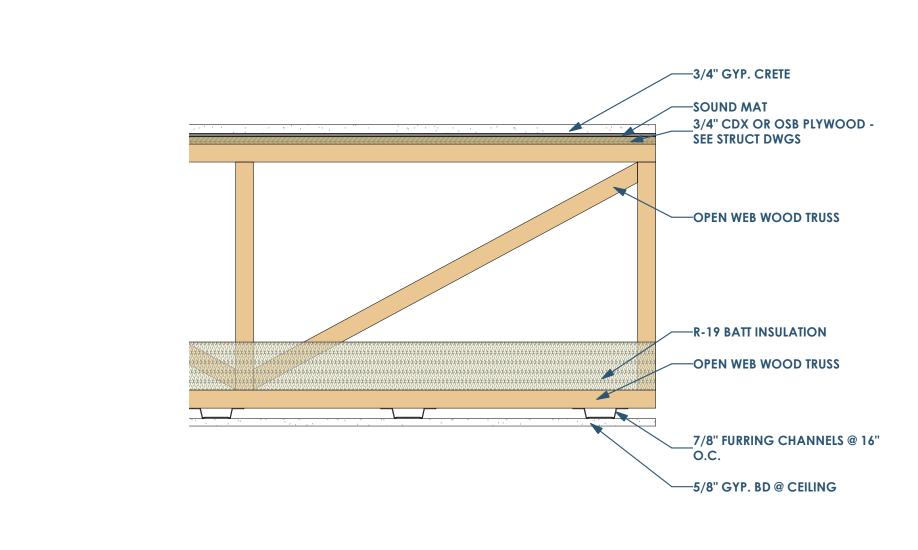




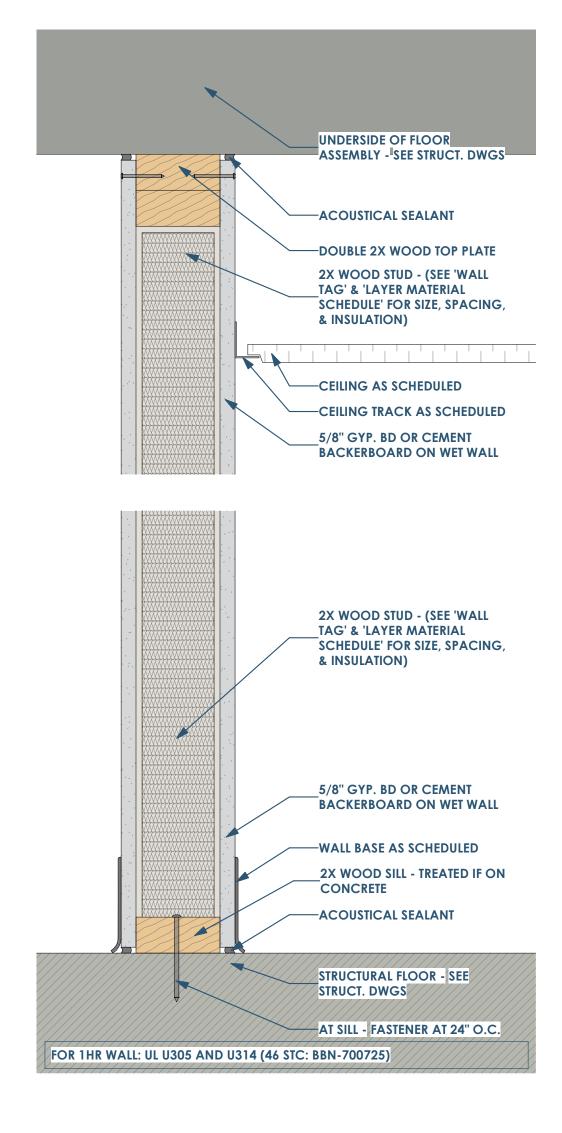
3











6A A800 <u>W()()()F FULL HEIGHT PARTITION</u> 3" = 1'-0"

6

3

PARTITION TAG NOMENCLATURE

- <u>1ST LETTER = CORE MATERIAL</u> • W= WOOD
- M=METAL • C=CONCRETE
- B=MASONRY BLOCK
- <u>2ND LETTER = SIZE OF CORE</u>
 WOOD: NOMINAL STUD SIZES (EX: 4 = 3 1/2") • METAL STUD: (EX 358 = 3 5/8")
- CONCRETE: ACTUAL WALL THICKNESS (EX: 8 = 8") • MASONRY: NOMINAL BRICK MODULES (EX: 8 = 7 5/8")

0

• <u>3RD LETTER = LAYER MATERIAL</u>

	LAYER MATERIAL (3RD LETTER)							
	LAYER 3	LAYER 2	LAYER 1	CORE	LAYER 1	LAYER 2	LAYER 3	
A=	-	-	5/8" GYP. BD.	STUDS 16" O.C. (20 GA. IF METAL)	5/8" GYP. BD.	-	-	
B=	-	-	5/8" GYP. BD.	STUDS 16" O.C. (20 GA. IF METAL) BATT INSULATION	5/8" GYP. BD.	-	-	
C=	-	-	5/8" GYP. BD.	STUDS 16" O.C. (20 GA. IF METAL)	(PROVIDE 1/4" AIR GAP IF AGAINST CONCRETE OR MASONRY)	-	-	
D=	-	-	5/8" GYP. BD.	STUDS 16" O.C. (20 GA. IF METAL) BATT INSULATION	(PROVIDE 1/4" AIR GAP IF AGAINST CONCRETE OR MASONRY) - USE TREATED WOOD STUDS IF IN CONTACT WITH CONCRETE/MASON RY	-	-	
E=								
F=		5/8" GYP. BD.	5/8" GYP. BD.	STUDS 16" O.C. (20 GA. IF METAL) BATT INSULATION	1/2" RESILENT CHANNEL	5/8" GYP. BD.	5/8" GYP. BD.	
G=	5/8" GYP. BD.	5/8" GYP. BD.	SHEATHING - SEE STRUCT. DWGS	STUDS 16" O.C. (20 GA. IF METAL)	SHEATHING - SEE STRUCT. DWGS	5/8" GYP. BD. OVER 1/2" RESILENT CHANNEL	5/8" GYP. BD.	
Q=				CONCRETE - VERT: NO. X AT X" O.C HORZ: NO. X AT X" O.C.				
R=	-	-	-	MASONRY - VERT: NO. X AT X" O.C HORZ: NO. X AT X" O.C.	-	-	-	

• 4TH NUMBER: FIRE RATING • Ø=0 HOUR

• 1=1 HOUR • 2=2 HOUR

• 3=3 HOUR • 5=1/2 HOUR

• <u>5TH (AND BEYOND) LETTERS = MODIFIERS</u>

• A=PARTIAL HEIGHT PARTITION (WALL LAYER 1, 2, & 3 TO EXTEND 6" ABOVE FINISHED CEILING HEIGHT) • B=PARTIAL HEIGHT WALL (WALL LAYER 1, 2, & 3 TO TERMINATE AT OR <u>BELOW</u> HUNG CEILING)

• D=FULL HEIGHT TO UNDERSIDE OF STRUCTURAL DECK/SHEATHING (CORE AND WALL LAYER 1, 2, & 3 TO TERMINATE AT STRUCTURAL DECK) • F=FULL HEIGHT TO THE BOTTOM OF STRUCTURE

• K=KNEE WALL PARTITION

• R=<u>FURRED</u> OUT WALL

• EXAMPLE: M358BØAR

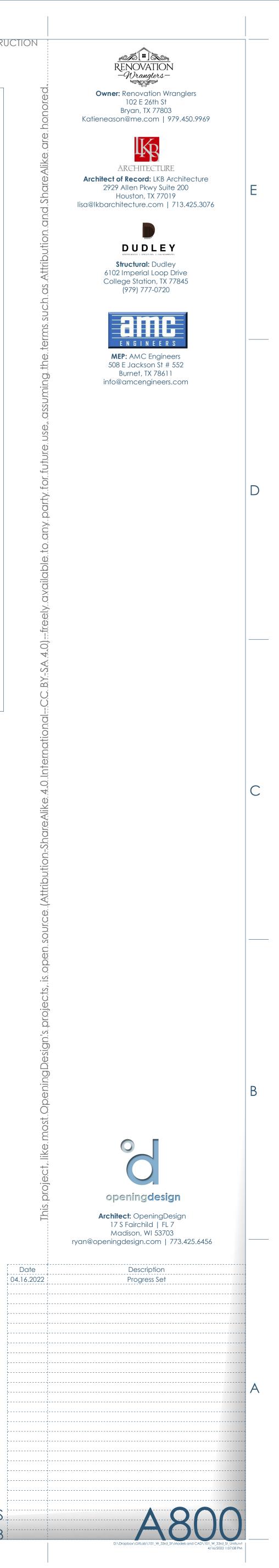
• M=METAL • 358=3 5/8" METAL STUD

• B=20 GA METAL STUDS 16" O.C. W/ BATT INSULATION • Ø=0 HOUR

2

• A=PARTIAL HEIGHT PARTITION (WALL LAYER 1, 2, & 3 TO EXTEND 6" <u>ABOVE</u> FINISHED CEILING HEIGHT)

• R=<u>FURRED</u> OUT WALL



A. BUILDING CODE VERSION:.

B. AUTHORITY HAVING JURISDICTION ...

design Criteria

2018 INTERNATIONAL BUILDING CODE W. LOCAL AHJ AMENDMENTS

- WEB STEEL LOISTS)
- STRUCTURAL DEFERRED SUBMITTALS ON THIS PROJECT INCLUDE: C. METAL PLATE CONNECTED WOOD TRUSSES

D. AWNINGS, CANOPIES, LOUVERS, ETC.

E. METAL BUILDING SYSTEM

ENERAL CONDITIONS

OF CONSTRUCTION. 6. ALL WORK SHALL CONFORM TO OSHA STANDARDS. OR SPECIFIED IN SIMILAR CONDITIONS STRICTEST REQUIREMENTS, AS INDICATED BY THE ENGINEER, SHALL GOVERN. PROTECTIVE COATING FOR CONCRETE, SEALANTS, CAULKED JOINTS, EXPANSION JOINTS, CONTROL JOINTS, SPALLS AND CRACKS IN CONCRETE, AND PRESSURE WASHING OF EXPOSED STRUCTURAL ELEMENTS EXPOSED TO A SALT ENVIRONMENT OR OTHER HARSH CHEMICALS. 15. THE STRUCTURAL ENGINEER'S ROLE DURING CONSTRUCTION WORK OF THE CONTRACTOR.

CONTRACTOR QUALIFICATION

<u>FUTURE EXPANSION</u>

REQUEST FOR INFORMATION (RFI)

1.	RFI	I'S MUST INCLUDE A TRANSMITTAL
	Α.	RFI NUMBER
	Β.	RFI CATEGORY:
		a. REQUEST FOR SUBSTITUTION
		b. CORRECTIVE REPAIR
		c. ADDITIONAL INFORMATION
		d. DISCREPANCY BETWEEN CC
	C.	DATE SUBMITTED
	D.	DATE RESPONSE NEED BY
	E.	SUBMITTED BY (INCLUDE EMAIL
	F.	RFI DESCRIPTION INCLUDING:
		a. SHEET NUMBER, DETAIL AND
		b. SKETCHES IF APPLICABLE

c. PHOTOS IF APPLICABLE

- SUBMITTAL LIST AND SCHEDULE LIST SHALL BE UPDATED AND REVISED AS THE JOB PROGRESSES. 2. SUBMITTAL REQUIREMENTS B. ALL SUBMITTALS MUST INCLUDE A TRANSMITTAL SHEET WHICH INDICATES: CONCRETE SUBMITTAL) b. BRIEF DESCRIPTION OF SUBMITTAL CONTENTS C. DATE ISSUED d. REQUESTED RETURN DATE e. ISSUING PARTY INCLUDING NAME, PHONE NUMBER AND EMAIL THE STRUCTURAL ENGINEER
- A. NO EXCEPTIONS
- B. EXCEPTIONS NOTED C. REVISE AND RESUBMIT

D. NOT REVIEWED

E. FOR INFORMATION ONLY F. IMPACT TO STRUCTURE

INSPECTIONS

A. FOUNDATION INSPECTION: B. CONCRETE SLAB AND UNDER-FLOOR INSPECTION: THE SUBFLOOR. C. FRAME INSPECTION

DRAWING INTERPRETATION

DRAWING VIEWS LABELED AS TYPICAL

REINFORCING STEEL - 03 20 00

- DETAILING MANUAL) DURING THE PLACING OF CONCRETE.
- CONCRETE COVER NOT SPECIFICALLY DETAILED ON THE DRAWINGS SHALL BE IN ACCORDANCE WITH ACI 318. 7. COVERAGE: THE FOLLOWING SHALL BE THE MINIMUM REINFORCEMENT CONCRETE COVERAGE (INCLUDING TENDONS): B. CONCRETE EXPOSED TO EARTH OR WEATHER:
- a. NO. 6 AND LARGER b. NO. 5 BAR AND SMALLER

. DEAD LOADS:	
	CONSTRUCTION AND FIXED SERVICE EQUIPMENT. ASSUMPTIONS FOR WALL AND ROOF ASSEMBLIES ARE SHOWN BELOW:
a. METAL PANELS - 3 PSF	
b. CURTAIN WALLS - 10 PSF	
c. STONE / BRICK VENEER - 40 PSF	
d. ADHERED STONE/BRICK - 10 PSF	
e. SINGLE PLY MEMBRANE ROOF WITH INSULATION ASSEMBLY - 10 PSF	
B. EQUIPMENT	
c. FOR EQUIPMENT NOT INDICATED ON THE STRUCTURAL DRAWINGS IN	I WHICH THE WEIGHT OF THE EQUIPMENTS DIVIDED BY ITS SURFACE AREA EXCEEDS THE INDICATED LIVE LOAD FOR THE LOCATION, THE
CONTRACTOR SHALL NOTIFY THE EOR PRIOR TO SUBMISSION OF SHO	
C. HANGING CEILING AND MECHANICAL LOADS: AN ALLOWANCE OF 5	PSF HAS BEEN MADE FOR HANGING CEILING AND MECHANICAL EQUIPMENTS SUCH AS DUCT WORK AND SPRINKLER PIPES.
. LIVE LOADS:	
	1.5 × SAME AS OCCUPANCY SERVED
B. PRIVATE ROOMS AND CORRIDORS SERVING THEM	
C. PUBLIC ROOMS AND CORRIDORS SERVING THEM	
D. STAIRS AND EXITS	100 PSF 300 LB
. ROOF LIVE LOAD a. ORDINARY, FLAT, PITCHED AND CURVED UNOCCUPIED ROOFS:	20 PSF 300 LB
 ORDINARY, FLAT, PITCHED AND CURVED UNOCCUPIED ROOFS: SNOW LOAD: 	
a. ORDINARY, FLAT, PITCHED AND CURVED UNOCCUPIED ROOFS: SNOW LOAD: A. GROUND SNOW LOAD, Pg:	20 PSF, 300 LB
a. ORDINARY, FLAT, PITCHED AND CURVED UNOCCUPIED ROOFS: SNOW LOAD: A. GROUND SNOW LOAD, Pg:	XX PSF
a. ORDINARY, FLAT, PITCHED AND CURVED UNOCCUPIED ROOFS: SNOW LOAD: A. GROUND SNOW LOAD, Pg: WIND: A. ULTIMATE DESIGN WIND SPEED Vult:	
a. ORDINARY, FLAT, PITCHED AND CURVED UNOCCUPIED ROOFS: SNOW LOAD: A. GROUND SNOW LOAD, Pg: WIND: A. ULTIMATE DESIGN WIND SPEED Vult: B. NOMINAL DESIGN WIND SPEED, Vasd:	XX PSF
a. ORDINARY, FLAT, PITCHED AND CURVED UNOCCUPIED ROOFS: SNOW LOAD: A. GROUND SNOW LOAD, Pg: WIND: A. ULTIMATE DESIGN WIND SPEED Vult: B. NOMINAL DESIGN WIND SPEED, Vasd: C. WIND EXPOSURE CATEGORY:	XX PSF
a. ORDINARY, FLAT, PITCHED AND CURVED UNOCCUPIED ROOFS: SNOW LOAD: A. GROUND SNOW LOAD, Pg: WIND: A. ULTIMATE DESIGN WIND SPEED Vult: B. NOMINAL DESIGN WIND SPEED, Vasd: C. WIND EXPOSURE CATEGORY: D. INTERNAL PRESSURE COEFFICIENT:	XX PSF
a. ORDINARY, FLAT, PITCHED AND CURVED UNOCCUPIED ROOFS:	XX PSF
a. ORDINARY, FLAT, PITCHED AND CURVED UNOCCUPIED ROOFS: SNOW LOAD: A. GROUND SNOW LOAD, Pg: WIND: A. ULTIMATE DESIGN WIND SPEED Vult: B. NOMINAL DESIGN WIND SPEED, Vasd: C. WIND EXPOSURE CATEGORY: D. INTERNAL PRESSURE COEFFICIENT: E. COMPONENTS AND CLADDING PRESSURES: F. MAIN WIND FORCE RESISTING SYSTEM:	XX PSF
a. ORDINARY, FLAT, PITCHED AND CURVED UNOCCUPIED ROOFS: SNOW LOAD: A. GROUND SNOW LOAD, Pg: WIND: A. ULTIMATE DESIGN WIND SPEED Vult: B. NOMINAL DESIGN WIND SPEED, Vasd: C. WIND EXPOSURE CATEGORY: D. INTERNAL PRESSURE COEFFICIENT: E. COMPONENTS AND CLADDING PRESSURES: F. MAIN WIND FORCE RESISTING SYSTEM: RAIN	XX PSF. 115 MPH (3-SEC PEAK GUST) 89 MPH (3-SEC PEAK GUST) II II ± 0.18 SEE SCHEDULE WOOD SHEAR WALLS
a. ORDINARY, FLAT, PITCHED AND CURVED UNOCCUPIED ROOFS: SNOW LOAD: A. GROUND SNOW LOAD, Pg: WIND: A. ULTIMATE DESIGN WIND SPEED Vult: B. NOMINAL DESIGN WIND SPEED, Vasd: C. WIND EXPOSURE CATEGORY: D. INTERNAL PRESSURE COEFFICIENT: E. COMPONENTS AND CLADDING PRESSURES: F. MAIN WIND FORCE RESISTING SYSTEM: RAIN A. 100-YEAR RAINFALL INTENSITY (IN/HR):	XX PSF. 115 MPH (3-SEC PEAK GUST) 89 MPH (3-SEC PEAK GUST) II ± 0.18 SEE SCHEDULE WOOD SHEAR WALLS XXX
a. ORDINARY, FLAT, PITCHED AND CURVED UNOCCUPIED ROOFS: SNOW LOAD: A. GROUND SNOW LOAD, Pg: WIND: A. ULTIMATE DESIGN WIND SPEED Vult: B. NOMINAL DESIGN WIND SPEED, Vasd: C. WIND EXPOSURE CATEGORY: D. INTERNAL PRESSURE COEFFICIENT: E. COMPONENTS AND CLADDING PRESSURES: F. MAIN WIND FORCE RESISTING SYSTEM: RAIN A. 100-YEAR RAINFALL INTENSITY (IN/HR): B. MAXIMUM ROOF RAIN LOAD:	XX PSF 115.MPH (3-SEC. PEAK GUST) 89.MPH (3-SEC. PEAK GUST) II ± 0.18 SEE SCHEDULE WOOD SHEAR WALLS XXX 20 PSF
a. ORDINARY, FLAT, PITCHED AND CURVED UNOCCUPIED ROOFS: SNOW LOAD: A. GROUND SNOW LOAD, Pg: WIND: A. ULTIMATE DESIGN WIND SPEED Vult: B. NOMINAL DESIGN WIND SPEED, Vasd: C. WIND EXPOSURE CATEGORY: D. INTERNAL PRESSURE COEFFICIENT: E. COMPONENTS AND CLADDING PRESSURES: F. MAIN WIND FORCE RESISTING SYSTEM: RAIN A. 100-YEAR RAINFALL INTENSITY (IN/HR): B. MAXIMUM ROOF RAIN LOAD: C. MAXIMUM RAINWATER LEVEL - PONDING (STATIC + HYDRAULIC HEAD):	XX PSF 115 MPH (3-SEC PEAK GUST) 89 MPH (3-SEC PEAK GUST) II ± 0.18 SEE SCHEDULE WOOD SHEAR WALLS XXX 20 PSF 4"
a. ORDINARY, FLAT, PITCHED AND CURVED UNOCCUPIED ROOFS: SNOW LOAD: A. GROUND SNOW LOAD, Pg: WIND: A. ULTIMATE DESIGN WIND SPEED Vult: B. NOMINAL DESIGN WIND SPEED, Vasd: C. WIND EXPOSURE CATEGORY: D. INTERNAL PRESSURE COEFFICIENT: E. COMPONENTS AND CLADDING PRESSURES: F. MAIN WIND FORCE RESISTING SYSTEM: RAIN A. 100-YEAR RAINFALL INTENSITY (IN/HR): B. MAXIMUM ROOF RAIN LOAD: C. MAXIMUM RAINWATER LEVEL - PONDING (STATIC + HYDRAULIC HEAD): D. THE STRUCTURAL ENGINEER SHALL BE NOTIFIED IF THE TOTAL RAIN WATER	XX PSF 115 MPH (3-SEC PEAK GUST) 89 MPH (3-SEC PEAK GUST) II ± 0.18 SEE SCHEDULE WOOD SHEAR WALLS XXX 20 PSF 4"
a. ORDINARY, FLAT, PITCHED AND CURVED UNOCCUPIED ROOFS: SNOW LOAD: A. GROUND SNOW LOAD, Pg: WIND: A. ULTIMATE DESIGN WIND SPEED Vult: B. NOMINAL DESIGN WIND SPEED, Vasd: C. WIND EXPOSURE CATEGORY: D. INTERNAL PRESSURE COEFFICIENT: E. COMPONENTS AND CLADDING PRESSURES: F. MAIN WIND FORCE RESISTING SYSTEM: RAIN A. 100-YEAR RAINFALL INTENSITY (IN/HR): B. MAXIMUM ROOF RAIN LOAD: C. MAXIMUM RAINWATER LEVEL - PONDING (STATIC + HYDRAULIC HEAD): D. THE STRUCTURAL ENGINEER SHALL BE NOTIFIED IF THE TOTAL RAIN WATER SEISMIC:	XX PSF 115 MPH (3-SEC PEAK GUST) 89 MPH (3-SEC PEAK GUST) II ± 0.18 SEE SCHEDULE WOOD SHEAR WALLS XXX 20 PSF 4" 21 LEVEL EXCEEDS THE DESIGNED RAIN ROOF LOAD.
a. ORDINARY, FLAT, PITCHED AND CURVED UNOCCUPIED ROOFS: SNOW LOAD: A. GROUND SNOW LOAD, Pg: WIND: A. ULTIMATE DESIGN WIND SPEED Vult: B. NOMINAL DESIGN WIND SPEED, Vasd: C. WIND EXPOSURE CATEGORY: D. INTERNAL PRESSURE COEFFICIENT: E. COMPONENTS AND CLADDING PRESSURES: F. MAIN WIND FORCE RESISTING SYSTEM: RAIN A. 100-YEAR RAINFALL INTENSITY (IN/HR): B. MAXIMUM ROOF RAIN LOAD: C. MAXIMUM RAINWATER LEVEL - PONDING (STATIC + HYDRAULIC HEAD): D. THE STRUCTURAL ENGINEER SHALL BE NOTIFIED IF THE TOTAL RAIN WATER SEISMIC: MAPPED SPECTRAL RESPONSE VALUES. DESIGN SPECTRAL RESPONSE VALUES	XX PSF. 115.MPH (3-SEC. PEAK.GUST) 89.MPH (3-SEC. PEAK.GUST) II + 0.18 SEE SCHEDULE WOOD SHEAR WALLS XXX 20.PSF 4" LEVEL EXCEEDS THE DESIGNED RAIN ROOF LOAD. S. AND AS SITE CLASS. HAVE BEEN PROVIDED BY :
a. ORDINARY, FLAT, PITCHED AND CURVED UNOCCUPIED ROOFS: SNOW LOAD: A. GROUND SNOW LOAD, Pg: WIND: A. ULTIMATE DESIGN WIND SPEED Vult: B. NOMINAL DESIGN WIND SPEED, Vasd: C. WIND EXPOSURE CATEGORY: D. INTERNAL PRESSURE COEFFICIENT: E. COMPONENTS AND CLADDING PRESSURES: F. MAIN WIND FORCE RESISTING SYSTEM: RAIN A. 100-YEAR RAINFALL INTENSITY (IN/HR): B. MAXIMUM ROOF RAIN LOAD: C. MAXIMUM RAINWATER LEVEL - PONDING (STATIC + HYDRAULIC HEAD): D. THE STRUCTURAL ENGINEER SHALL BE NOTIFIED IF THE TOTAL RAIN WATER SEISMIC: MAPPED SPECTRAL RESPONSE VALUES, DESIGN SPECTRAL RESPONSE VALUES A. GEOTECHNICAL COMPANY AND REPORT NO.:	XX PSF. 115 MPH (3-SEC. PEAK. GUST) 89 MPH (3-SEC. PEAK. GUST) II + 0.18 SEE SCHEDULE WOOD SHEAR WALLS XXX 20 PSF 4" 1 LEVEL EXCEEDS THE DESIGNED RAIN ROOF LOAD. S, AND AS SITE CLASS, HAVE BEEN PROVIDED BY : TERRACON REPORT NO.1054056
a. ORDINARY, FLAT, PITCHED AND CURVED UNOCCUPIED ROOFS: SNOW LOAD: A. GROUND SNOW LOAD, Pg: WIND: A. ULTIMATE DESIGN WIND SPEED Vult B. NOMINAL DESIGN WIND SPEED Vult C. WIND EXPOSURE CATEGORY: D. INTERNAL PRESSURE COEFFICIENT: E. COMPONENTS AND CLADDING PRESSURES: F. MAIN WIND FORCE RESISTING SYSTEM: RAIN A. 100-YEAR RAINFALL INTENSITY (IN/HR): B. MAXIMUM ROOF RAIN LOAD: C. MAXIMUM RAINWATER LEVEL - PONDING (STATIC + HYDRAULIC HEAD): D. THE STRUCTURAL ENGINEER SHALL BE NOTIFIED IF THE TOTAL RAIN WATER SEISMIC: MAPPED SPECTRAL RESPONSE VALUES, DESIGN SPECTRAL RESPONSE VALUES A. GEOTECHNICAL COMPANY AND REPORT NO.: B. MAAPPED SPECTRAL RESPONSE ACCEI FRATION PARAMETERS. St & St	XX PSF
a. ORDINARY, FLAT, PITCHED AND CURVED UNOCCUPIED ROOFS: SNOW LOAD: A. GROUND SNOW LOAD, Pg: WIND: A. ULTIMATE DESIGN WIND SPEED Vult: B. NOMINAL DESIGN WIND SPEED Vult: C. WIND EXPOSURE CATEGORY: D. INTERNAL PRESSURE COEFFICIENT: E. COMPONENTS AND CLADDING PRESSURES: F. MAIN WIND FORCE RESISTING SYSTEM: RAIN A. 100-YEAR RAINFALL INTENSITY (IN/HR): B. MAXIMUM ROOF RAIN LOAD: C. MAXIMUM RAINWATER LEVEL - PONDING (STATIC + HYDRAULIC HEAD): D. THE STRUCTURAL ENGINEER SHALL BE NOTIFIED IF THE TOTAL RAIN WATER SEISMIC: MAPPED SPECTRAL RESPONSE VALUES, DESIGN SPECTRAL RESPONSE VALUES A. GEOTECHNICAL COMPANY AND REPORT NO.: B. MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETERS, S _D & S _{D1} C. DESIGN SPECTRAL RESPONSE ACCELERATION PARAMETERS, S _D & S _{D1}	XX PSF
 a. ORDINARY, FLAT, PITCHED AND CURVED UNOCCUPIED ROOFS:	XX PSF
a. ORDINARY, FLAT, PITCHED AND CURVED UNOCCUPIED ROOFS: SNOW LOAD: A. GROUND SNOW LOAD, Pg: WIND: A. ULTIMATE DESIGN WIND SPEED Vult: B. NOMINAL DESIGN WIND SPEED Vult: C. WIND EXPOSURE CATEGORY: D. INTERNAL PRESSURE COEFFICIENT: E. COMPONENTS AND CLADDING PRESSURES: F. MAIN WIND FORCE RESISTING SYSTEM: RAIN A. 100-YEAR RAINFALL INTENSITY (IN/HR): B. MAXIMUM ROOF RAIN LOAD: C. MAXIMUM RAINWATER LEVEL - PONDING (STATIC + HYDRAULIC HEAD): D. THE STRUCTURAL ENGINEER SHALL BE NOTIFIED IF THE TOTAL RAIN WATER SEISMIC: MAPPED SPECTRAL RESPONSE VALUES, DESIGN SPECTRAL RESPONSE VALUES A. GEOTECHNICAL COMPANY AND REPORT NO.: B. MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETERS, S _D & S _{D1} C. DESIGN SPECTRAL RESPONSE ACCELERATION PARAMETERS, S _D & S _{D1}	XX PSF

1. THE CONSTRUCTION DOCUMENTS ARE BASED ON THE REQUIREMENTS OF THE INTERNATIONAL BUILDING CODE WITH LOCAL AMENDMENTS FROM THE AUTHORITY HAVING JURISDICTION.

CITY OF COLLEGE STATION

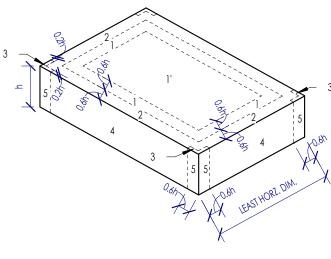
C&C - GROSS ULTIMATE WIND PRESSURES

Cladding	Location	Effective	Coeffi	cients	Wind pre	ssures
Туре		Area (sf)	+GCp	-GCp	+p (psf)	-p (psf)
Wall	Interior	10	0.90	-0.99	+30.6	-33.2
		40	0.80	-0.89	+27.9	-30.5
		50	0.79	-0.88	+27.5	-30.0
		100	0.74	-0.83	+26.1	-28.7
		500	0.63	-0.72	+23.0	-25.5
Wall	Edge	10	0.90	-1.26	+30.6	-40.8
		40	0.80	-1.07	+27.9	-35.4
		50	0.79	-1.04	+27.5	-34.5
		100	0.74	-0.94	+26.1	-31.8
		500	0.63	-0.72	+23.0	-25.5
Roof	Interior	10	0.30	-1.00	+13.6	-33.5
		40	0.24	-0.94	+11.9	-31.8
		50	0.23	-0.93	+11.6	-31.5
		100	0.20	-0.90	+10.8	-30.6
		341	0.20	-0.90	+10.8	-30.6
Roof	Edge	10	0.90	-1.80	+30.6	-56.2
		40	0.80	-1.38	+27.9	-44.2
		50	0.79	-1.31	+27.5	-42.3
		100	0.74	-1.10	+26.1	-36.3
		500	0.63	-1.10	+23.0	-36.3
Roof	Corner	10	0.90	-1.80	+30.6	-56.2
		40	0.80	-1.38	+27.9	-44.2
		50	0.79	-1.31	+27.5	-42.3
		100	0.74	-1.10	+26.1	-36.3
		500	0.63	-1.10	+23.0	-36.3
O∨erhang	Interior	10	0.00	-1.70	+10.0	-48.2
	& Edge	40	0.00	-1.64	+10.0	-46.5
		50	0.00	-1.63	+10.0	-46.2
		100	0.00	-1.60	+10.0	-45.4
		500	0.00	-1.10	+10.0	-31.2
O∨erhang	Corner	10	0.00	-1.70	+10.0	-48.2
		40	0.00	-1.64	+10.0	-46.5
		50	0.00	-1.63	+10.0	-46.2
		100	0.00	-1.60	+10.0	-45.4
		500	0.00	-1.10	+10.0	-31.2
Parapet	Interior	10	2.70	-1.89	+76.6	-53.6
		40	2.18	-1.70	+61.9	-48.2
		50	2.10	-1.67	+59.6	-47.3
		100	1.84	-1.57	+52.2	-44.6
		500	1.73	-1.35	+49.1	-38.3
Parapet	Edge	10	2.70	-2.16	+76.6	-61.3
		40	2.18	-1.87	+61.9	-53.1
		50	2.10	-1.83	+59.6	-51.8
		100	1.84	-1.68	+52.2	-47.7
		500	1.73	-1.35	+49.1	-38.3

a = MINIMUM OF (10% OF LEAST HORIZONTAL DIMENSION OR 0.4h) BUT NOT LESS THAN 4% OF LEAST HORIZONTAL DIMENSION OR 3FT

h = MEAN ROOF HEIGHT OF A BUILDING, EXCEPT THAT EAVE HEIGHT SHALL BE USED FOR ROOF ANGLES LESS THAN OR EQUAL TO 10° (~2:12 ROOF PITCH) MEAN ROOF HEIGHT = THE AVERAGE OF THE ROOF EAVE HEIGHT AND HEIGHT TO THE HIGHEST POINT ON THE ROOF SURFACE.

COMPONENTS AND CLADDING ZONES DESCRIPTION ZONE ROOF INTERIOR ROOF EDGE ROOF CORNER 3 WALL INTERIOR WALL EDGE



LAT /HIP/ GABLE ROOF - h ≤ 6 ° (0:12) < SLOPE ≤ 7° (1.5:1)

FOUNDATION DESIGN CRITERIA

- . GEOTECHNICAL REPORT: THIS FOUNDATION DESIGN IS BASED ON THE RECOMMENDATIONS PROVIDED IN SITE-SPECIFIC GEOTECHNICAL REPORT. IN DESIGNING THE FOUNDATION FOR THE PROPOSED STRUCTURE, THE FOUNDATION DESIGN ENGINEER DOES NOT ASSUME RESPONSIBILITY FOR THE ACCURACY OF THE GEOTECHNICAL ENGINEER'S REPORT OR ANY INFORMATION CONTAINED THEREIN. INFORMATION CONTAINED IN THE GEOTECHNICAL REPORT(S) REFLECTS CONDITIONS AS FOUND AT THE LOCATION OF THE BORINGS. ACTUAL CONDITIONS AT LOCATIONS BETWEEN AND SURROUNDING THE BORINGS MAY DIFFER FROM THE SOIL STRATIGRAPHY DEPICTED BY THE BORINGS. IE THERE ARE ANY CONDITIONS DIFFERING FROM THOSE DESCRIBED IN THE GEOTECHNICAL REPORT, OR IF ANY CHANGES HAVE BEEN IMPOSED ON THE SOILS IN QUESTION SINCE THE REPORT WAS WRITTEN, THEN THE DESIGN ENGINEER OF RECORD SHOULD BE NOTIFIED IN WRITING PRIOR TO CONSTRUCTION OF THE FOUNDATION IN ORDER TO REVIEW THE EFFECTS ON THE PERFORMANCE OF THE DESIGNED FOUNDATION. A. GEOTECHNICAL ENGINEER: RABA KISTNER B. REPORT NUMBER: AHA17-053-00 (REVISE)
- C. REPORT DATE: JULY 12, 2021 D. THE FOUNDATION DESIGN PARAMETERS PROVIDED WILL NOT ELIMINATE POST-CONSTRUCTION FOUNDATION MOVEMENT. AS SUCH, MEASURES SHALL BE TAKEN TO INCREASE THE TOLERANCE OF THE STRUCTURE SUPPORTED BY THE FOUNDATION. MEASURES INCLUDE BUT ARE NOT LIMITED TO FREQUENT CONTROL JOINTS FOR MASONRY/BRICK/STONE/STUCCO
- EXTERIOR VENEER (15'-0 MAXIMUM), VERTICALLY SLOTTED CLIPS TO ATTACH ROOF TRUSSES TO NON-LOAD BEARING WALLS. ETC ABNORMAL CONDITIONS: IF THE FOUNDATION IS INSTALLED DURING A DRY OR WET PERIOD, WHICH IS CONSIDERED EXTREME OR ABNORMAL, THEN THE BUILDER SHALL NOTIFY THE SEOTECHNICAL ENGINEER AND FOUNDATION ENGINEER PRIOR TO CONSTRUCTION FOR POSSIBLE SOIL CONDITIONING OR FOUNDATION RE-DESIGN. FOUNDATION MOVEMENT: THE FOUNDATION HAS BEEN DESIGNED WITH THE ASSUMPTION THAT MOVEMENT CAN BE TOLERATED WITHIN A STANDARD PERFORMANCE LIMIT:
- A. STANDARD PERFORMANCE DEFLECTION LIMIT: L/360 B. STANDARD PERFORMANCE TILT LIMIT: 177
- . SOIL MOISTURE LEVEL: A REASONABLY UNIFORM SOIL MOISTURE LEVEL IS MAINTAINED AROUND THE FOUNDATION FOR THE LIFE OF THE STRUCTURE . FOUNDATION MAINTENANCE: POSITIVE DRAINAGE AWAY FROM THE STRUCTURE SHALL BE MAINTAINED FOR THE LIFE OF THE STRUCTURE AND THE CONTRACTOR SHALL CONVEY THIS REQUIREMENT TO THE OWNER. THE INITIAL AND ALL SUBSEQUENT OWNERS MAINTAIN THE FOUNDATION IN ACCORDANCE WITH THE LATEST REVISION OF DOCUMENT NO. FPA-SC-07. "FOUNDATION MAINTENANCE AND INSPECTION GUIDE FOR RESIDENTIAL AND OTHER LOW-RISE BUILDINGS", AVAILABLE ON THE FOUNDATION PERFORMANCE ASSOCIATION'S WEBSITE:
- WWW.FOUNDATIONPERFORMANCE.ORG. CONTRACTOR SHALL PROVIDE THIS DOCUMENT TO OWNER. . EXPIRATION: PLANS ARE VALID FOR 6-MONTHS FROM THE DATE THE PLANS ARE ISSUED OR REVISED BY THE ENGINEER. CONTACT ENGINEER FOR REVIEW IF PLANS HAVE EXPIRED OR IF CONSTRUCTION OF THE FOUNDATION HAS NOT COMMENCED WITHIN THIS TIME FRAME.

LATERAL LOAD RESISTING SYSTEM

1. ALL LATERAL LOAD RESISTANCE AND STABILITY OF THE BUILDING IS PROVIDED EXCLUSIVELY BY VERTICAL LATERAL LOAD RESISTING SYSTEM. THE HORIZONTAL DIAPHRAGMS DISTRIBUTE THE LATERAL WIND AND SEISMIC FORCES HORIZONTALLY TO THE VERTICAL LATERAL LOAD RESISTING SYSTEM. A. VERTICAL LATERAL LOAD RESISTING SYSTEM: CMU AND WOOD-FRAMED SHEAR WAL B. HORIZONTAL LATERAL LOAD RESISTING SYSTEM: WOOD STRUCTURAL PANEL ROOF DECK

STAIR, HANDRAILS, RESTROOM ACCESSORIES AND GUARDRAIL SPECIFICATIONS:

- 1. ALL STAIRS, GUARDRAILS AND HANDRAILS SHALL BE DESIGNED BY A REGISTERED STRUCTURAL ENGINEER BASED ON THE FOLLOWING DESIGN CRITERIA: A. STAIRS:
- a. STAIR STRINGERS, TREADS AND RISERS SHALL BE DESIGNED TO SUPPORT 100 PSF LIVE LOAD. b. INDIVIDUAL STAIR TREADS SHALL BE DESIGNED TO SUPPORT A 300 LB CONCENTRATED LOAD PLACED IN A POSITION THAT WOULD CAUSE THE MAX STRESS
- B. HANDRAIL AND GUARDS a. GUARD TOP RAIL AND HANDRAILS: THE TOP RAIL OF GUARDRAILS AND HANDRAILS SHALL BE DESIGNED TO WITHSTAND A LOAD OF 50 PLF APPLIED HORIZONTALLY AT RIGHT ANGLES, OR A 200 LB CONCENTRATED LOAD IN ANY DIRECTION b. INTERMEDIATE RAILS, PANEL FILLER AND THEIR CONNECTIONS SHALL BE DESIGNED TO WITHSTAND A LOAD OF 50 PSF APPLIED HORIZONTALLY AT RIGHT ANGLES OVER THE ENTIRE TRIBUTARY AREA, INCLUDING OPENINGS AND SPACES BETWEEN RAILS.
- C. RESTROOM ACCESSORIES: a. GRAB BARS, TUB AND SHOWER SEATS, FASTENERS, AND MOUNTING DEVICES SHALL BE DESIGNED TO RESIST A CONCENTRATED LOAD OF 250 POUNDS AT ANY LOCATION AND IN ANY DIRECTION.

1. STRUCTURAL DEFERRED SUBMITTALS ARE THOSE PORTIONS OF THE DESIGN WHICH REQUIRE STRUCTURAL ENGINEERING THAT ARE NOT SUBMITTED AT THE TIME OF THE APPLICATION BUT ARE TO BE SUBMITTED TO THE BUILDING OFFICIAL AT A LATER DATE. DEFERRED SUBMITTALS SHALL BE SUBMITTED TO AND APPROVED BY THE BUILDING OFFICIAL PRIOR TO INSTALLATION OF ANY SAID WORK.

2. COMPLETE STRUCTURAL SHOP DRAWINGS FOR CONSTRUCTION OF EACH BUILDING COMPONENT NOT DESIGNED BY THE STRUCTURAL ENGINEER-OF-RECORD (SER) AND NOT SPECIFIED ON THE PROJECT CONSTRUCTION DOCUMENTS SHALL BE SEALED AND SIGNED BY A SPECIALTY STRUCTURAL ENGINEER (SSE) WHO IS A LICENSED PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS BEING CONSTRUCTED WHO IS QUALIFIED TO PERFORM SAID WORK. A SEAL BY A LICENSED PROFESSIONAL ENGINEER IS NOT REQUIRED FOR EITHER PRODUCTS WHICH HAVE BEEN TESTED AND CERTIFIED BY AN APPROVED AGENCY SUCH AS THE ICC NOR FOR COMPONENTS WHICH ARE FABRICATED BY A FABRICATOR THAT IS CERTIFIED BY AN APPROVED AGENCY IN WHICH THE AGENCY SPECIFIED THAT SEALING OF THE SHOP DRAWINGS IS NOT REQUIRED (E.G. STEEL JOIST INSTITUTE IN REGARDS TO OPEN 3. THE SPECIALTY STRUCTURAL ENGINEER (SSE) SHALL SPECIFICALLY INDICATE IN A COVER PAGE AT THE FRONT OF THE SHOP DRAWING THAT THEY ARE THE STRUCTURAL ENGINEER IN RESPONSIBLE CHARGE FOR THE DEFERRED

SUBMITTAL AND THAT THEY HAVE REVIEWED THE SHOP DRAWING TO ENSURE COMPLIANCE WITH THEIR DESIGN AND CALCULATIONS. 4. ALL STRUCTURAL DEFERRED SUBMITTALS SHALL BE REVIEWED BY THE SER AND MARKED AS EITHER NO EXCEPTIONS OR EXCEPTION NOTED, PRIOR TO SUBMITTING TO THE "FOR CONSTRUCTION" VERSION TO THE AUTHORITY HAVING JURISDICTION (AHJ) AND PRIOR TO RELEASE FOR FABRICATION.

A. STAIRS, GUARDRAIL, HANDRAILS, GRAB BARS, LADDERS, ETC. (NOT REQUIRED IF USING CERTIFIED AND TESTED PRODUCTS/ASSEMBLIES) B. CURTAINWALL, STOREFRONT, WINDOWS (NOT REQUIRED IF USING CERTIFIED AND TESTED PRODUCTS/ASSEMBLIES)

1. THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. METHODS, PROCEDURES AND SEQUENCES OF CONSTRUCTION ARE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO MAINTAIN AND ENSURE THE INTEGRITY OF THE STRUCTURE AT ALL STAGES 2. THE CONTRACTOR IS RESPONSIBLE FOR QUALITY CONTROL, INCLUDING WORKMANSHIP AND MATERIALS FURNISHED BY SUBCONTRACTORS AND SUPPLIERS.

3. REFER TO DRAWINGS OTHER THAN STRUCTURAL FOR COMPLETE INFORMATION REGARDING: SLEEVES, CURBS, INSERTS, DEPRESSIONS, OPENINGS, ETC. 4. IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO OBTAIN ALL CONTRACT DOCUMENTS AND LATEST REVISIONS/ADDENDA AND TO SUBMIT SUCH DOCUMENTS TO ALL SUBCONTRACTORS AND MATERIAL SUPPLIERS PRIOR TO THE SUBMITTAL OF SHOP DRAWINGS OR MATERIAL PROCUREMENT. 5. THE USE OR REPRODUCTIONS OF THESE CONTRACT DRAWINGS BY ANY CONTRACTOR OR MATERIAL SUPPLIER IN LIEU OF PREPARATION OF SHOP DRAWINGS SIGNIFIES HIS ACCEPTANCE OF ALL INFORMATION SHOWN HEREON AS CORRECT, AND OBLIGATES HIMSELF TO ANY JOB EXPENSE, REAL OR IMPLIED, DUE TO ANY ERRORS THAT MAY OCCUR

7. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL EXCAVATION PROCEDURES INCLUDING LAGGING, SHORING AND PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS AND UTILITIES IN ACCORDANCE WITH ALL CODES AND REQUIREMENTS OF THE AUTHORITY HAVING JURISDICTION. 8. THE GENERAL CONTRACTOR SHALL COMPARE THE ARCHITECTURAL AND STRUCTURAL DRAWINGS AND REPORT ANY DISCREPANCIES BETWEEN EACH SET OF DRAWINGS AND WITHIN EACH SET OF DRAWINGS TO THE ARCHITECT AND ENGINEER PRIOR TO THE FABRICATION AND INSTALLATION OF ANY STRUCTURAL MEMBERS. 9. FRAMING LAYOUTS ARE PROVIDED TO REPRESENT DESIGN CONCEPTS AND SYSTEMS CONSTRUCTION. THE CONTRACTOR AND SUBCONTRACTORS ARE RESPONSIBLE FOR MATERIAL QUANTITIES AND ANY AND ALL UNSPECIFIED COMPONENTS REQUIRED FOR CONSTRUCTION. 10. WHERE MEMBER LOCATIONS ARE NOT SPECIFICALLY DIMENSIONED, MEMBERS ARE EITHER LOCATED ON COLUMN LINES OR ARE EQUALLY SPACED BETWEEN THE LOCATED 11. IF CERTAIN FEATURES ARE NOT FULLY SHOWN OR SPECIFIED ON THE DRAWINGS OR IN THE SPECIFICATIONS, THEIR CONSTRUCTION SHALL BE OF THE SAME CHARACTER AS SHOWN 12. WHERE CONFLICT EXISTS AMONG THE VARIOUS PARTS OF THE STRUCTURAL CONTRACT DOCUMENTS, STRUCTURAL DRAWINGS, GENERAL NOTES AND SPECIFICATIONS, THE 13. THE FLOOR DESIGN LIVE LOAD FOR EACH ELEVATED FLOOR STRUCTURE OR PORTION THEREOF THAT EXCEEDS 50 POUNDS PER SQUARE FOOT (PSF) SHALL BE STATED ON DURABLE SIGNS AND CONSPICUOUSLY POSTED BY THE OWNER IN THE APPLICABLE AREA(S) OF THE BUILDING. 14. ALL STRUCTURES REQUIRE PERIODIC MAINTENANCE TO EXTEND LIFESPAN AND ENSURE STRUCTURAL INTEGRITY FROM EXPOSURE TO THE ENVIRONMENT. A PLANNED PROGRAM OF MAINTENANCE SHALL BE ESTABLISHED BY THE BUILDING OWNER. THIS PROGRAM SHALL INCLUDE SUCH ITEMS AS, BUT NOT LIMITED TO, PAINTING OF STRUCTURAL STEEL,

A. THE ENGINEER SHALL NOT HAVE CONTROL NOR CHARGE OF, AND SHALL NOT BE RESPONSIBLE FOR, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, OR PROCEDURES, FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK, FOR THE ACTS OR OMISSION OF THE CONTRACTOR, SUBCONTRACTOR, OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. B. PERIODIC SITE OBSERVATION BY FIELD REPRESENTATIVES OF THE STRUCTURAL ENGINEER IS SOLELY FOR THE PURPOSE OF BECOMING GENERALLY FAMILIAR WITH THE PROGRESS AND QUALITY OF THE WORK COMPLETED AND DETERMINING, IN GENERAL, IF THE WORK OBSERVED IS BEING PERFORMED IN A MANNER INDICATING THAT THE WORK, WHEN FULLY COMPLETED, WILL BE IN ACCORDANCE WITH THE STRUCTURAL CONTRACT DOCUMENTS. THIS LIMITED SITE OBSERVATION SHOULD NOT BE CONSTRUED AS EXHAUSTIVE OR CONTINUOUS TO CHECK THE QUALITY OR QUANTITY OF THE WORK, BUT RATHER PERIODIC IN AN EFFORT TO GUARD THE OWNER AGAINST DEFECTS OR DEFICIENCIES IN THE 16. WATERPROOFING OF THE BUILDING ENVELOPE IS OF CRITICAL IMPORTANCE TO LONG-TERM STRUCTURAL PERFORMANCE. WATERPROOFING DESIGN SHALL BE THE RESPONSIBILITY OF THE ARCHITECT/CONTRACTOR AND SHALL BE IN ACCORDANCE WITH BEST PRACTICES FOR THE LOCALITY AND THE PARTICULAR ASSEMBLY.

1. WORK SHALL BE PERFORMED BY A QUALIFIED CONSTRUCTION CONTRACTOR AND SUBCONTRACTOR EXPERIENCED IN THIS TYPE OF WORK. SUCH KNOWLEDGE SHALL INCLUDE MAKING ALLOWANCES FOR PERFORMING WORK OF THIS NATURE FOLLOWING INDUSTRY STANDARDS OF CARE. 2. THE CONSTRUCTION CONTRACTOR AND SUBCONTRACTORS SHALL UNDERSTAND THE NATURE OF DRAWING PRODUCTION AND COORDINATION BETWEEN CONSULTANTS AND SHALL NOT ENTER INTO A CONTRACT BASED ON DRAWINGS THAT ARE BELIEVED TO CONTAIN DISCREPANCIES OR ARE OTHERWISE INCOMPLETE UNLESS PROPER ALLOWANCES HAVE BEEN MADE FOR COST IMPLICATIONS THAT MAY ARISE DUE TO FUTURE DRAWING CHANGES MADE IN PREPARATION OF FINAL CONSTRUCTION DOCUMENTS. 3. IN THE COURSE OF PRODUCING AND ISSUING DRAWINGS, VARIOUS STAGES OF COMPLETION ARE DEVELOPED. THE CONSTRUCTION CONTRACTOR AND SUBCONTRACTORS SHALL UNDERSTAND THE PURPOSE AND CONTENT CONTAINED IN PERMIT, PRICING, AND CONSTRUCTION DRAWINGS. COST IMPLICATIONS AND CONTRACTIBILITY ARE THE RESPONSIBILITY OF THE CONSTRUCTION CONTRACTOR AND SUBCONTRACTORS UNLESS PRIOR ARRANGEMENTS HAVE BEEN MADE WITH THE OWNER.

1. NO PROVISIONS FOR ANY FUTURE EXPANSION HAVE BEEN MADE IN THE STRUCTURAL DESIGN

1. ALL REQUESTS FOR SUBSTITUTIONS OF MATERIALS OR DETAILS SHOWN IN THE CONTRACT DOCUMENTS SHALL BE SUBMITTED FOR APPROVAL DURING THE BIDDING PERIOD. ONCE BIDS ARE ACCEPTED, PROPOSED SUBSTITUTIONS WILL BE CONSIDERED ONLY WHEN THEY ARE OFFICIALLY SUBMITTED WITH AN IDENTIFIED SAVINGS TO BE DEDUCTED FROM THE

SHEET THAT INDICATES THE FOLLOWING

VREQUIRED

ONSTRUCTION DOCUMENTS AND PHONE NUMBER)

D/OR SPECIFICATION NUMBER IF APPLICABLE

A. THE GENERAL CONTRACTOR SHALL PREPARE A DETAILED LIST AND SCHEDULE OF ALL SUBMITTAL ITEMS TO BE SENT TO THE STRUCTURAL ENGINEER PRIOR TO THE START OF CONSTRUCTION. THIS

A. ALL SUBMITTALS MUST BE REVIEWED AND ELECTRONICALLY STAMPED BY THE GENERAL CONTRACTOR PRIOR TO SUBMITTAL TO THE DESIGN TEAM AS NO EXCEPTIONS.

a. SUBMITTAL NUMBER PER THE FOLLOWING FORMAT: E.G. 03 30 00-01.00 (DIVISION, SUBMITTAL # FOR DIVISION, ISSUE # - THE EXAMPLE INDICATES THE FIRST SUBMITTAL, FIRST ISSUE OF A

C. CONTRACTOR SHALL PROVIDE THE SUBMITTAL IN ELECTRONIC (PDF) FORMAT. SUBMITTALS SHALL NOT BE SCANNED COPIES OF PRINTED DOCUMENTS.

D. THE OMISSION FROM THE SHOP DRAWINGS OF ANY MATERIALS REQUIRED BY THE CONTRACT DOCUMENTS SHALL NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY OF FURNISHING AND INSTALLING SUCH MATERIALS, REGARDLESS OF WHETHER SHOWN OR COMMENTED IN THE SHOP DRAWING E. THE CONTRACTOR MUST ALLOW A MINIMUM OF 14 DAYS FOR STRUCTURAL REVIEW OF ALL SUBMITTALS. THE CONTRACTOR CAN REQUEST AN EXPEDITED REVIEW AT AN AGREED UPON FEE WITH F. STRUCTURAL STEEL SUBMITTALS MUST BE ACCOMPANIED BY THE SDS/2 OR TEKLA MODEL WHICH WILL BE USED BY THE DESIGN TEAM AS A VISUAL AID TO THE SHOP DRAWINGS.

3. REFER TO THE SPECIFICATIONS FOR A LIST OF ALL THE REQUIRED SUBMITTALS. 4. ENGINEER REVIEW STAMP DESIGNATIONS: ALL DESIGNATIONS ARE INDICATIVE OF A REVIEW FOR GENERAL CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS.

a. NO ITEMS WERE FOUND TO BE IN CONFLICT WITH THE CONSTRUCTION DOCUMENTS, NO "FOR REVIEW" RESUBMITTAL REQUIRED.

a. ITEMS WERE FOUND IN CONFLICT WITH THE CONSTRUCTION DOCUMENTS AND NEED TO BE REVISED PRIOR TO SUBMITTING "FOR CONSTRUCTION" SUBMITTAL

a. SIGNIFICANT ITEMS WERE FOUND IN CONFLICT WITH THE CONSTRUCTION DOCUMENTS. THE SUBMITTAL NEEDS TO BE RESUBMITTED "FOR REVIEW"

a. THE SUBMITTAL WAS NOT STRUCTURAL. a. THE SUBMITTAL DID NOT REQUIRE REVIEW BUT HAS BEEN FILED FOR THE RECORD.

a. THE SUBMITTAL HAS BEEN REVIEWED FOR THE STRUCTURALLY IMPACT TO THE STRUCTURE ONLY

1. CONSTRUCTION OR WORK FOR WHICH A PERMIT IS REQUIRED SHALL BE SUBJECT TO INSPECTION BY THE BUILDING OFFICIAL AND SUCH CONSTRUCTION OR WORK SHALL REMAIN ACCESSIBLE AND EXPOSED FOR INSPECTION PURPOSES UNTIL APPROVED. REQUIRED TESTING INCLUDES BUT IS NOT LIMITED TO THE FOLLOWING: a. FOOTING AND FOUNDATION INSPECTIONS SHALL BE MADE AFTER EXCAVATIONS FOR FOOTINGS ARE COMPLETE AND ANY REQUIRED REINFORCING STEEL IS IN PLACE.

FOR CONCRETE FOUNDATIONS, ANY REQUIRED FORMS SHALL BE IN PLACE PRIOR TO INSPECTION. MATERIALS FOR THE FOUNDATION SHALL BE ON THE JOB, EXCEPT WHERE CONCRETE IS READY MIXED IN ACCORDANCE WITH ASTM C94, THE CONCRETE NEED NOT BE ON THE JOB. a. CONCRETE SLAB AND UNDER-FLOOR INSPECTIONS SHALL BE MADE AFTER IN-SLAB OR UNDER-FLOOR REINFORCING STEEL AND BUILDING SERVICE EQUIPMENT, CONDUIT,

PIPING ACCESSORIES AND OTHER ANCILLARY EQUIPMENT ITEMS ARE IN PLACE, BUT BEFORE ANY CONCRETE IS PLACED OR FLOOR SHEATHING INSTALLED, INCLUDING

a. FRAMING INSPECTIONS SHALL BE MADE AFTER THE ROOF DECK OR SHEATHING, ALL FRAMING, FIREBLOCKING AND BRACING ARE IN PLACE AND PIPES, CHIMNEYS AND VENTS TO BE CONCEALED ARE COMPLETE AND THE ROUGH ELECTRICAL, PLUMBING, HEATING WIRES, PIPES AND DUCTS ARE APPROVED. . SPECIAL INSPECTIONS - REFER TO THE STATEMENT OF SPECIAL INSPECTION FOR REQUIRED STRUCTURAL SPECIAL INSPECTIONS . ADDITIONAL INSPECTIONS REQUIRED BY STRUCTURAL ENGINEER: REFERENCE SPECIFICATIONS

A. PARTIAL PLANS, ELEVATIONS, SECTIONS, DETAIL OR SCHEDULES LABELED WITH "TYPICAL" AT THE BEGINNING OF THEIR TITLE SHALL APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THE THOSE SPECIFICALLY SHOWN. THE APPLICABILITY OF THE CONTENT OF THESE VIEWS TO LOCATIONS ON THE PLAN CAN BE DETERMINED FROM THE TITLE OF THE VIEW. SUCH VIEWS SHALL APPLY WHETHER OR NOT THEY ARE KEYED IN AT EACH LOCATION, DECISIONS REGARDING APPLICABILITY OF THESE "TYPICAL" VIEWS SHALL BE DETERMINED BY THE STRUCTURAL ENGINEER. 2. COLOR: THESE DRAWINGS ARE INTENDED TO BE VIEWED IN COLOR. IF THE FOLLOWING COLORS ARE NOT RED_GREEN BLUE THEN THIS DRAWING SET IS NOT BEING VIEWED AS INTENDED. 3. SCALE: IF THE FOLLOWING LINE IS NOT EXACTLY 1" LONG, THEN THIS SET HAS BEEN SCALED.

1. DETAILING OF CONCRETE REINFORCEMENT BARS AND ACCESSORIES SHALL CONFORM TO THE RECOMMENDATIONS OF THE ACI DETAILING MANUAL ACI 315 AND SP-66 (ACI 2. CONCRETE REINFORCEMENT BARS SHALL CONFORM TO ASTM A615, GRADE 60, WITH SUPPLEMENTARY REQUIREMENTS. 3. COMPLETE REINFORCING PLACEMENT DRAWINGS PREPARED IN ACCORDANCE WITH ACI315 SHALL BE REVIEWED BY THE ENGINEER AND AVAILABLE ON THE JOB SITE PRIOR TO & 4. ALL REINFORCING STEEL SHALL BE SUPPORTED AT DESIGNED DEPTH USING PLASTIC OR METALLIC CHAIRS SPACED AT 48" OC IN ALL DIRECTIONS TO SUPPORT FULL LENGTH OF REINFORCEMENT. IF ALTERNATE IS TO BE USED, PROPOSED CHAIR IS TO BE SUBMITTED IN WRITING AND APPROVED BY E.O.R. 5. END HOOKS, DEVELOPMENT LENGTHS, AND SPLICES SHALL CONFORM TO THE REQUIREMENTS OF ACI 318. 6. REINFORCEMENT MAY BE PLACED IN BUNDLES OF NOT MORE THAN TWO W/ THE CLEAR DISTANCE BETWEEN BUNDLES OF REINFORCEMENT OR TENDONS OF 3 INCHES MINIMUM.

8. UNO, ALL LAP SPLICES OF REINFORCEMENT IN GROUND SUPPORTED ELEMENTS (GRADE BEAMS, FOOTINGS, MAT FOUNDATIONS) SHALL BE A MINIMUM OF 48Ø, WHERE Ø = THE DIAMETER OF THE BAR, REINFORCEMENT IN ELEVATED STRUCTURES SHALL REFER TO THE TYPICAL LAP SPLICE DETAIL.

REINFORCED CONCRETE - 03 30 00

- A. CONCRETE WORK SHALL CONFORM TO THE LATEST ED. OF ACI 301 (SPECIFICATIONS FOR STRUCTURAL CONCRETE) UNO IN THESE CONSTRUCTION DOCUMENTS. 2. MIX DESIGN A. ALL CONCRETE MIXES SHALL BE DESIGNED BY A QUALIFIED REGISTERED ENGINEER. MIX DESIGN DATA RESULTS EITHER COMPLYING WITH THE FIELD EXPERIENCE OR TRIAL MIXTURE METHOD PER ACI 301/318 SHALL BE SUBMITTED FOR EACH CONCRETE MIX. PROPORTIONS OF MATERIALS FOR CONCRETE SHALL BE ESTABLISHED TO: a. PROVIDE WORKABILITY AND CONSISTENCY TO PERMIT CONCRETE TO BE WORKED READILY INTO FORMS AND AROUND REINFORCEMENT UNDER CONDITIONS OF PLACEMENT TO BE EMPLOYED, WITHOUT SEGREGATION OR EXCESSIVE BLEEDING. b. MEET REQUIREMENTS FOR APPLICABLE EXPOSURE REQUIREMENTS.
- c. MEET OR EXCEED THE REQUIRED F'C. d. NOT EXCEED THE MAXIMUM W/C RATIO B. THE CONTRACTOR MUST INDICATE THE PLANNED PLACEMENT METHOD FOR EACH CONCRETE MIX.
- C. WATER MAY NOT BE ADDED TO THE CONCRETE MIX IN THE FIELD TO ADJUST THE SLUMP (RETEMPERING) WITHOUT THE SPECIAL INSPECTOR BEING PRESENT TO CONFIRM THAT IT DOES NOT EXCEED THE W/C RATIO OR DESIGN SLUMP. THE READY-MIX COMPANY MUST INDICATE THE MAXIMUM WATER WITHHELD AT THE PLANT. IF THE AMOUNT, THE W/C RATIO OR DESIGN SLUMP IS EXCEEDED THEN THE CONCRETE SHALL BE REJECTED. D. SLUMP TESTS SHALL BE PERFORMED AT THE POINT OF PLACEMENT WITH THE EXCEPTIONS NOTED BELOW:
- G. IF THE POINT OF DELIVERY IS THE SAME AS THE POINT OF PLACEMENT (CONCRETE IS PLACED DIRECTLY FROM TRUCK) b. IF THE CONTRACTOR HAS DEVELOPED AN ACCEPTABLE (APPROVED BY SPECIAL INSPECTOR AND EOR) CORRELATION BETWEEN FRESH CONCRETE PROPERTIES AT THE POINT OF DELIVERY AND POINT OF PLACEMEN E. AIR-ENTRAINED CONCRETE SHALL NOT BE USED IN ANY NORMAL WEIGHT CONCRETE FLOOR SLAB THAT IS TO RECEIVE A HARD-TROWELED FINISH. 3. CONCRETE MATERIALS:
- A. HYDRAULIC CEMENT a. USE ASTM C150 TYPE I OR TYPE III, EXCEPT WHERE SPECFICALLY INDICATED OTHERWISE IN TABLE BELOW. B. FLY ASH:
- a. FLY ASH MAY BE USED TO REPLACE A PORTION OF THE PORTLAND CEMENT, SUBJECT TO THE APPROVAL OF THE ARCHITECT AND STRUCTURAL ENGINEER NOT TO EXCEED THE AMOUNTS LISTED IN THE CONCRETE TABLE. b. USE ASTM C618 CLASS C OR F
- C. NORMAL WEIGHT AGGREGATE: a. USE ASTM C33.
- b. MATERIAL CERTIFICATES FROM THE AGGREGATE SUPPLIER MUST BE SUBMITTED WITH THE CONCRETE MIX DESIGN. c. RIVER ROCK OR PEA STONE AGGREGATES ARE NOT ACCEPTABLE. D. WATER COMPLY WITH THE REQUIREMENTS OF ASTM C1602.
- 4. CHLORIDE ION A. FOR CORROSION PROTECTION OF REINFORCEMENT IN CONCRETE, MAXIMUM WATER SOLUBLE ION CONCENTRATIONS IN HARDENED CONCRETE AT AGES FROM 28 TO 42 DAYS CONTRIBUTED FROM THE THE INGREDIENTS INCLUDING WATER, AGGREGATES, CEMENTITIOUS MATERIALS, AND ADMIXTURES SHALL NOT EXCEED THE LIMITS INDICATED IN THE TABLE BELOW.
- 5. PLACEMENT A. CONCRETE SHALL BE PLACED CAREFULLY SO AS NOT TO DEVIATE REINFORCEMENT FROM THE DESIGN LOCATION. B. CONCRETE SHALL BE PROPERLY VIBRATED, ESPECIALLY AROUND POST-TENSIONED ANCHORAGES AND CONGESTED AREAS SUCH AS COLUMN JOINTS.
- C. PLACEMENT OF CONCRETE SHALL BE COMPLETED WITHIN 90 MINUTES AFTER THE INTRODUCTION OF THE MIXING WATER, IN ACCORDANCE WITH ASTM C94. D. TOLERANCES FOR CONCRETE CONSTRUCTION SHALL CONFORM TO THE LATEST ED. OF ACI 117 (SPECIFICATION FOR TOLERANCES FOR CONCRETE CONSTRUCTION AND MATERIALS).

f'c

3,500

EXPOSURE CATEGORY

F0,S0,P(W)0,C1

CONCRETE FINISHING AND CURING

ELEMENT

INTERIOR SLABS-ON-GROUNDA

- . FINISHING: FINISHING OPERATIONS AND BULL FLOATING SHALL BE COMPLETED PRIOR TO THE ACCUMULATION OF BLEED WATER ON THE SURFACE. FINAL FINISHING SHOULD NOT BEGIN UNTIL THE
- DUSTING. REFER TO ARCHITECTURE FOR FINAL FINISHING REQUIREMENTS (STEEL TROWEL, BROOM FINISH, ETC.).
- BLEED WATER IS TO DRAG THE SURFACE WITH A GARDEN HOSE.
- 3. CONTROL JOINTS (SAW CUTS) IF REQUIRED, SHALL BE MADE AS SOON AS THE CONCRETE CAN SUPPORT THE WEIGHT OF WORKER AND THE EQUIPMENT. CURING: IMMEDIATELY AFTER FINISHING THE SLAB, THE SLAB MUST BE CURED FOR A MINIMUM OF 7 DAYS BY EITHER: A. APPLYING A WATER-BASED DISSIPATING RESIN TYPE CURING COMPOUND WHICH CHEMICALLY BREAKS DOWN AFTER APPROXIMATELY 4 WEEKS. MEMBRANE FORMING COMPOUND SHALL
- ADHERE TO ASTM C 309, TYPE O OR 1D, CLASS B. THE COMPOUND SHALL BE APPLIED IN TWO COATS, EACH AT RIGHT ANGLES TO THE OTHER TO ENSURE A TIGHTLY SEALED SURFACE. B. WET-CURED BY KEEPING THE SURFACE WET AFTER THE CONCRETE HAS SET AND FINISHING IS COMPLETE.

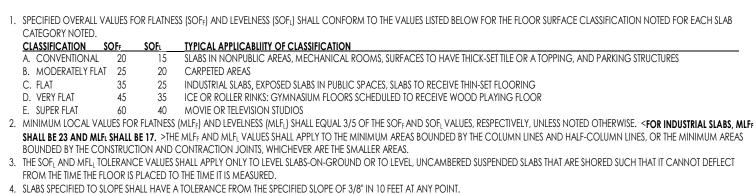
CONCRETE CRACKS

- 1. EVEN WITH PROPER DESIGN AND CONSTRUCTION ALL CONCRETE WILL CRACK. PLASTIC SHRINKAGE CRACKS CONTINUE TO OPEN AS THE SLAB AGES UP TO APPROXIMATELY ONE YEAR, AND REACH 50% OF THEIR FINAL SIZE IN APPROXIMATELY 30 DAYS, MANY PLASTIC SHRINKAGE CRACKS ARE VERY SMALL WHICH MAKE THEM BARELY NOTICEABLE AND INCONSEQUENTIAL TO THE STRUCTURAL
- FINISHING AND CURING REQUIREMENTS. IN ADDITION TO BEING VISIBLY OBJECTIONABLE, IF THESE CRACKS EXIST IN REGULAR CONSISTENCY, THEY MAY REDUCE THE STRUCTURAL PERFORMANCE OF THE CONCRETE AND REQUIRE STRUCTURAL REPAIR (FILL CRACKS WITH EPOXY PRODUCT) OR REPLACEMENT. 2. PLASTIC SHRINKAGE CRACKS: OCCUR SOON AFTER THE CONCRETE IS PLACED AND WHILE IT IS STILL PLASTIC. IT IS CAUSED BY OVERLY RAPID DRYING OF THE SURFACE, USUALLY DUE TO HOT WEATHER, HIGH WIND, LOW HUMIDITY, OR A DELAY IN APPLYING THE CURING MEMBRANE.

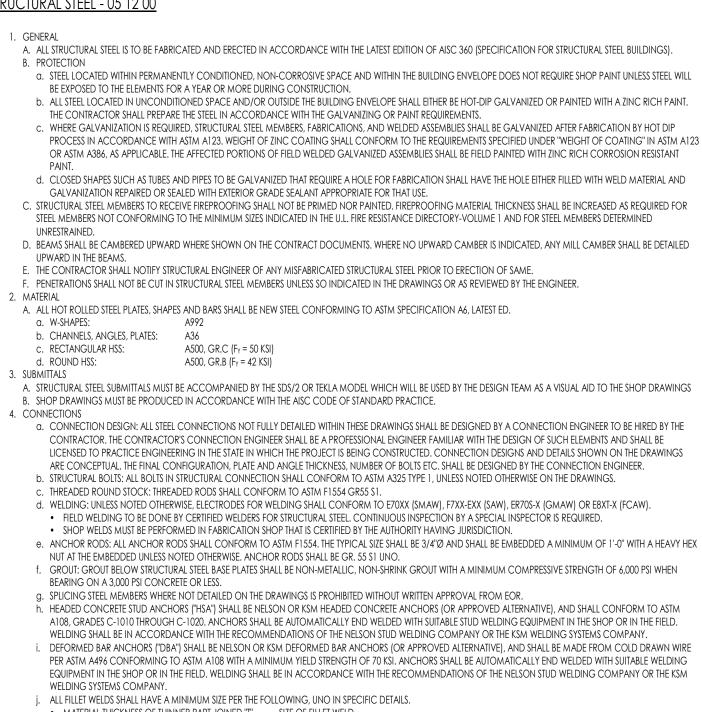
ETEMPERING (ADDING WATER TO CONCRETE ON-SITE)

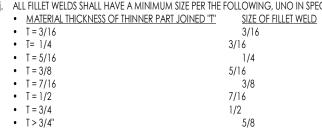
1. WATER SHALL NOT BE ADDED TO THE MIX TRUCKS ON THE JOB SITE IN EXCESS OF THE VOLUME OF WATER THAT IS SPECIFICALLY INDICATED TO HAVE BEEN WITHHELD FROM THE READY MIX . PRIOR TO ADDING WATER, THE CONTRACTOR SHALL CONFIRM THAT THE MIX IS NOT ALREADY WITHIN TOLERANCE ON SLUMP. WATER SHALL ONLY BE ADDED IF THE SLUMP IS BELOW TOLERANCE AND THE READY MIX SUPPLIER HAD INDICATED THE VOLUME OF WITHHELD (TRIM) WATER.

FLOOR FLATNESS AND LEVELNESS



STRUCTURAL STEEL - 05 12 00





PRELIMINARY - NOT FOR CONSTRUCTION

MAX CL-	max fly ash	MAX W/CM RATIO	MAX COARSE AGG. SIZE	MIN. AIR CONTENT
0.30	20%	0.45	1"	N/A

BLEED WATER HAS EVAPORATED AND THE WATER SHEEN HAS DISAPPEARED FROM THE SURFACE. TROWELLING THE WET SURFACE WILL WEAKEN IT AND CAN RESULT IN SURFACE CRAZING AND

- 2. EXCESSIVE BLEED WATER REMOVAL: BLEEDING (FREE SURFACE WATER) OCCURS AS AGGREGATES SETTLE IN THE PLACED CONCRETE, DISPLACING WATER TO THE SURFACE. IF ALLOWED TO REMAIN ON THE SURFACE, IT DILUTES THE CEMENT CONTENT, SIGNIFICANTLY REDUCING THE STRENGTH NEAR THE SURFACE. THE CONTRACTOR SHALL REMOVE BLEED WATER. ONE METHOD OF REMOVING

PERFORMANCE OF THE CONCRETE. CRACKS WIDER THAN APPROXIMATELY 0.06" ARE LIKELY INDICATIVE OF CONCRETE THAT DID NOT ADHERE TO THE CONCRETE MIX REQUIREMENTS, PLACEMENT,

- SLABS IN NONPUBLIC AREAS, MECHANICAL ROOMS, SURFACES TO HAVE THICK-SET TILE OR A TOPPING, AND PARKING STRUCTURES

WOOD FRAMING SPECIFICATIONS (06 10 00): 1. WOOD FRAMING SIZES, FIRESTOPS, ANCHORAGE, FURRING AND CONNECTORS NOT SHOWN ON THE CONSTRUCTION DOCUMENTS SHALL AT A MINIMUM ADHERE TO THE

PRESCRIPTIVE DESIGN PER THE BUILDING CODE. 2. STRUCTURAL LUMBER IN PERMANENTLY CONDITIONED SPACE SHALL MEET OR EXCEED THE FOLLOWING GRADES, PRODUCT LINE OR CRITERIA:

- a. STUD GRADE SOUTHERN YELLOW PINE b. STUD GRADE DOUGLAS FIR-LARCH
- C. VERTICAL STUD USE ONLY CERTIFIED FINGER-IOINTED OF HEM-FIR. SOUTHERN PINE OR DOUGLAS-FIR HRA DESIGNATION (HEAT RESISTANT ADHESIVE) REQUIRED FOR STUDS IN FIRE-RESISTANCE RATED ASSEMBLIES. B. JOISTS:
- a. NO.2 GRADE SOUTHERN YELLOW PINE b. NO.2 GRADE DOUGLAS FIR-LARCH
- C. LAMINATED VENEER LUMBER (LVL) BEAM / HEADER / GIRDER a. WEYERHAEUSER 2.0E MICROLAM LVL
- b. BOISE CASCADE VERSA-LAM 2.0E 3100Fb D. GLUED LAMINATED (GLULAM) BEAM / HEADER / GIRDER:
- a. 31/2" & 51/2" WIDE: ANTHONY POWER BEAM 3000Fb 2.1E 300Fv b. 7" WIDE: ANTHONY POWER BEAM 2800Fb - 2.1E - 300Fv E. RAFTERS:
- a. NO.2 GRADE SOUTHERN YELLOW PINE b. NO.2 GRADE DOUGLAS FIR-LARCH
- c. NO.2 STRUCTURAL FINGER-JOINTED OF HEM-FIR, SOUTHERN PINE OR DOUGLAS FIR HRA DESIGNATION (HEAT RESISTANT ADHESIVE) REQUIRED FOR STUDS IN FIRE-RESISTANCE RATED ASSEMBLIES F POSTS
- a. DIMENSIONAL LUMBER/TIMBERS: NO.2 GRADE SOUTHERN YELLOW PINE OR DOUGLAS FIR-LARCH b. PARALLEL STRAND LUMBER (PSL): TRUSJOIST 1.8E PARALLAM PSL POSTS
- G. PLATES: a. NO.3 GRADE SOUTHERN YELLOW PINE
- b. NO.3 GRADE DOUGLAS FIR-LARCH H. BLOCKING
- a. NO.3 GRADE SOUTHERN YELLOW PINE b. NO.3 GRADE DOUGLAS FIR-LARCH
- 3. STRUCTURAL LUMBER NOT IN PERMANENTLY CONDITIONED SPACE SHALL ADHERE TO THE FOLLOWING SPECIFICATIONS: A. DIMENSION LUMBER
- a. SAME SPECIES AND GRADES AS LISTED ABOVE, HOWEVER THEY MUST BE PRESSURE-TREATED. B GILIED LAMINATED BEAM / HEADERS / GIRDER: a. POWER PRESERVED GLULAM BEAM (24F-V5M1/SP) TREATED WITH COPPER GUARD AT 0.04 PCF OR CLEAR-GUARD AT 0.055 PCF.
- 4. WOOD STRUCTURAL PANEL A. WOOD STRUCTURAL PANELS, WHEN USED STRUCTURALLY (INCLUDING THOSE USED FOR SIDING, ROOF AND WALL SHEATHING, SUBFLOORING, DIAPHRAGMS AND BUILT-UP MEMBERS). SHALL BE APA PERFORMANCE-RATED CONFORMING TO DOC PS 1. DOC PS 2 OR ANSI/APA PRP 210. EACH PANEL OR MEMBER SHALL BE IDENTIFIED FOR GRADE. BOND CLASSIFICATION, AND PERFORMANCE CATEGORY BY THE TRADEMARKS OF AN APPROVED TESTING AND GRADING AGENCY. THE PERFORMANCE CATEGORY VALUE SHALL BE USED AS THE "NOMINAL PANEL THICKNESS" OR "PANEL THICKNESS".
- a. SUBFLOOR: 23/32 PERFORMANCE CATEGORY APA RATED STURD-I-FLOOR. 24 o.c., EXPOSURE 1 b. SHEATHING/DECKING: 7/16 (UNLESS NOTED OTHERWISE, REF SHEAR WALL SCHEDULE) PERFORMANCE CATEGORY APA RATED SHEATHING, 32/16, EXPOSURE 1 B. WOOD STRUCTURAL PANELS WHEN PERMANENTLY EXPOSED IN OUTDOOR APPLICATIONS SHALL BE OF EXTERIOR TYPE, EXCEPT THAT WOOD STRUCTURAL PANEL ROOF SHEATHING EXPOSED TO THE OUTDOORS ON THE UNDERSIDE IS PERMITTED TO BE EXPOSURE 1 TYPE.
- 5. ALL LUMBER SHALL BE KILN DRIED WITH A MAXIMUM MOISTURE CONTENT OF 19%. 6. ALL GLUED-LAMINATED (GLULAM) MEMBERS SHALL ADHERE TO ANSI A 190.1 & ASTM D 3737 AND BE MANUFACTURED BY ANTHONY POWER BEAM (3000Fb - 2.1E).
- 7. ALL LUMBER SHALL BE IDENTIFIED BY THE GRADE MARK OF A LUMBER GRADING OR INSPECTION AGENCY THAT HAS BEEN APPROVED BY AN ACCREDITATION AGENCY THAT COMPLIED WITH DOC PS 20 8. ALL WOOD IN CONTACT WITH CONCRETE OR EXPOSED TO WEATHER SHALL BE PRESERVATIVE TREATED WOOD. ALL FASTENERS INCLUDING ANCHOR BOLTS, POWER-ACTUATED FASTENERS, NAILS, CLIPS, AND HANGERS ATTACHED TO PRESERVATIVE TREATED SHALL BE APPROVED FOR THE ENVIRONMENT.
- 9. NAILS AND STAPLES A. NAILS AND STAPLES SHALL CONFORM TO REQUIREMENTS OF ASTM F 1667. NAILS USED FOR FRAMING AND SHEATHING CONNECTIONS SHALL HAVE MINIMUM AVERAGE BENDING YIELD STRENGTHS AS FOLLOWS: 80 KIPS PER SQUARE INCH (KSI) (551 MPA) FOR SHANK DIAMETERS LARGER THAN 0.177 INCH (4.50 MM) BUT NOT LARGER THAN 0.254 INCH (6.45 MM). 90 KSI (620 MPA) FOR SHANK DIAMETERS LARGER THAN 0.142 INCH (3.61 MM) BUT NOT LARGER THAN 0.177 INCH (4.50 MM) AND 100 KSI (689 MPA) FOR SHANK DIAMETERS OF AT LEAST 0.099 INCH (2.51 MM) BUT NOT LARGER THAN 0.142 INCH (3.61 MM).
- 10. FASTENERS FOR FIRE-RETARDANT TREATED WOOD SHALL BE HOT-DIPPED GALVANIZED STEEL OR STAINLESS STEEL.

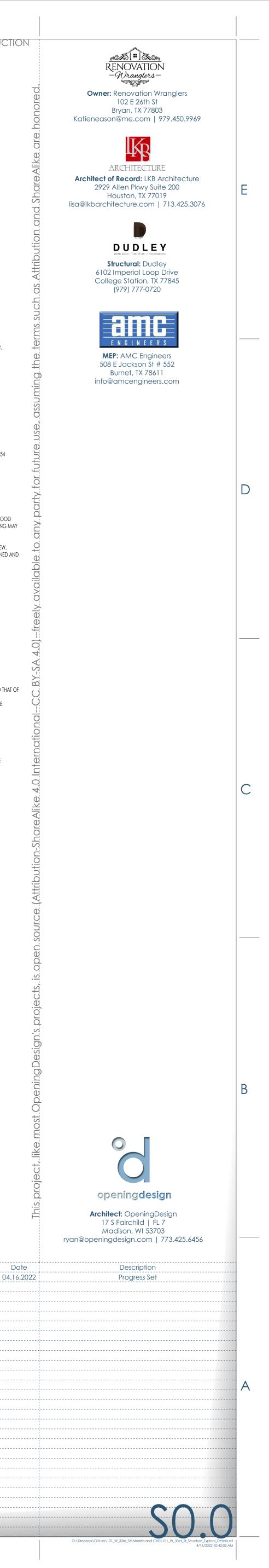
WOOD TRUSS SPECIFICATIONS (06 17 60)

- . TRUSSES SHALL BE DESIGNED BY THE TRUSS MANUFACTURER IN ACCORDANCE WITH THE TRUSS PLATE INSTITUTE "NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION" (ANSI/TPI 1-2014) AND SHALL CONFORM TO IBC §2303.4. THE DESIGN SHALL INCLUDE ALL TEMPORARY AND PERMANENT BRACING. TEMPORARY BRACING MAY REMAIN IN-PLACE IF IT DOES NOT INTERFERE WITH ARCHITECTURAL REQUIREMENTS. 2. SUBMITTALS: THE TRUSS MANUFACTURER SHALL PREPARE AND SUBMIT A TRUSS SUBMITTAL PACKAGE (PRODUCT DATA AND SHOP DRAWINGS) FOR THE WOOD TRUSSES TO THE CONTRACTOR. THE CONTRACTOR AFTER REVIEWING AND APPROVING THE TRUSS SUBMITTAL PACKAGE, SHALL FORWARD THE TRUSS SUBMITTAL PACKAGE TO DUDLEY FOR REVIEW.
- DUDLEY WILL REVIEW THE TRUSS SUBMITTAL PACKAGE FOR GENERAL CONFORMANCE WITH THE STRUCTURAL DRAWINGS. TRUSS SUBMITTAL PACKAGES SHALL BE PREPARED, SIGNED AND SEALED BY AN ENGINEER LICENSED IN THE STATE IN WHICH THE PROJECT IS LOCATED. THE TRUSS SUBMITTAL PACKAGE SHALL INCLUDE AT A MINIMUM: A. PRODUCT DATA AND ICC APPROVAL FOR FRAMING MEMBERS AND FASTENERS THAT HAVE BEEN DESIGNED BY OTHERS. B. SHOP DRAWINGS SHALL INCLUDE AT A MINIMUM:
- a. PROJECT NAME, LOCATION AND BUILDING CODE b. LAYOUTS INCLUDING TEMPORARY AND PERMANENT BRIDGING REQUIREMENTS. c. PROFILES INCLUDING ALL JOINTS, BEARING POINTS, DEFLECTION RATIOS, AND REACTIONS.
- d. BLOCKING REQUIREMENTS e. REQUIRED BEARING WIDTHS
- f. NUMBER OF PLIES IF GREATER THAN ONE a. LUMBER SPECIES AND GRADE
- h. SIZE, GAUGE AND LOCATION OF PLATES . TRUSS TO TRUSS HARDWARE REQUIREMENTS
- NAME AND TRADEMARK OF PLATE MFR AND TRUSS FABRICATOR
- C. CALCULATIONS INCLUDING BUT NOT LIMITED TO: BUILDING CODE DESIGN LOADS
- STRESS REDUCTION FACTORS USED FOR PLATES 3. DRAG TRUSS SHALL BE PROVIDED ABOVE AND BELOW ALL INTERIOR SHEAR WALLS. THE DRAG TRUSSES SHALL BE DESIGNED TO SUPPORT AN ALLOWABLE LINEAR LOAD EQUAL TO THAT OF
- THE SHEAR WALL (IF SHEAR WALLS ARE ABOVE AND BELOW THE LARGE ALLOWABLE SHEAR LOAD SHALL APPLY). 4. TRUSS RESTRAINT/BRACING METHODS SHALL BE IN ACCORDANCE WITH BCSI-B3 (PERMANENT RESTRAINT/BRACING OF CHORDS AND WEB MEMBERS) UNLESS NOTED OTHERWISE
- 5. TRUSS DEFLECTION LIMITS: TRUSSES SHALL BE LIMITED TO THE FOLLOWING DEFLECTION LIMITS: A. FLOOR TRUSSES LIVE LOAD (L/360) TOTAL LOAD (L/240
- B. PITCHED ROOF TRUSS : LIVE LOAD (L/240) TOTAL LOAD (L/180) C. SHALLOW (< = 4 : 12) PITCHED ROOF TRUSSES : LIVE LOAD (L/360) TOTAL LOAD (L/240)
- 6. CAMBER SHALL BE BUILT INTO ROOF TRUSSES TO COMPENSATE FOR VERTICAL DEFLECTION. THE CAMBER SHALL BE LARGEST AT THE MID-SPAN OF THE TRUSS. A. PITCHED ROOF TRUSS: 1.00 X DEFLECTION FROM ACTUAL DEAD LOAD.
- 7. TRUSSES SPANNING 60 FEET OR FURTHER A. THE TRUSS MFR. SHALL CONTRACT WITH A QUALIFIED REGISTERED DESIGN PROFESSIONAL FOR THE DESIGN OF THE TEMPORARY INSTALLATION RESTRAINT/BRACING AND THE PERMANENT INDIVIDUAL TRUSS MEMBER RESTRAINT/BRACING FOR ALL TRUSSES WITH CLEAR SPANS 60 FEET OR GREATER.

<u>WOOD SHRINKAGE</u>

- A. WOOD SHRINKAGE: a. REGARDLESS OF THE BUILDING TYPE, BUILDING DESIGNS MUST COMPENSATE FOR THE FACT THAT WOOD SHRINKS AS IT DRIES. SHRINKAGE CONTINUES UNTIL WOOD REACHES IT EQUILIBRIUM MOISTURE CONTENT (EMC), WHICH AVERAGES 8-12% OF MOISTURE CONTENT FOR MOST STRUCTURES IN THE U.S... THE CONTRACTOR SHALL
- PREPARE AND ENFORCE A PLAN TO MINIMIZE MOISTURE IN THE WOOD FRAMING. DRYWALL SHALL NOT BE INSTALLED UNTIL THE MOISTURE CONTENT OF ALL WOOD FRAMING IS BELOW 15% b. THE CONSTRUCTION OF A WOOD FRAMED BUILDING REQUIRES AN UNDERSTANDING OF FRAMING TOLERANCES, SHRINKAGE, AND INTERACTION WITH DISSIMILAR
- c. ROUGH OPENINGS IN EXTERIOR WALLS SHALL BE UPSIZED APPROXIMATELY 1/2" TO ACCOMMODATE SHRINKAGE.
- d. PROVIDE 1/8" WIDE JOINTS IN SHEATHIN e. THE CONTRACTOR SHALL INCORPORATE DIFFERENTIAL VERTICAL MOVEMENT INTO THE DESIGN OF THE PLUMBING SYSTEM INCLUDING VERTICAL EXPANSION JOINTS, GAPS AROUND HORIZONTAL PLUMBING RUNS, AVOIDING HORIZONTAL PLUMBING RUNS IN LOAD BEARING STUDS.
- f. CARE SHALL BE TAKEN DURING CONSTRUCTION TO LIMIT THE MOISTURE EXPOSURE OF THE LUMBER. IF THE LUMBER DOES BECOME WET, MEASURES SHALL BE TAKEN TO BRING THE MOISTURE CONTENT BACK TO OR BELOW 15% PRIOR TO INSTALLING ARCHITECTURAL FINISHES. g. THE APPROXIMATE WOOD SHRINKAGE ASSUMING ALL LUMBER IS SOUTHERN PINE WITH AN INSTALLED MOISTURE CONTENT OF 19% AND A FINAL MOISTURE CONTENT OF 10% IS AS FOLLOWS:
- 3rd STORY BOTTOM PLATE:
- FLOOR TRUSS: DOUBLE TOP PLATE: 0.067"
- 2ND STORY BOTTOM PLATE: 0.168 FLOOR TRUSS:
- DOUBLE TOP PLATE: 0.067" 1ST STORY
- BOTTOM PLATE: 0.034" TOTAL ESTIMATED SHRINKAGE: [0.370"]

TYPICAL GENERAL NOTES



REQUIRED VERIFICATION AND INSPECTION OF GRADING AND DRAINAG	E FOR FOUNDATI
VERIFICATION AND INSPECTION	CONTINUOUS
AFTER BUILDING CONSTRUCTION AND LANDSCAPING HAVE BEEN COMPLETED, FINAL GRADES SHALL BE VERIFIED TO DOCUMENT REQUIRED DRAINAGE	-
AFTER BUILDING CONSTRUCTION AND LANDSCAPING HAVE BEEN COMPLETED, DOWNSPOUTS SHALL BE INSPECTED TO CONFIRM CONFORMANCE.	-
GRADES AROUND THE STRUCTURE SHALL BE PERIODICALLY INSPECTED AND ADJUSTED AS PART OF THE BUILDING'S MAINTENANCE PROGRAM	-
PLUMBING LEAK "HYRDROSTATIC" TEST PERFORMED BY A LICENSED PLUMBER. TEST TO OCCUR AFTER ROUGH PLUMBING INSTALL	-
WHERE PAVING/FLATWORK ABUT THE FOUNDATION, A MAINTENANCE PROGRAM SHALL BE ESTABLISHED TO EFFECTIVELY SEAL AND MAINTAIN JOINTS AND PREVENT SURFACE WATER INFILTRATION.	-

REQUIRED VERIFICATION AND INSPECTION OF SOILS (TABLE 1705.6)					
VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REQUIRED		
VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY	-	Х	YES		
VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIALS	-	Х	YES		
PERFORM CLASSIFICATION AND TESTING OF COMPACTED MATERIALS	-	Х	YES		
VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESS DURING PLACEMENT AND COMPACTION OF COMPACTED FILL	Х	-	YES		
PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THE SITE HAS BEEN PREPARED PROPERLY	-	Х	YES		

REQUIRED VERIFICATION AND INSPECTION OF WOOD	CONSTRUCTION (§17	<i>'</i> 0
VERIFICATION AND INSPECTION	CONTINUOUS	
PREFABRICATED WOOD STRUCTURAL ELEMENTS (METAL PLATE CONNECTED WOOD TRUSSES) FABRICATION AND IMPLEMENTATION PROCEDURES (NOT REQUIRED WHERE THE WORK IS DONE ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION	-	
HIGH-LOAD DIAPHRAGMS	-	
 A. INSPECT GRADE AND THICKNESS OF WOOD STRUCTURAL PANEL SHEATHING. B. VERIFY NOMINAL SIZE OF FRAMING MEMBERS AT ADJOINING PANEL EDGES, THE NAILS OR STAPLE DIAMETER AND LENGTH, THE NUMBER OF FASTENER LINES AND THAT THE SPACING BETWEEN FASTENERS IN EACH LINE AND AT EDGE MARGINS AGREES WITH THE APPROVED BUILDING PLANS 		
METAL-PLATE-CONNECTED WOOD TRUSSES SPANNING 60 FT OR GREATER	-	
A. VERIFY THAT TEMPORARY INSTALLATION RESTRAINT/BRACING AND THE PERMANENT INDIVIDUAL TRUSS MEMBER RESTRAIN/BRACING ARE INSTALLED IN ACCORDANCE WITH THE APPROVED TRUSS SUBMITTAL PACKAGE		
INSPECTION OF NAILING, BOLTING, ANCHORING AND OTHER FASTENING COMPONENTS WITHIN THE SEISMIC / MAIN WIND FORCE RESISTING SYSTEM, INCLUDING WOOD SHEAR WALLS, WOOD DIAPHRAGMS, DRAG STRUTS, BRACES, SHEAR WALLS AND HOLD-DOWNS.	-	
 MOISTURE CONTENT OF LOAD BEARING WOOD FRAMING: MOISTURE CONTENT JUST PRIOR TO INSTALLING SHEET ROCK SHOULD BE AT OR BELOW 15%. SPECIAL ATTENTION SHALL BE PAID TO MEMBERS ORIENTED WITH THEIR VERTICAL AXIS PERPENDICULAR TO THE VERTICAL PLANE (PLATES, JOISTS, TRUSS CHORDS, ETC.) 	-	

STRUCTURAL STATEMENT OF SPECIAL INSPECTIONS & TESTING

- 1. SPECIAL INSPECTIONS AND STRUCTURAL TESTING SHALL BE PROVIDED BY AN INDEPENDENT AGENCY EMPLOYED BY THE OWNER FOR THE ITEMS IDENTIFIED IN THIS SECTION AND IN OTHER AREAS OF THE APPROVED CONSTRUCTION PLANS AND SPECIFICATIONS. (SEE IBC CHAPTER 17). 2. THE NAMES AND CREDENTIALS OF THE SPECIAL INSPECTORS TO BE USED SHALL BE SUBMITTED TO THE BUILDING OFFICIAL FOR APPROVAL. DUDLEY ENGINEERING CAN BE SOLICITED TO PROVIDE SPECIAL INSPECTIONS. WE RECOMMEND THAT THE PROJECT GEOTECHNICAL ENGINEER
- BE SOLICITED TO PROVIDE SPECIAL INSPECTIONS FOR THE SOILS AND TESTING FOR THE SOIL AND CONCRETE.
 DUTIES OF THE SPECIAL INSPECTOR:
- A. THE SPECIAL INSPECTOR SHALL REVIEW ALL WORK LISTED BELOW FOR CONFORMANCE WITH THE APPROVED CONSTRUCTION PLANS AND SPECIFICATIONS AND THE IBC. B. THE SPECIAL INSPECTOR SHALL FURNISH SPECIAL INSPECTION REPORTS TO THE EOR, CONTRACTOR, OWNER AND BUILDING OFFICIAL ON A WEEKLY BASIS, OR MORE FREQUENTLY AS REQUIRED BY THE BUILDING OFFICIAL. ALL ITEMS NOT IN COMPLIANCE SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION, AND IF UNCORRECTED, TO THE EOR AND THE BUILDING OFFICIAL. C. ONCE CORRECTIONS HAVE BEEN MADE BY THE CONTRACTOR, THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL SIGNED REPORT TO THE BUILDING OFFICIAL STATING THAT THE WORK REQUIRING SPECIAL INSPECTION WAS, TO THE BEST OF THE SPECIAL INSPECTOR'S KNOWLEDGE, IN
- CONFORMANCE WITH THE APPROVED CONSTRUCTION PLANS AND SPECIFICATIONS AS WELL AS THE APPLICABLE WORKMANSHIP PROVISIONS OF THE IBC. 4. DUTIES AND RESPONSIBILITIES OF THE CONTRACTOR:
- A. THE CONTRACTOR SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE OWNER AND THE BUILDING OFFICIAL PRIOR TO THE COMMENCEMENT OF WORK. IN ACCORDANCE WITH IBC 1704.4, THE STATEMENT OF RESPONSIBILITY SHALL CONTAIN ACKNOWLEDGEMENT OF THE SPECIAL INSPECTION REQUIREMENTS CONTAINED WITHIN THIS "STATEMENT OF SPECIAL INSPECTIONS".
- B. THE CONTRACTOR SHALL NOTIFY THE RESPONSIBLE SPECIAL INSPECTOR THAT WORK IS READY FOR INSPECTION AT LEAST ONE WORKING DAY (24 HOURS MINIMUM) BEFORE SUCH INSPECTION IS REQUIRED. C. ALL WORK REQUIRING SPECIAL INSPECTION SHALL REMAIN ACCESSIBLE AND EXPOSED UNTIL IT HAS BEEN OBSERVED BY THE SPECIAL INSPECTOR.
- PLEASE SEE THE "SPECIAL INSPECTION SCHEDULE" FOR THE TYPES, EXTENTS AND FREQUENCY OF SPECIFIC ITEMS REQUIRING SPECIAL INSPECTIONS AND STRUCTURAL TESTS AS PART OF THIS PROJECT.
 REFER TO ARCHITECTURAL AND/OR MEP DRAWINGS FOR ADDITIONAL SPECIAL INSPECTION REQUIRED. DUDLEY ENGINEERING HAS LISTED THE STRUCTURAL SPECIAL INSPECTIONS AND TESTING.

WIND-RESISTING COMPONENTS (1705.11.3)

PERIODIC SPECIAL INSPECTION IS REQUIRED FOR FASTENING OF THE FOLLOWING SYSTEMS AND COMPONENTS: 1. ROOF COVERING, ROOF DECK AND ROOF FRAMING CONNECTIONS. 2. EXTERIOR WALL COVERING AND WALL CONNECTIONS TO ROOF AND FLOOR DIAPHRAGMS AND FRAMING

ATIONS ON EXPANSIVE SOILS

PERIODIC	REQUIRED
Х	YES

UOUS	PERIODIC	REQUIRED
	Х	YES
	Х	YES
	Х	YES
	-	YES
	Х	YES

N (§1705.5) ____

	CONTINUOUS	PERIODIC	REQUIRED
VOOD WHERE PROVED	-	Х	YES
athing. Ledges, Ener Id at	-	Х	NO
HE STALLED	-	Х	NO
CES,	-	Х	YES
AT OR VITH VISTS,	-	Х	YES

REQUIRED VERIFICATION AND INSPECTION OF STRUCTURAL STEEL CO		g1705.2.1)	
STRUCTURAL STEEL - GENERAL			
THE SPECIAL INSPECTOR SHALL INSPECT THE FABRICATED OR ERECTED STEEL FRAME, AS APPROF SHOWN ON THE CONSTRUCTION DOCUMENTS, SUCH AS BRACES, STIFFENERS, MEMBER LOCATI DETAILS AT EACH CONNECTION.			
STRUCTURAL STEEL - ANCHOR RODS / EMBED PL	ATES		
THE SPECIAL INSPECTOR SHALL BE ON THE PREMISES FOR INSPECTION DURING THE PLACEMENT SUPPORTING STRUCTURAL STEEL FOR COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS. AS LENGTH OF THE ANCHOR RODS OR EMBEDDED ITEM, AND THE EXTENT OR DEPTH OF EMBEDME	A MINIMUM,	THE DIAMETER, GR	RADE, TYPE AND
PRIOR TO PLACEMENT OF CONCRETE.			
STRUCTURAL STEEL - WELDS			
VERIFICATION AND INSPECTION COM	ITINUOUS	PERIODIC	REQUIRED
INSPECTION TASKS PRIOR TO WELDING (AISC 360 TABLE N5.4-1)	1	1	
WELDING PROCEDURE SPECIFICATION (WPS'S) AVAILABLE	Х	-	YES
MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE	Х	-	YES
MATERIAL IDENTIFICATION (TYPE / GRADE)	-	X	YES
	-	X	YES
FIT-UP GROOVE WELDS	-	X	NO
CONFIGURATION AND FINISH OF ACCESS HOLES	-	X	NO
	-	X	YES
	-	Х	YES
USE OF QUALIFIED WELDERS		Х	VES
CONTROL AND HANDLING OF WELDING CONSUMABLES		X	YES
NO WELDING OVER CRACKED TACK WELDS	-	X	YES
ENVIRONMENTAL CONDITIONS (WIND SPEED WITHIN LIMITS, PRECIPITATION AND	_	X	YES
TEMPERATURE		~	
WPS FOLLOWED • SETTINGS ON WELDING EQUIPMENT	-	X	YES
TRAVEL SPEEDSELECTED WELDING MATERIALS			
SHIELDING GAS TYPE / FLOW RATE PREHEAT APPLIED			
 INTERPASS TEMPERATURE MAINTAINED (MIN/ MAX) PROPER POSITION (F, V, H, OH) 			
WELDING TECHNIQUES INTERPASS AND FINAL CLEANING 	-	X	YES
EACH PASS WITHIN PROFILE LIMITATIONS EACH PASS MEET QUALITY REQUIREMENTS			
WELDS CLEANED	-	Х	YES
SIZE, LENGTH AND LOCATION OF WELDS	Х	-	YES
WELDS MEET VISUAL ACCEPTANCE CRITERIA • CRACK PROHIBITION	Х	-	YES
WELD / BASE-METAL FUSION CRATER CROSS SECTION			
WELD PROFILES WELD SIZE			
UNDERCUT POROSITY			
ARC STRIKES	Х	-	YES
k-AREA	Х	-	YES
BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED)	Х	-	YES
REPAIR ACTIVITIES	Х	-	YES
DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT MEMBER	Х	-	YES
	X	-	YES
NON-DESTRUCTIVE TESTING OF WELDED JOI	NTS		
FILLET WELDS:			
MT TEST A MINIMUM OF 10% OF THE LENGTH OF EACH FILLET WELD EXCEEDING 5/16".	-	Х	YES
PERIODIC MT TESTING OF REPRESENTATIVE FILLET WELDS 5/16" AND LESS BUT NEED NOT EXCEED 10% OF ALL SUCH WELDS, EXCEPT AS REQUIRED FOR HIGH REJECTION RATES AS INDICATED IN THE FOLLOWING PARAGRAPH.	-	X	YES
INCREASE MT TESTING RATE FOR WELDERS HAVING A HIGH REJECTION RATE AS REQUIRED TO ENSURE ACCEPTABLE WELDS.	X	-	YES
PARTIAL JOINT PENETRATION (PJP) WELDS INCLUDING FLARE BEVEL WELDS			- .
MT TEST A MINIMUM OF 25% OF THE LENGTH OF EACH PJP WELD EXCEEDING 5/16" EFFECTIVE THROAT.	-	Х	YES
PERIODIC MT TESTING OF REPRESENTATIVE PJP WELDS 5/16" AND LESS BUT NEED NOT EXCEED	_	Х	YES
10% OF ALL SUCH WELDS, EXCEPT AS REQUIRED FOR HIGH REJECTION RATES AS INDICATED IN THE FOLLOWING PARAGRAPH.			
INCREASE MT TESTING RATE FOR WELDERS HAVING A HIGH REJECTION RATE AS REQUIRED TO ENSURE ACCEPTABLE WELDS	Х	-	YES
COMPLETE JOINT PENETRATION (CJP) WELDS	·		
ALL CJP WELDS EXCEEDING 5/16" THICKNESS SHALL BE 100% UT TESTED PER AWS D1.1 CLAUSE 6 PART F. THE TESTING LABORATORY SHALL REVIEW THE CJP JOINTS TO DETERMINE WHERE GEOMETRY OR ACCESSIBILITY PRECLUDES THE USE OF STANDARD SCANNING PATTERNS PER AWS D1.1 CLAUSE 6 PART F. AT THESE LOCATIONS THE TESTING LABORATORY SHALL DEVELOP AND SUBMIT FOR APPROVAL A WRITTEN TESTING PROCEDURE IN ACCORDANCE WITH AWS	X	-	YES
D1.1 ANNEX S. PERIODIC MT TESTING OF REPRESENTATIVE CJP WELDS 5/16" AND LESS NOT TO EXCEED 10%	_	X	YES
OF ALL SUCH WELDS.			
INCREASE MT TESTING RATE FOR WELDERS HAVING A HIGH REJECTION RATE AS REQUIRED TO	Х	-	YES

SUBSEQUENTLY, IT SHALL BE THE UNTURNED ELEMENT B' INITIAL FIT-UP OF THE JOINT OBSERVATION. NO FURTHE	NG: THE INSPECTOR SHALL C ENSURED BY ROUTINE OBSEI Y THE AMOUNT SPECIFIED IN I BUT PRIOR TO PRETENSIONI R EVIDENCE OF CONFORMI REJECTION, A ROTATION THA TION,			
	TABLE 8.2: NUT ROTATION	1		
BOLT LENGTH	DI	5		
	BOTH FACE NORMAL TO BOLT AXIS			
LENGTH≤ 4db	1/3 TURN			
$4d_b < LENGTH \le 8d_b$	1/2 TURN			
8d _b < LENGTH ≤ 12d _b	2/3 TURN	-		
a. NUT ROTATION IS RELATIVE TO BOLT REGARDLESS OF TOLERANCE IS PLUS 60° AND MINUS 30° b. APPLICABLE TO JOINTS IN WHICH ALL MATERIAL WITH				

STRUCTURAL STEEL HI VERIFICATION AND DOCUMENTATION AND ACCEPTANCE OR REJE

STRUCTURAL STEEL H

VERIFICATION AND I DOCUMENTATION OF ACCEPTANCE OR REJECT

3

STRUCTURAL STEEL HIGH-STRENGTH BOLTS (TURN-OF-NUT) ECTOR SHALL OBSERVE THE PRE-INSTALLATION VERIFICATION TESTING REQUIRED IN SECTION 8.2.

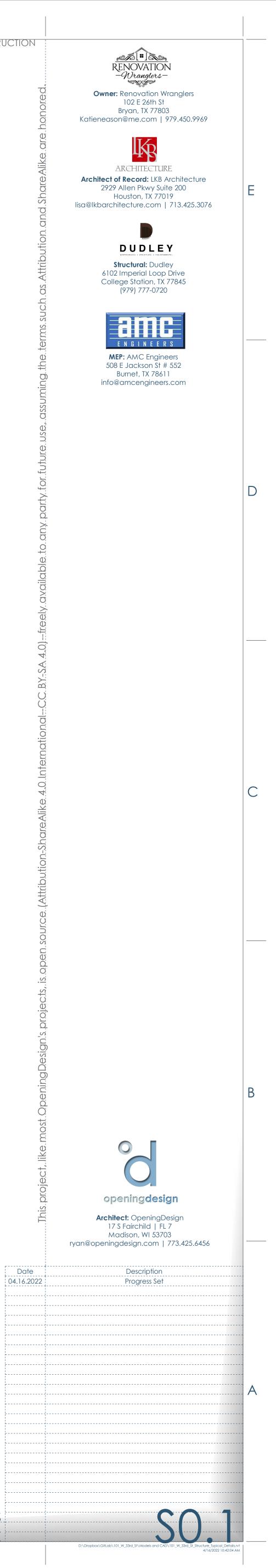
ROUTINE OBSERVATION THAT THE BOLTING CREW PROPERLY ROTATES THE TURNED ELEMENT RELATIVE TO VT SPECIFIED IN TABLE 8.2. ALTERNATIVELY, WHEN FASTENER ASSEMBLIES ARE MATCH-MARKED AFTER THE) PRETENSIONING, VISUAL INSPECTION AFTER PRETENSIONING IS PERMITTED IN LIEU OF ROUTINE OF CONFORMITY IS REQUIRED. A PRETENSION THAT IS GREATER THAN THE VALUE SPECIFIED IN TABLE 8.1 ROTATION THAT EXCEEDS THE REQUIRED VALUES, INCLUDING TOLERANCE, SPECIFIED IN TABLE 8.2 SHALL

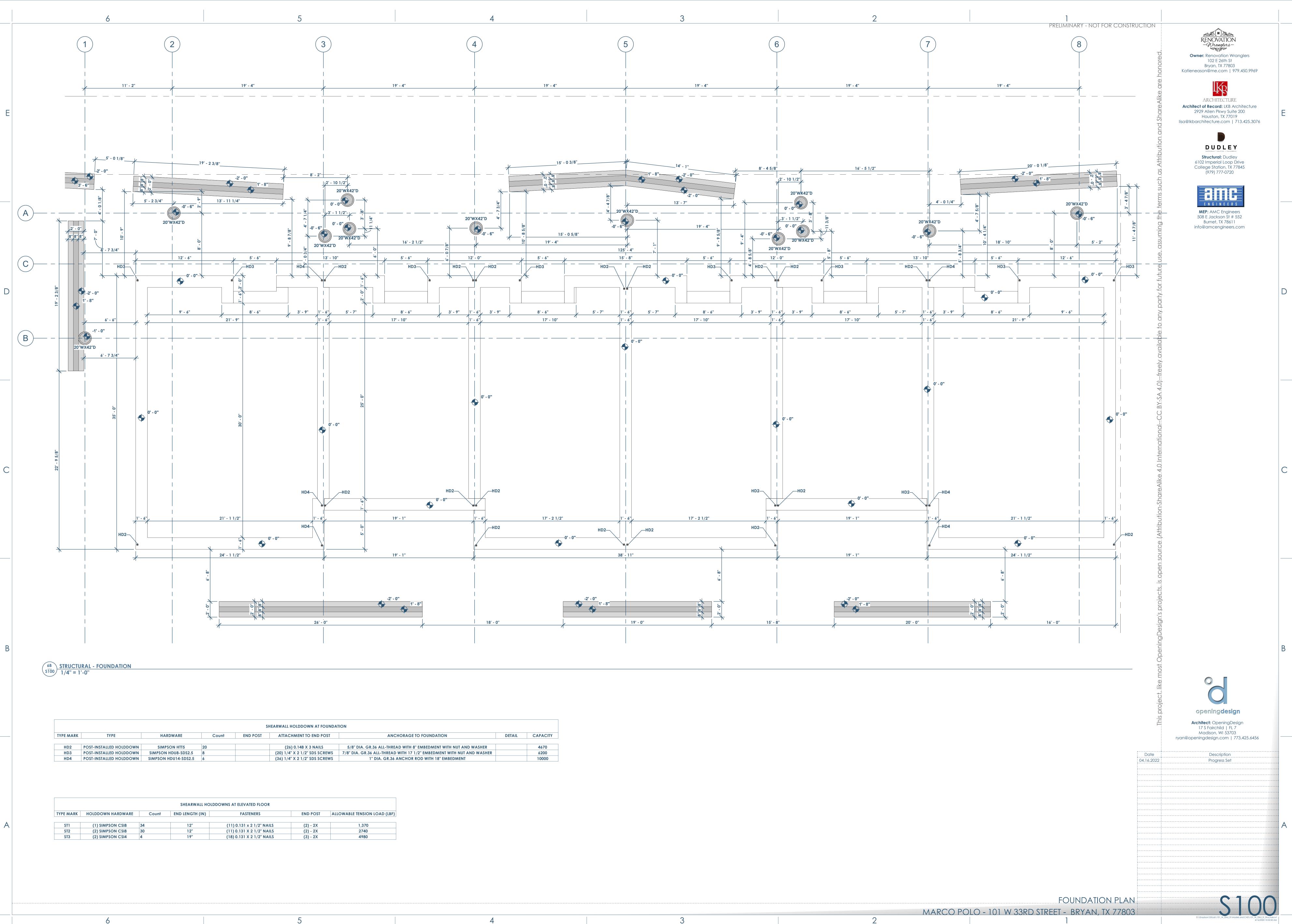
2: NUT ROTATION FROM SNUG-TIGHT CONDITION FOR TURN-OF-NUT PRETENSIONING							
DISPOSITION OF OUTER FACES OF BOLTED PARTS							
e normal to Lt axis	ONE FACE NORMAL TO BOLT AXIS, OTHER SLOPED NOT MORE THAN 1:20	BOTH FACES SLOPED NOT MORE THAN 1:20 FROM NORMAL TO BOLT AXIS					
3 TURN	1/2 TURN	2/3 TURN					
2 TURN	2/3 TURN	5/6 TURN					
3 TURN	5/6 TURN	1 TURN					

EGARDLESS OF THE ELEMENT (NUT OR BOLT) BEING TURNED. FOR ALL REQUIRED ROTATIONS, THE L MATERIAL WITHIN THE GRIP IS STEEL.

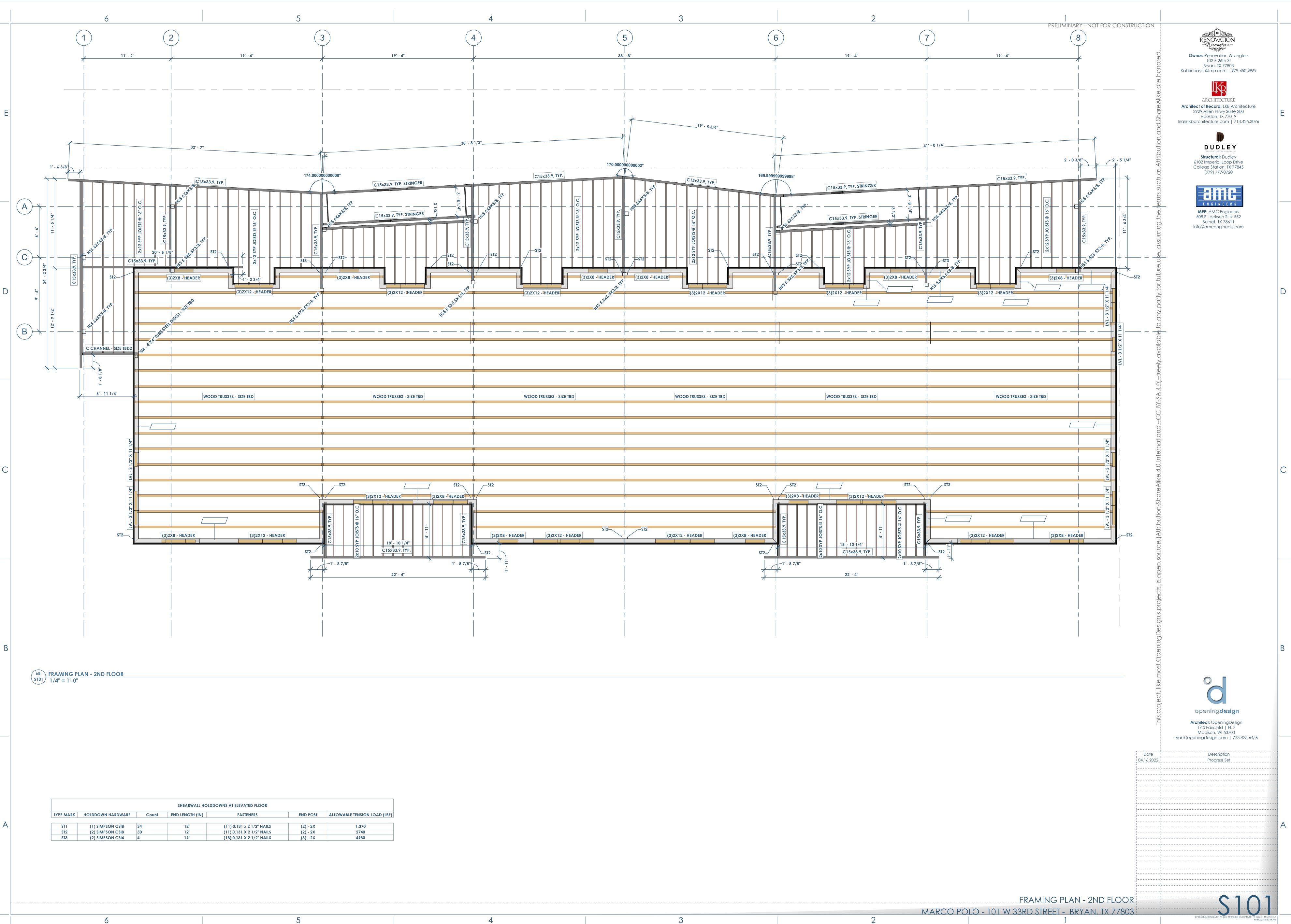
	CONTINUOUS	PERIODIC	REQUIRED
EPTANCE OR REJECTION OF BOLTED CONNECTIONS	-	Х	YES

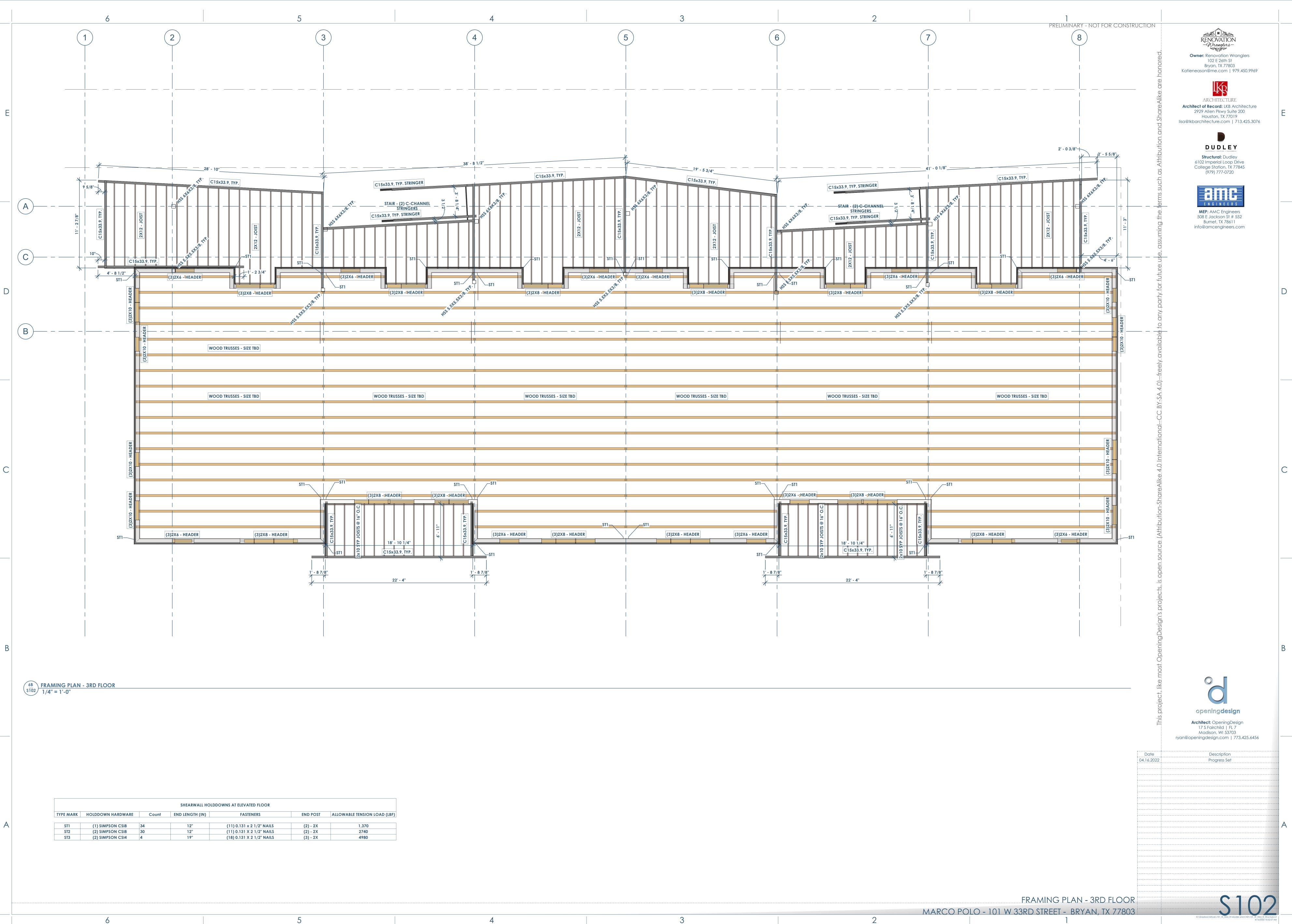
L HIGH-STRENGTH BOLTS (SNUG-TIGHT) - INSPECTION TASKS DURING BOLTING							
ND INSPECTION	CONTINUOUS	PERIODIC	REQUIRED				
JECTION OF BOLTED CONNECTIONS.	-	Х	YES				

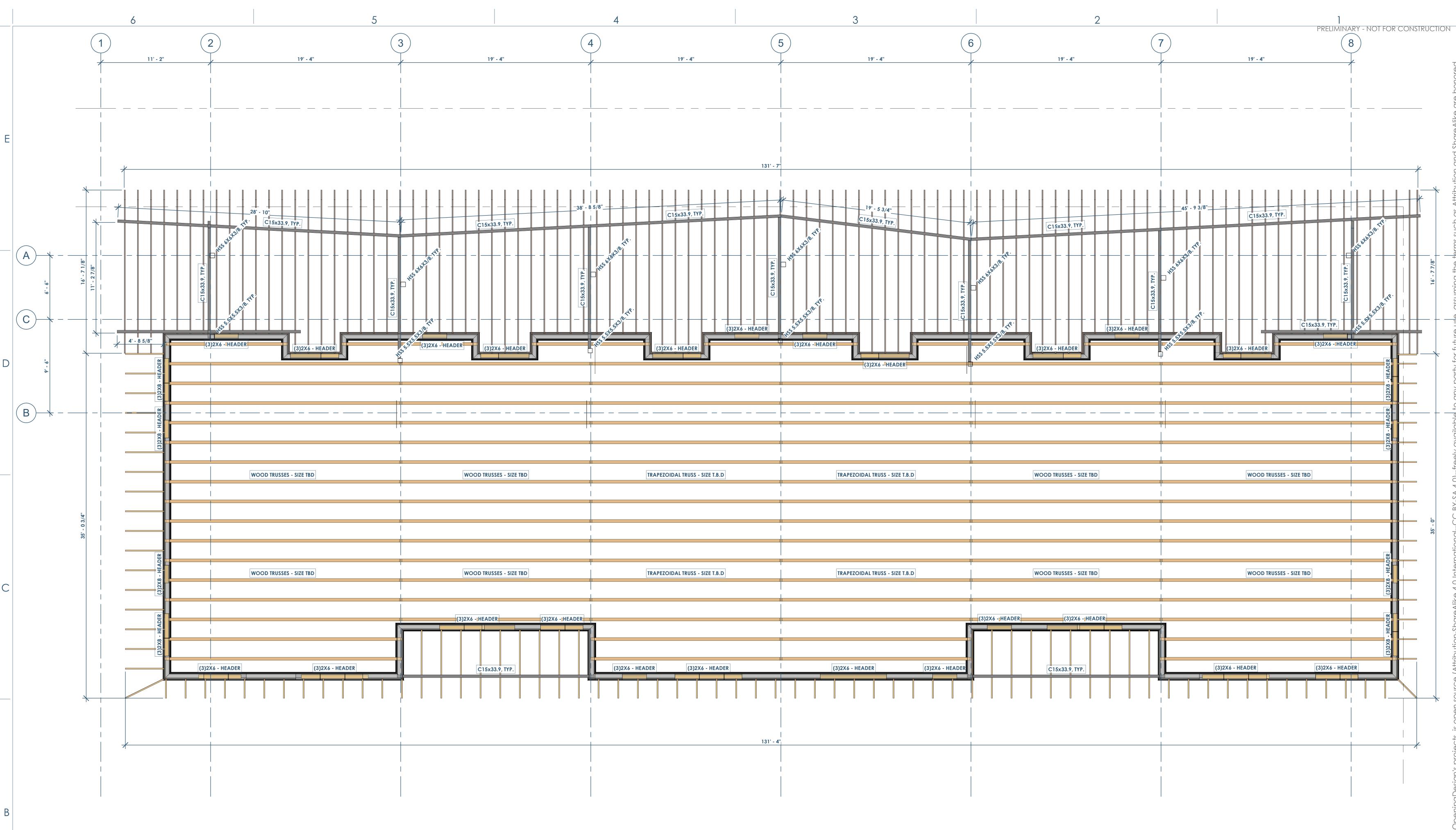




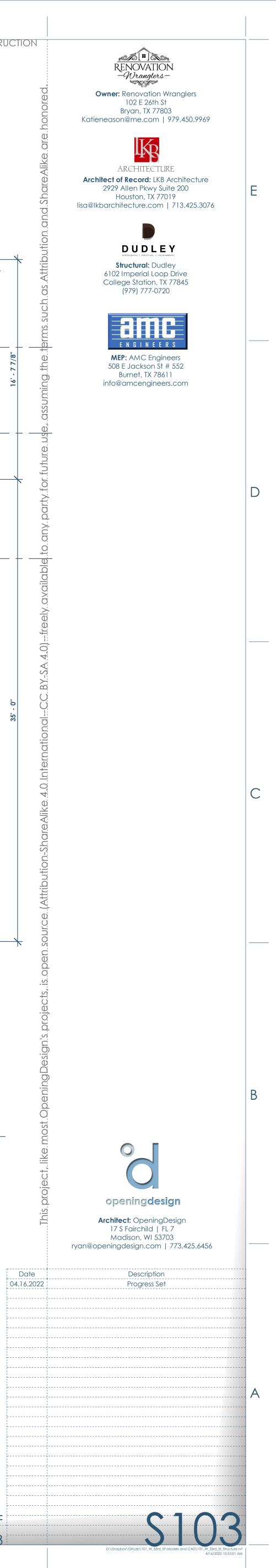
ANCHORAGE TO FOUNDATION	DETAIL	CAPACITY
ALL-THREAD WITH 8" EMBEDMENT WITH NUT AND WASHER		4670
L-THREAD WITH 17 1/2" EMBEDMENT WITH NUT AND WASHER		6200
A. GR.36 ANCHOR ROD WITH 18" EMBEDMENT		10000

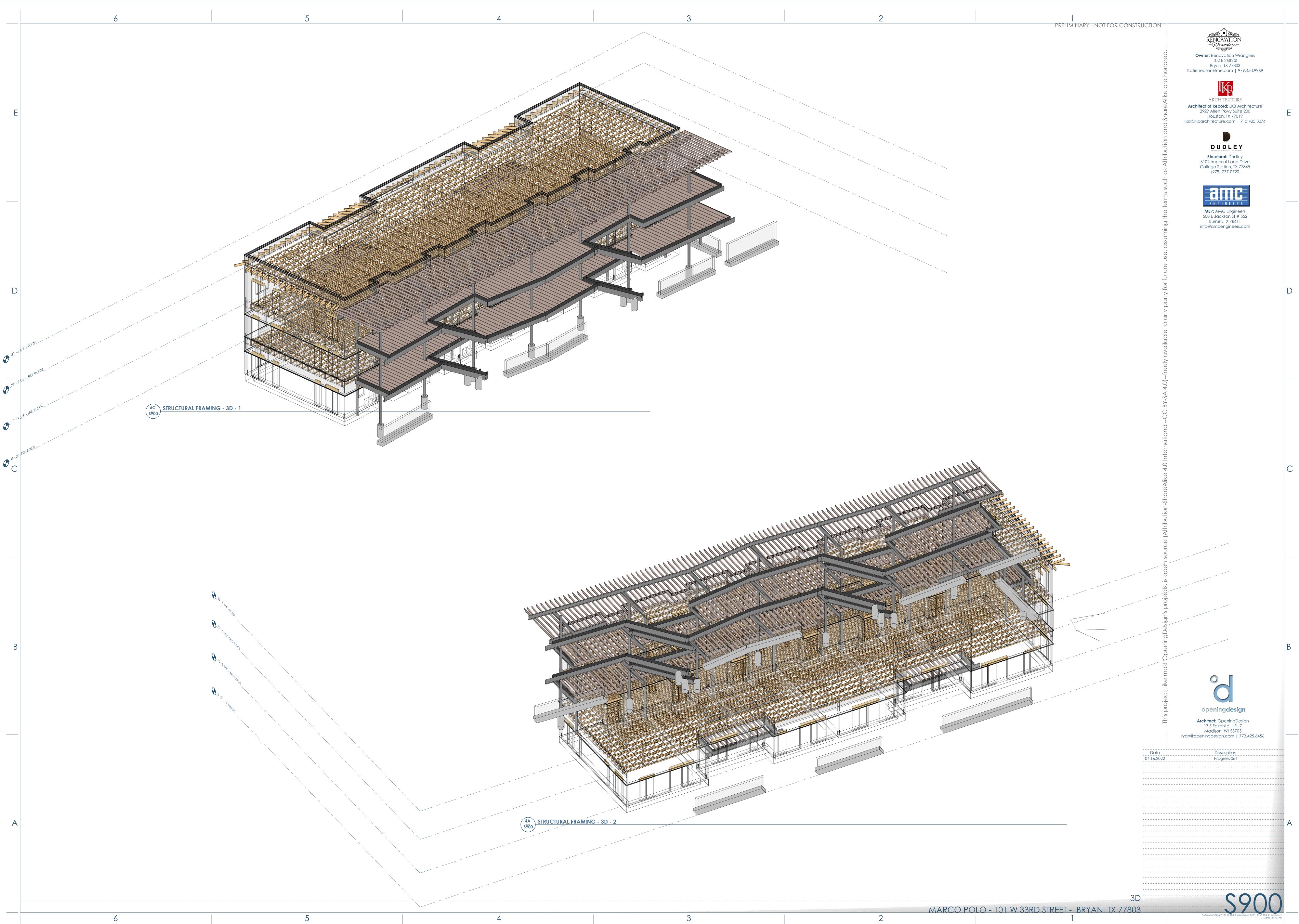


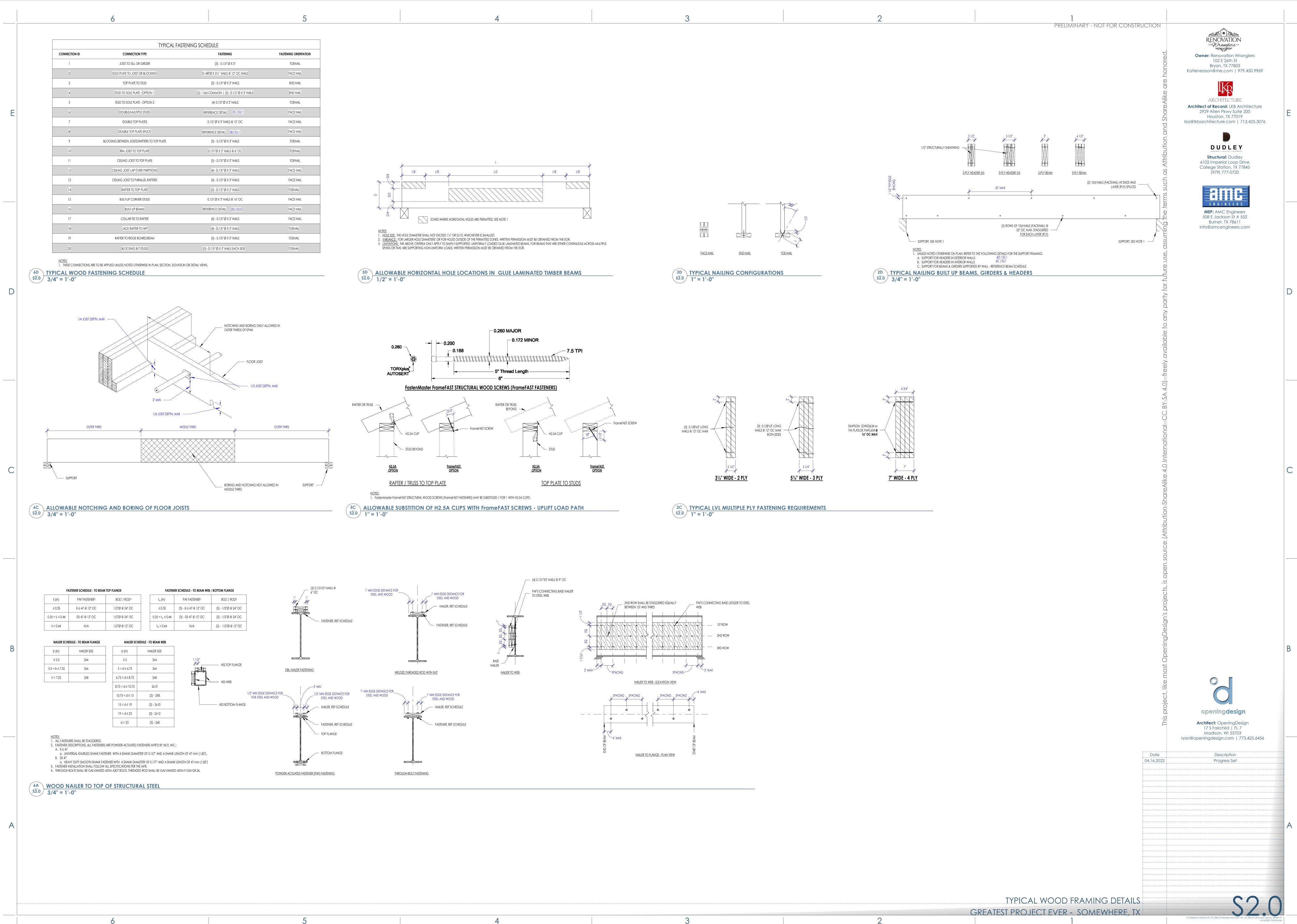


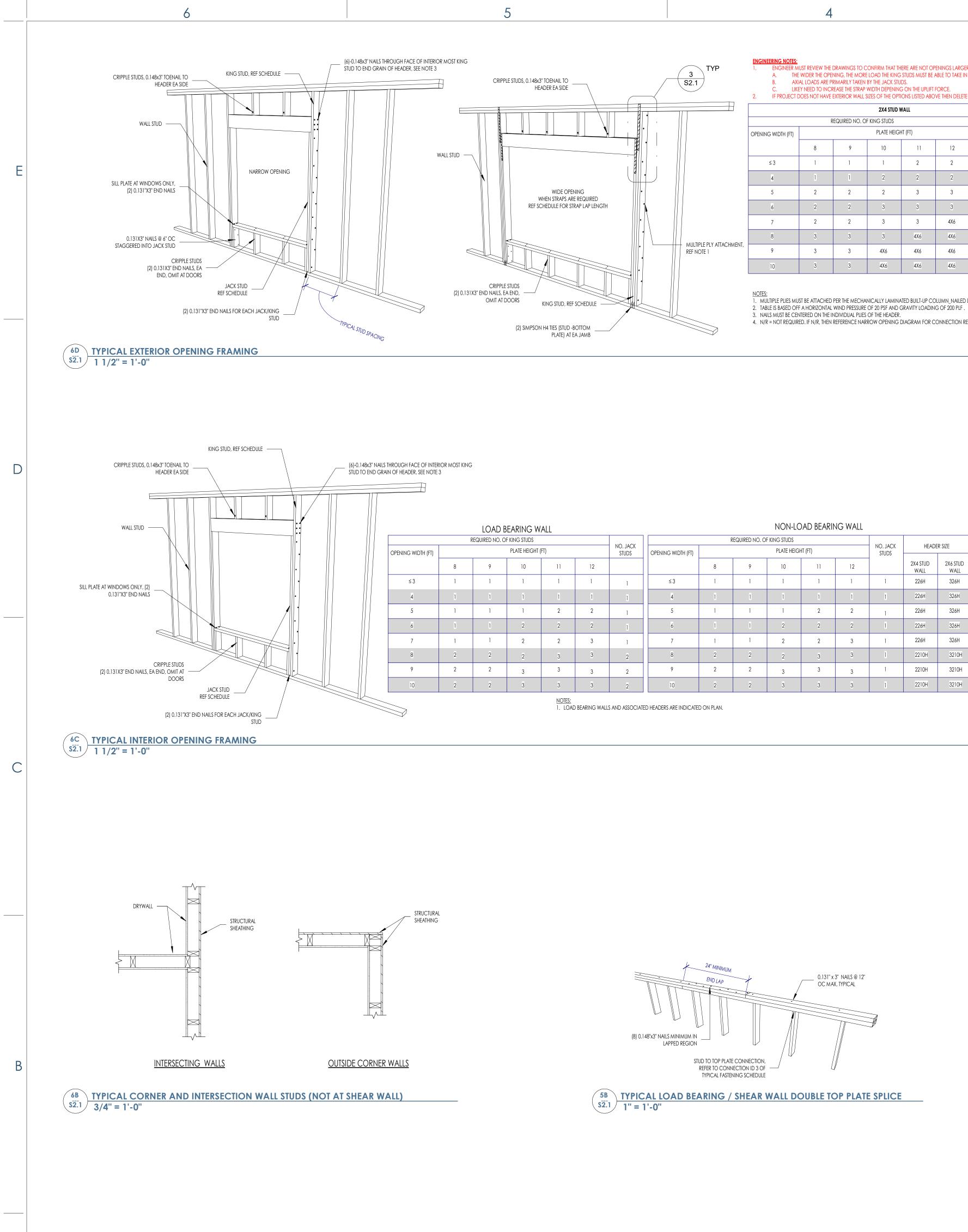


6B S103 FRAMING PLAN - ROOF 1/4" = 1'-0"









ENGINEER MUST REVIEW THE DRAWINGS TO CONFIRM THAT THERE ARE NOT OPENINGS LARGER THAN THAN LISTED IN THE TABLES ABOVE. IF THERE ARE, THEN WE ADD THESE TO OUR SCHEDULE. KEEP IN MIND THE FOLLOWING: A. THE WIDER THE OPENING, THE MORE LOAD THE KING STUDS MUST BE ABLE TO TAKE IN BENDING.

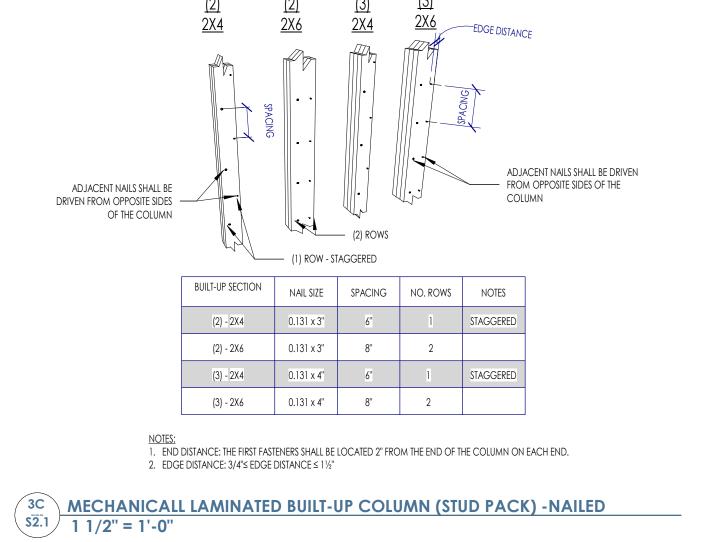
			2X4 STUD V	VALL			
OPENING WIDTH (FT)				NO. JACK STUDS	STRAP LAP LENGTH ⁴		
-	8	9	10	11	12		(IN)
≤3	1	1	1	2	2	1	N/R
4	1	1	2	2	2	1	N/R
5	2	2	2	3	3	1	N/R
6	2	2	3	3	3	1	N/R
7	2	2	3	3	4X6	1	N/R
8	3	3	3	4X6	4X6	2	8
9	3	3	4X6	4X6	4X6	2	8
10	3	3	4X6	4X6	4X6	2	8

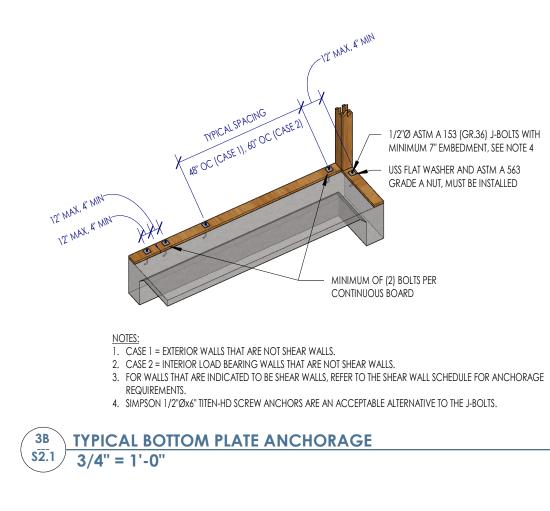
			2X6 STUD V	VALL		
REQUIRED NO. OF KING STUDS						NO. JAC
OPENING WIDTH (FT)	PLATE HEIGHT (FT)					
	8	9	10	11	12	
≤ 3	1	1	1	1	1	1
4	1	1	1	1	1	1
5	1	1	1	1	2	1
6	1	1	1	2	2	1
7	1	1	2	2	2	1
8	1	1	2	2	2	2
9	1	2	2	2	2	2
10	1	2	2	2	3	2

1. MULTIPLE PLIES MUST BE ATTACHED PER THE MECHANICALLY LAMINATED BUILT-UP COLUMN_NAILED DETAIL.

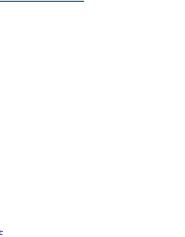
4. N/R = NOT REQUIRED. IF N/R, THEN REFERENCE NARROW OPENING DIAGRAM FOR CONNECTION REQUIREMENTS, OTHERWISE REFERENCE THE WIDE OPENING DIAGRAM.

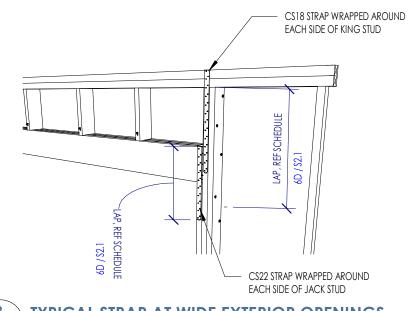
			NON-LC) AD BEARIN	NG WALL			
	REQUIRED NO. OF KING STUDS						HEADER SIZE	
OPENING WIDTH (FT)	PLATE HEIGHT (FT)					NO. JACK STUDS		K JIZE
	8	9	10	11	12		2X4 STUD WALL	2X6 STUD WALL
≤ 3	1	1	1	1	1	1	226H	326H
4	1	1	1	1	1	1	226H	326H
5	1	1	1	2	2	1	226H	326H
6	1	1	2	2	2	1	226H	326H
7	1	1	2	2	3	1	226H	326H
8	2	2	2	3	3	1	2210H	3210H
9	2	2	3	3	3	1	2210H	3210H
10	2	2	3	3	3	1	2210H	3210H





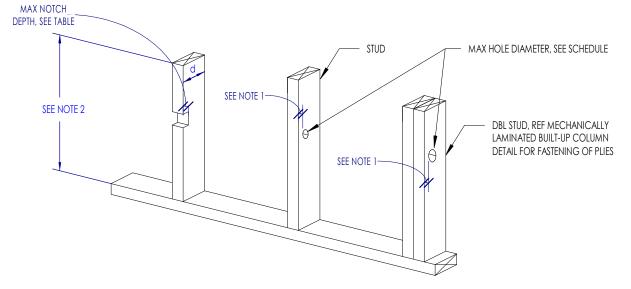






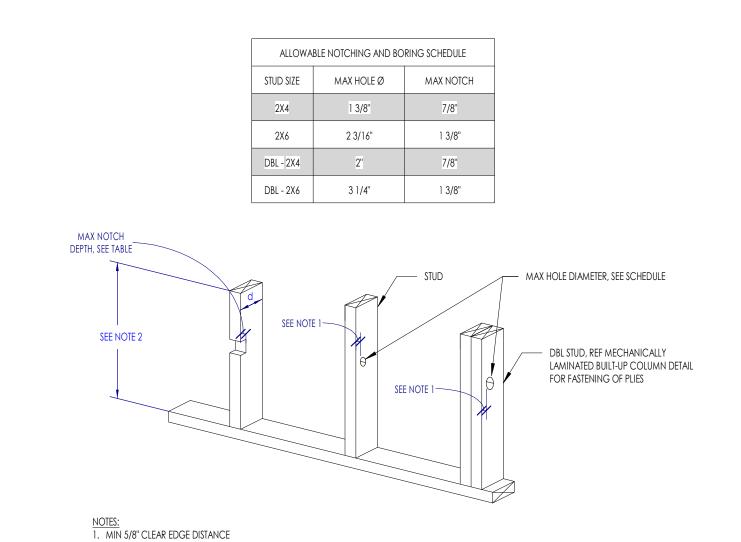


ALLOWABLE NOTCHING AND BORING SCHEDULE							
STUD SIZE	MAX HOLE Ø	MAX NOTCH					
2X4	2"	1 3/8"					
2X6	3 1/4"	2 3/16"					
DBL - 2X4	2"	1 3/8"					
DBL - 2X6	3 1/4"	2 3/16"					



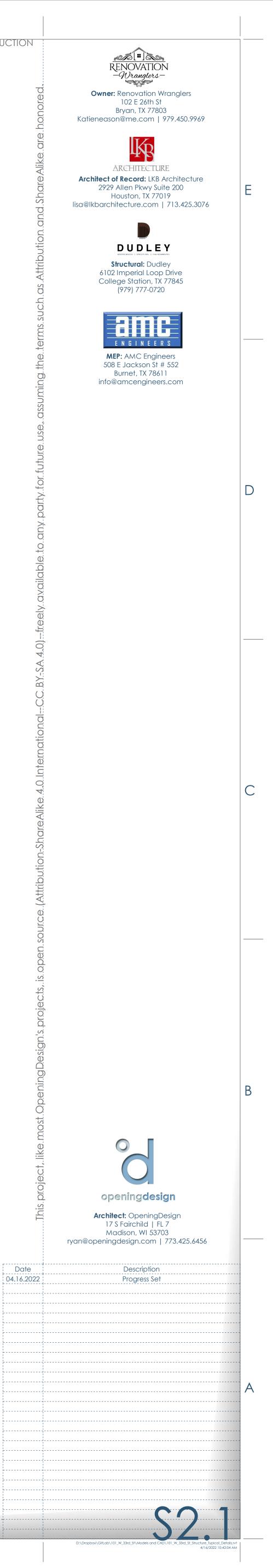


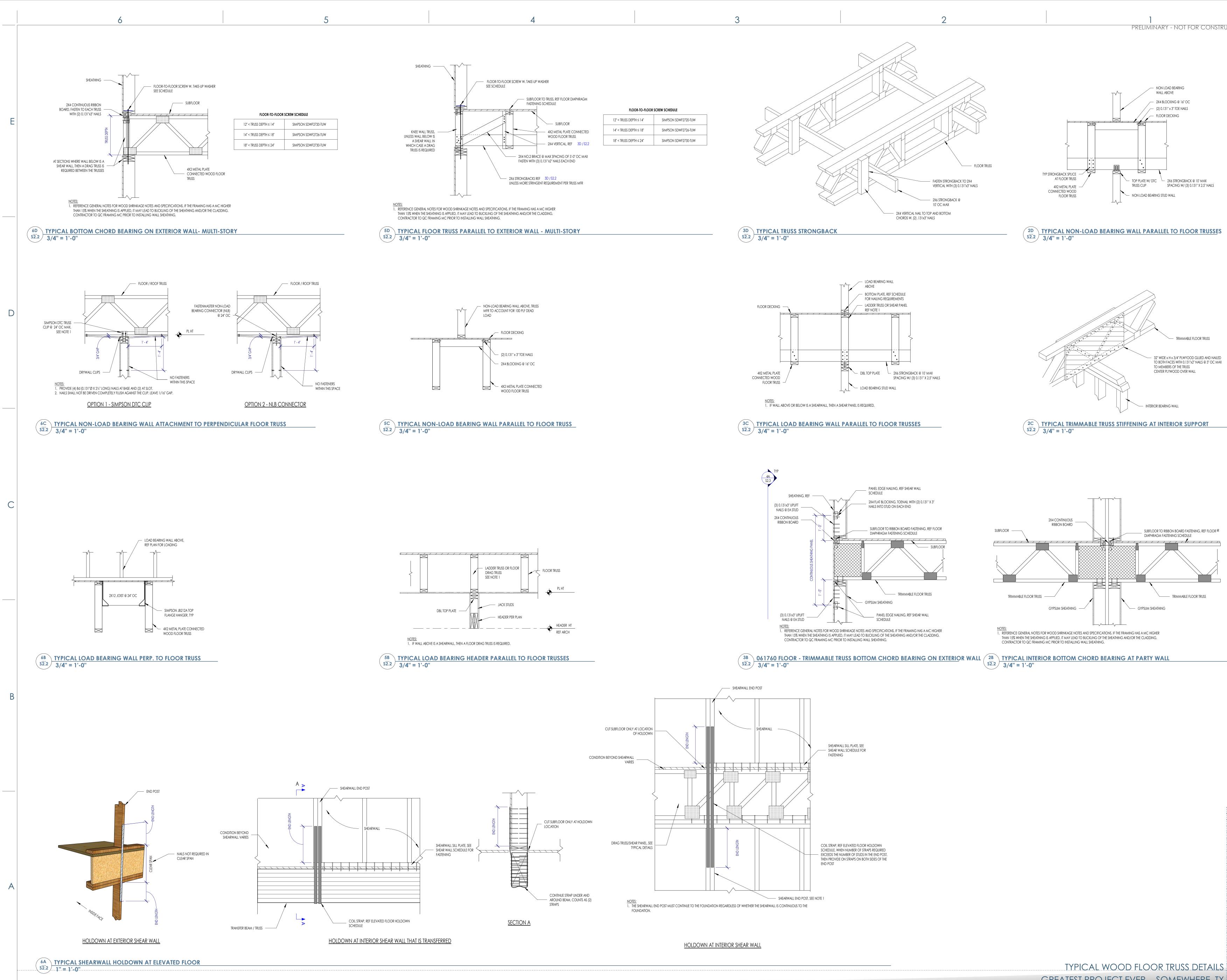




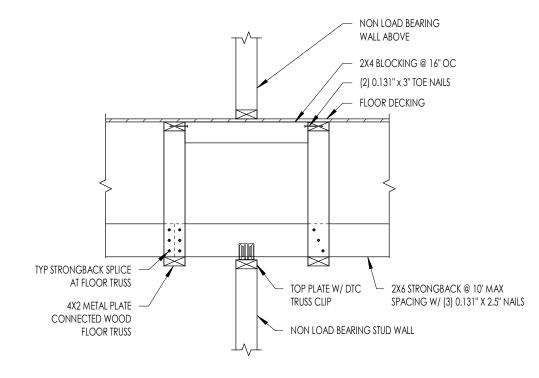
2. NOTCHES IN EITHER SIDE OF A STUD SHALL NOT BE LOCATED WITHIN THE MIDDLE THIRD OF THE STUD LENGTH. 3. NOTCHES AND BORINGS SHALL NOT OCCUR IN THE SAME CROSS SECTION.

2B ALLOWABLE STUD NOTCHING AND BORING IN EXTERIOR & LOAD BEARING WALLS (s2.1) 3/4" = 1'-0"



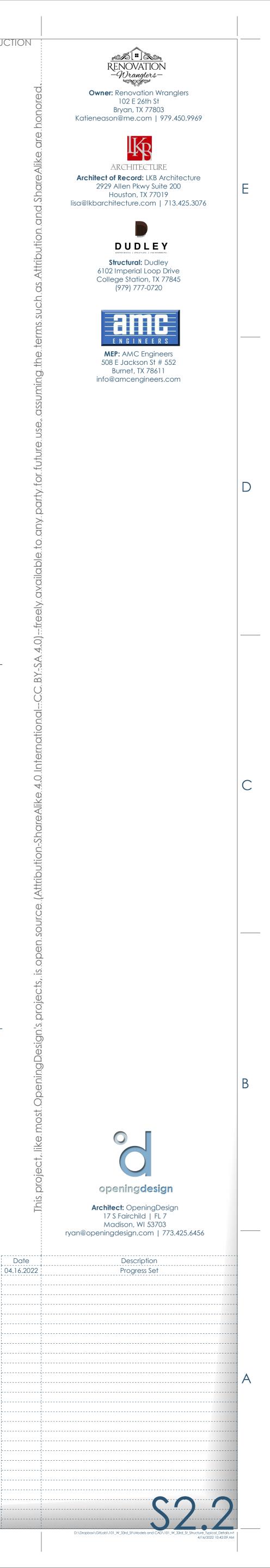






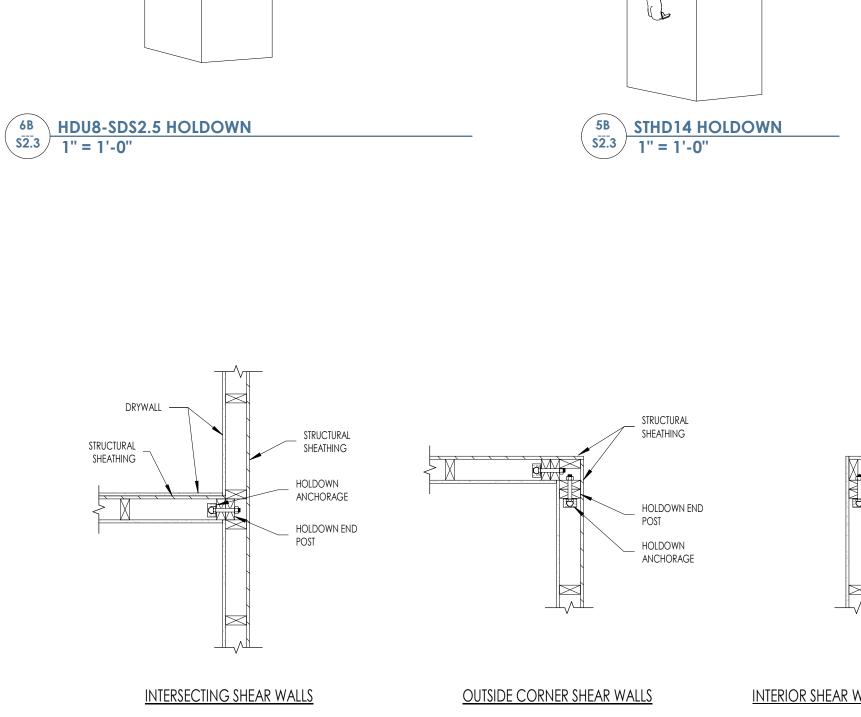


TYPICAL WOOD FLOOR TRUSS DETAILS GREATEST PROJECT EVER - SOMEWHERE, TX

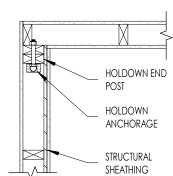




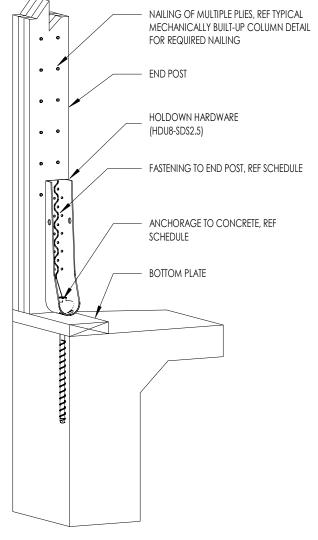


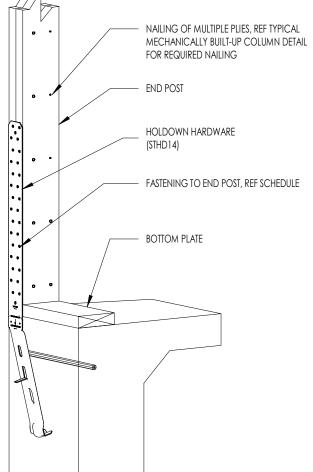


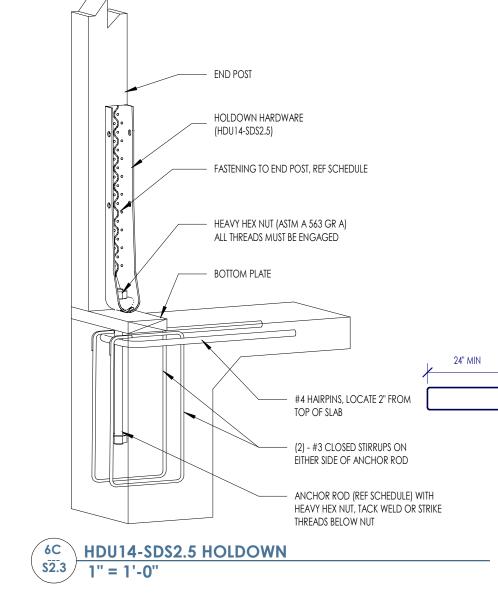


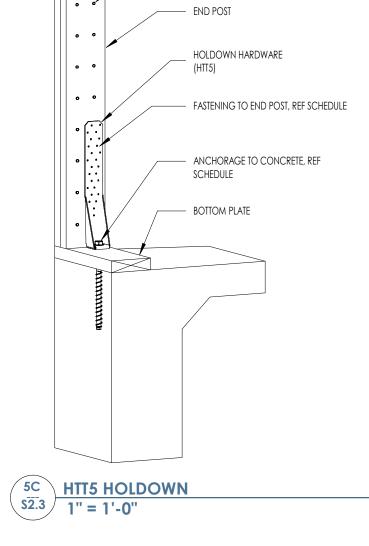










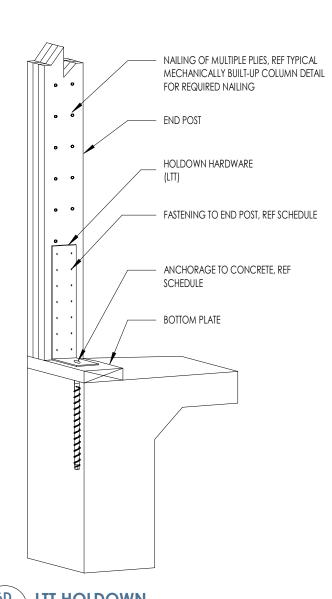


NAILING OF MULTIPLE PLIES, REF TYPICAL

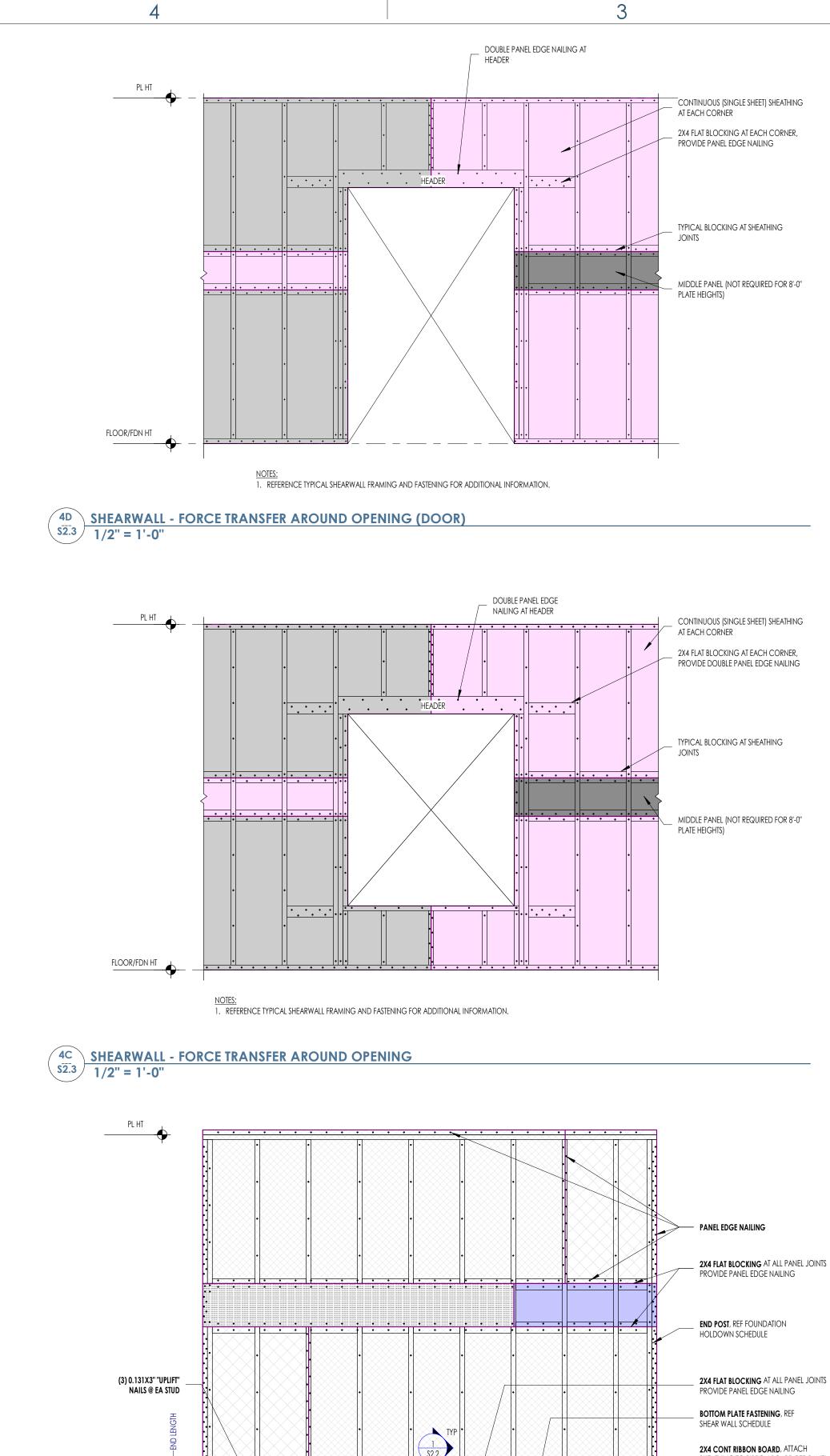
MECHANICALLY BUILT-UP COLUMN DETAIL

FOR REQUIRED NAILING



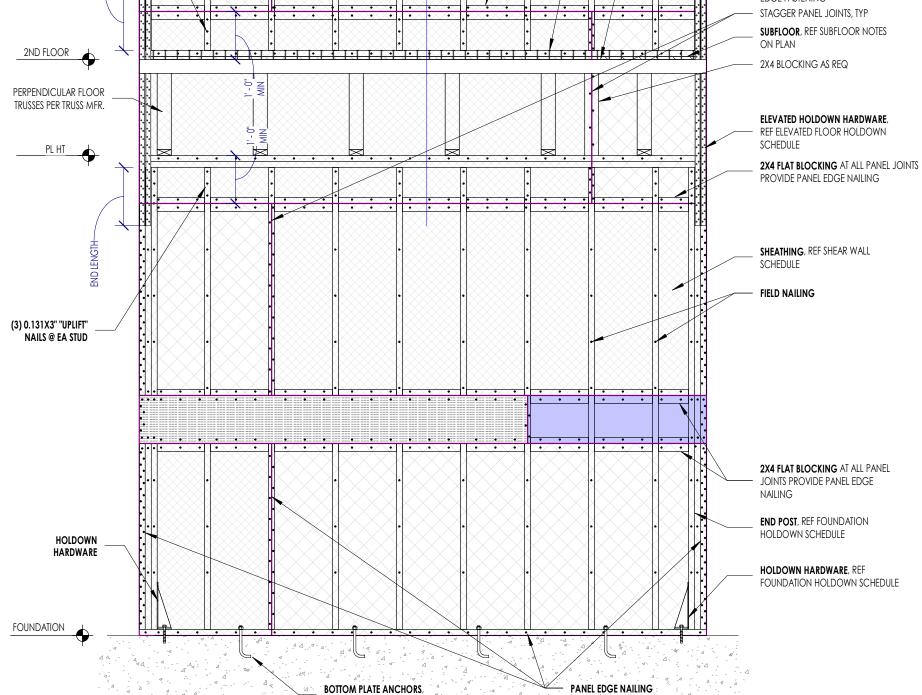






- SHEATHING TO RIBBON BOARD PER PANEL EDGE FASTENING SUBFLOOR, REF SUBFLOOR NOTES ON PLAN

ELEVATED HOLDOWN HARDWARE, - REF ELEVATED FLOOR HOLDOWN SCHEDULE

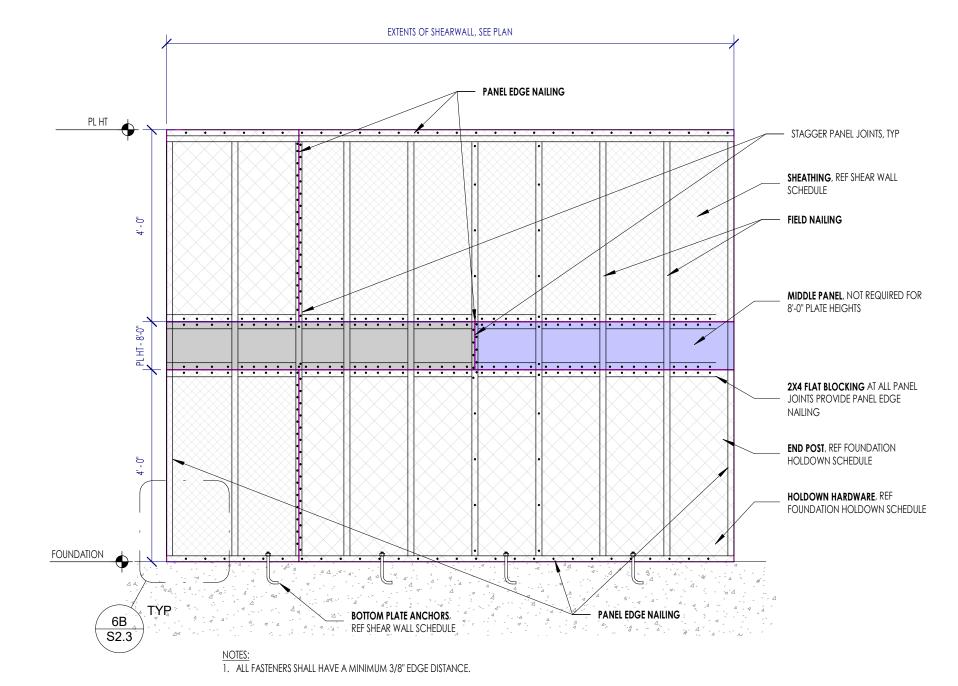


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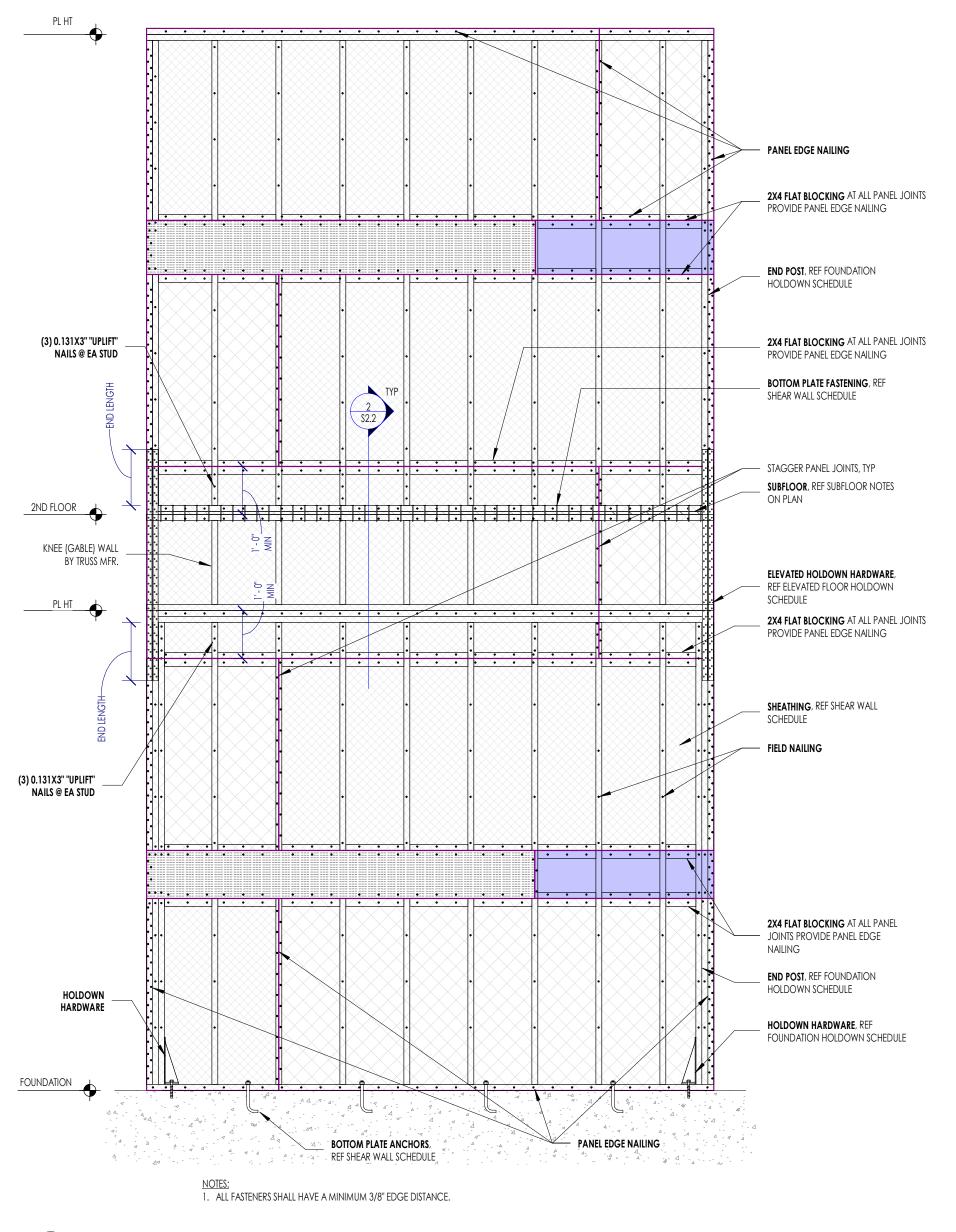
4A S2.3 TYPICAL MULTIPLE STORY SHEARWALL FRAMING AND FASTENING_TRUSSES PERPENDICULAR 1/2" = 1'-0"

NOTES: 1. ALL FASTENERS SHALL HAVE A MINIMUM 3/8" EDGE DISTANCE.

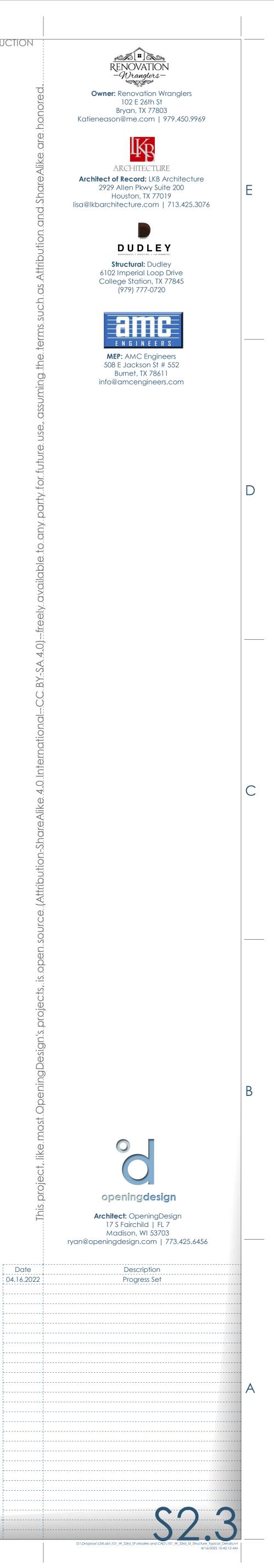
REF SHEAR WALL SCHEDULE

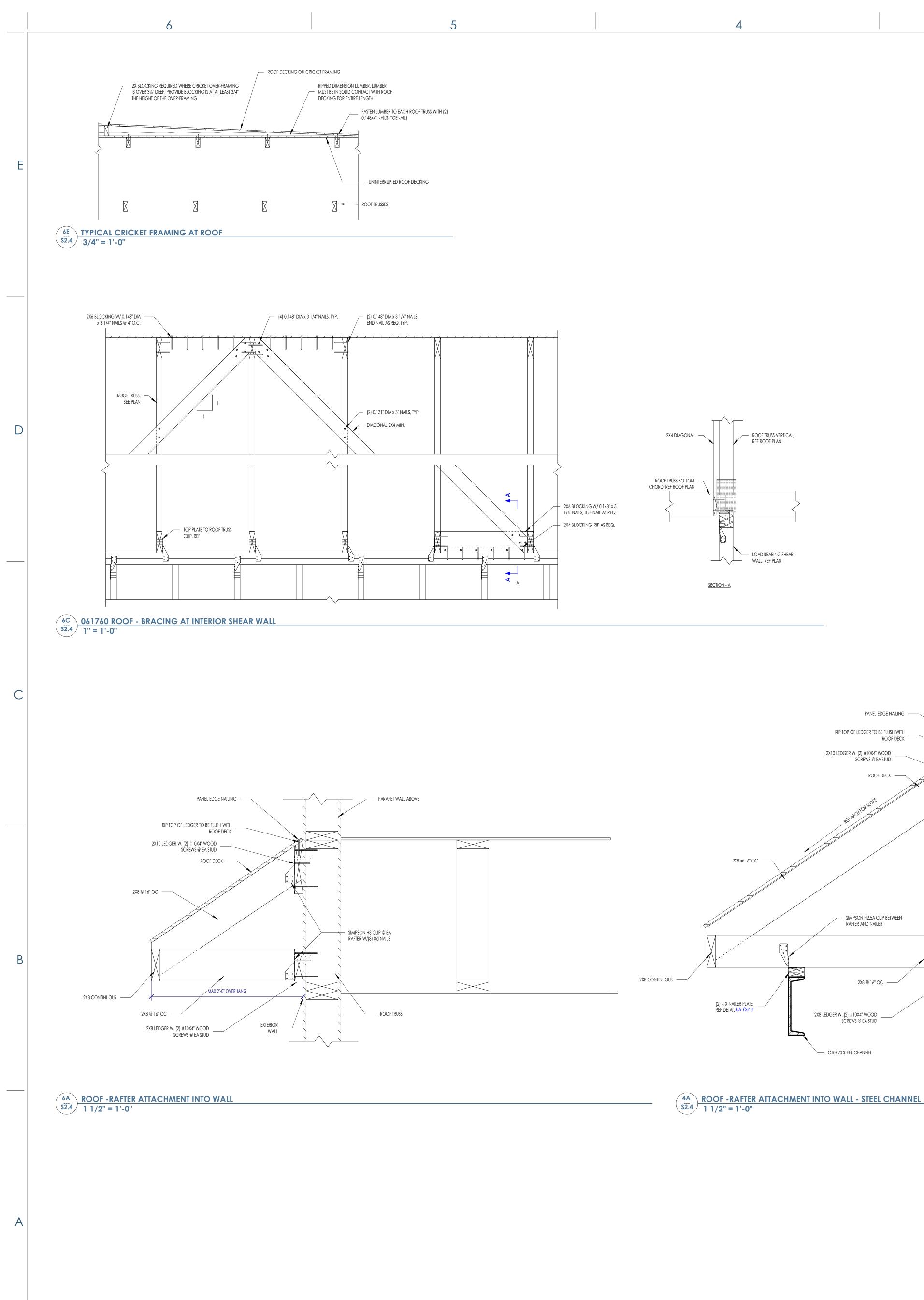


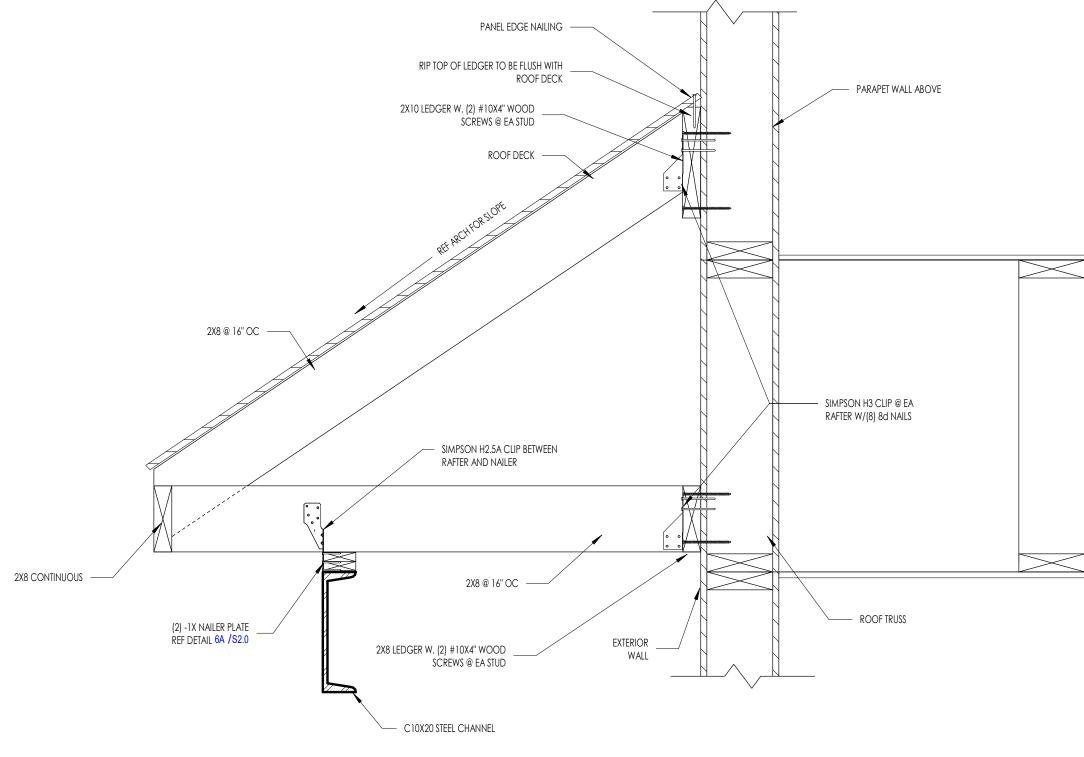




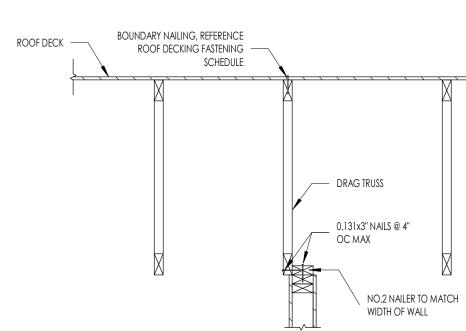
2A TYPICAL MULTIPLE STORY SHEARWALL FRAMING AND FASTENING_TRUSSES PARALLEL s^{2.3} 1/2" = 1'-0"

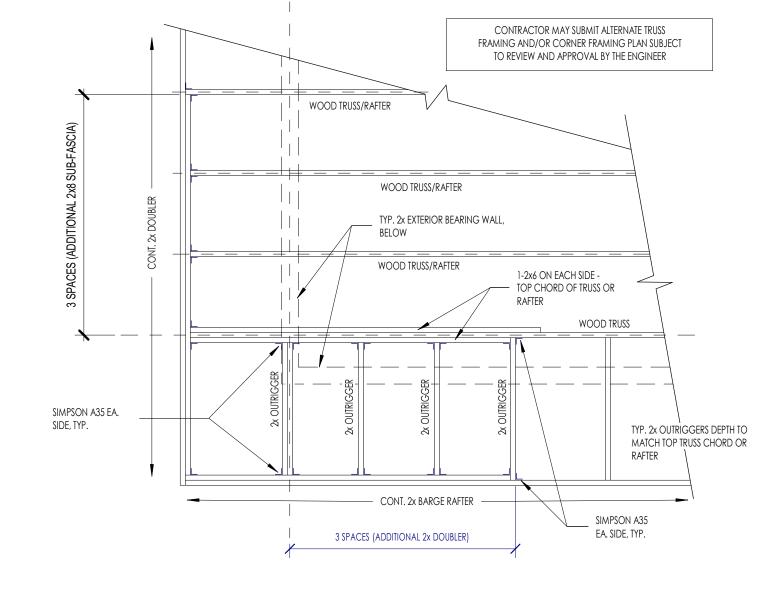




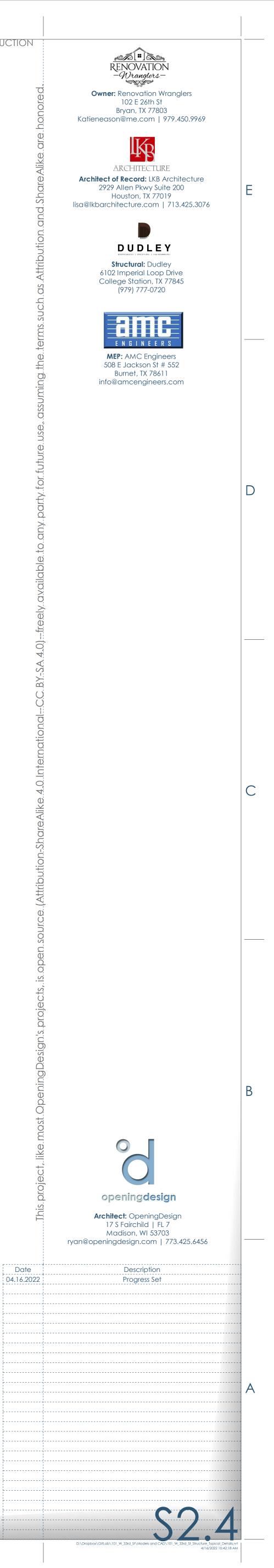


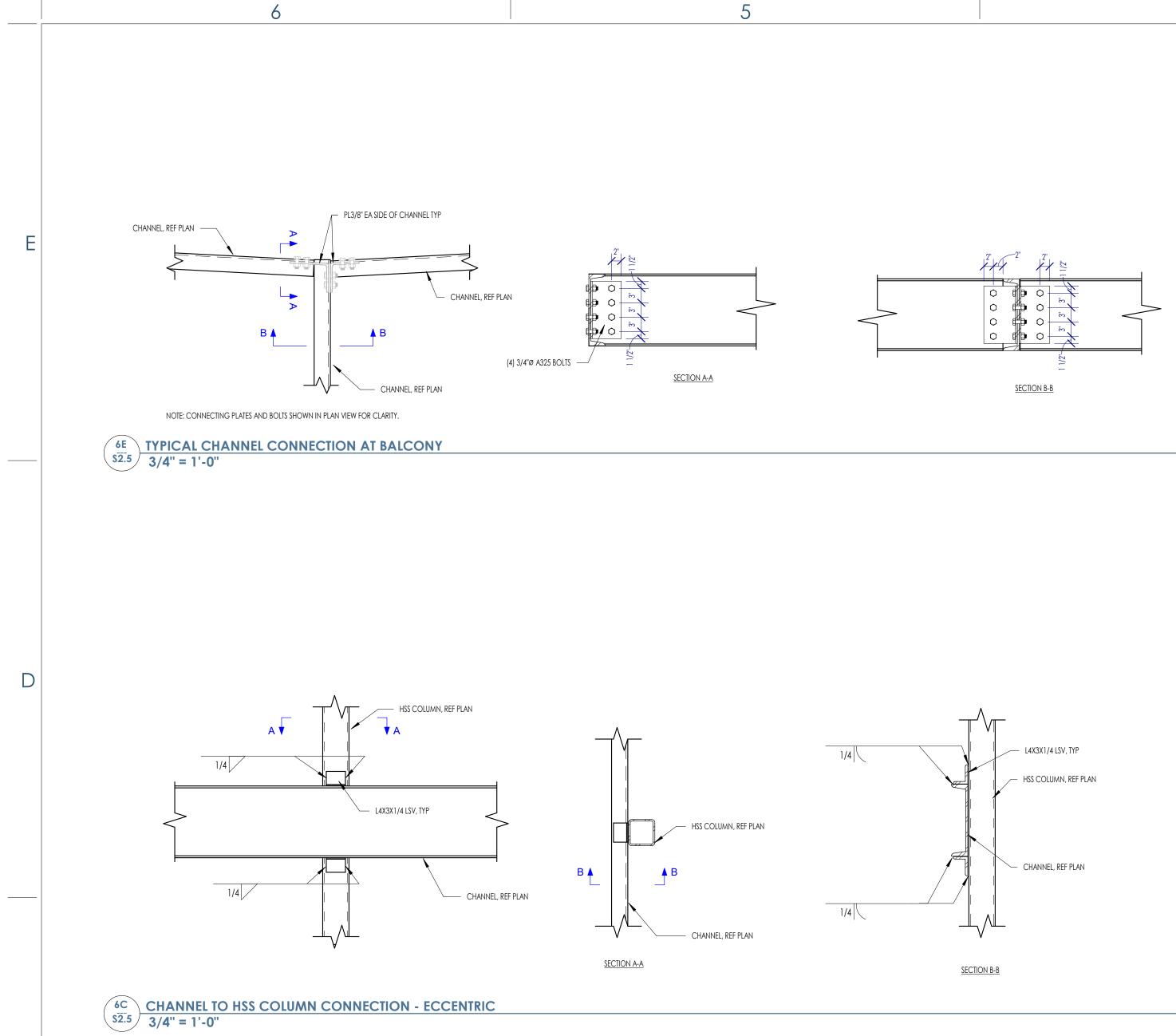








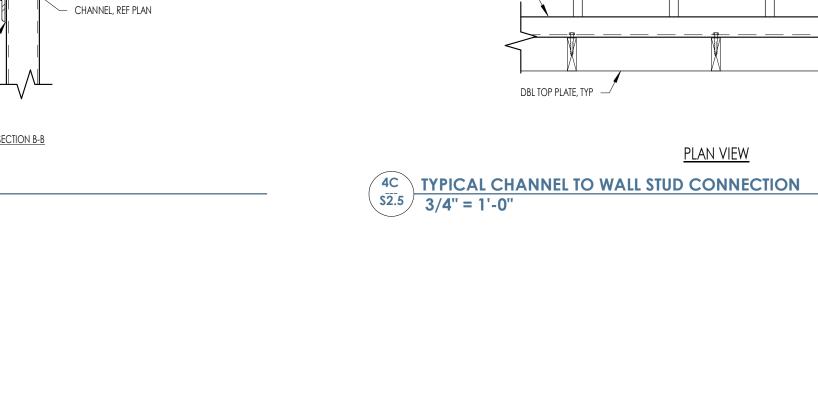




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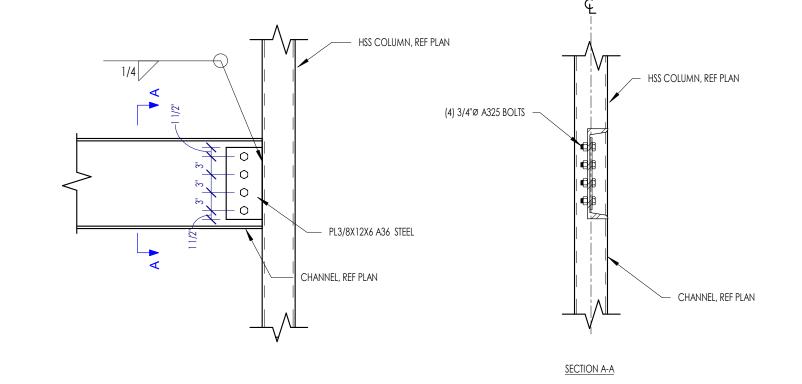
6

5



CHANNEL, REF PLAN

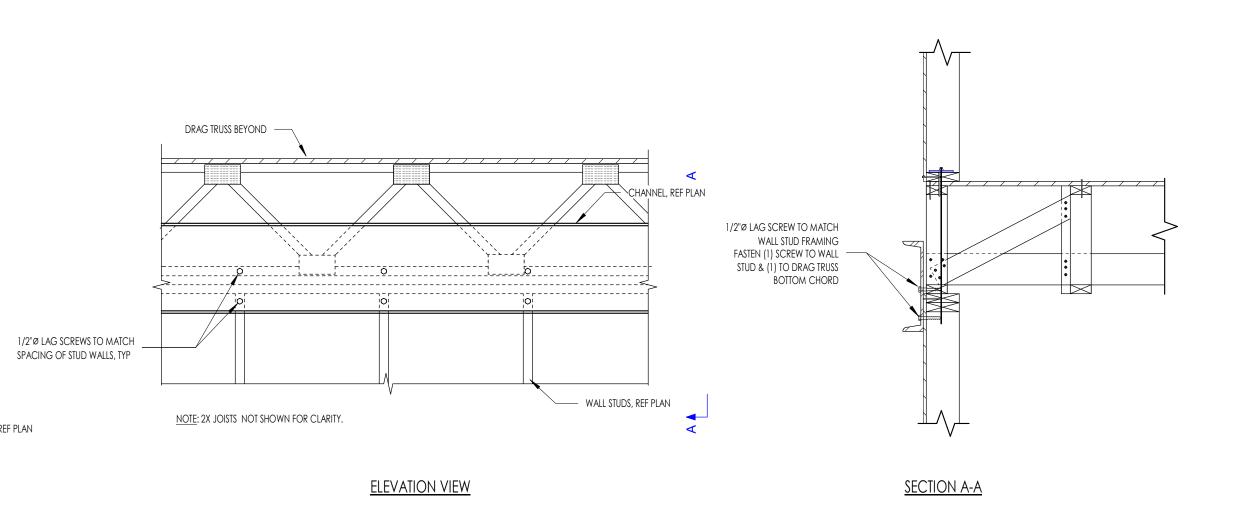




3

/--- 2X Joists, Ref Plan

WALL STUDS BELOW TOP PLATE, REF PLAN



2

TYPICAL STEEL DETAILS GREATEST PROJECT EVER - SOMEWHERE, TX

